

Attachments:

Volume 10 – Attachments

Delete the following pages and in lieu thereof replace the accompanying pages:

<u>DELETE</u>	<u>REPLACE</u>	<u>DESCRIPTION</u>
N/A	MRWJ-117-1974 MR Subsurface Investigation	New
N/A	Bethesda Visual Survey Photographs	New

REPORT NO. 18  
CONTRACT MODS. NOS.  
3Z725K-003, -008 AND -015  
(REPORT NO. 117, MRWJ SERIES)  
SECTION A011, ROCKVILLE ROUTE  
SUBSURFACE INVESTIGATION

Washington Metropolitan Area Transit Authority  
600 5th Street N.W.  
Washington, D. C. 20001

DeLew, Cather and Company, Inc.  
600 5th Street N.W.  
Washington, D. C. 20001

Mueser, Rutledge, Wentworth & Johnston  
415 Madison Avenue  
New York, New York 10017

December 30, 1974

BIBLIOGRAPHIC DATA SHEET	1. Report No. MRWJ-74-117	2.	3. Recipient's Accession No.
	4. Title and Subtitle Subsurface Investigation, Section A011, Rockville Route		5. Report Date December 30, 1974
7. Author(s) Mueser, Rutledge, Wentworth & Johnston	8. Performing Organization Rept. No.		6.
9. Performing Organization Name and Address Mueser, Rutledge, Wentworth & Johnston 415 Madison Avenue New York, New York 10017		10. Project/Task/Work Unit No.	11. Contract/Grant No. 3Zj25K, Mods. 3, 8 & 15
12. Sponsoring Organization Name and Address Washington Metropolitan Area Transit Authority Washington, D.C. 20001		13. Type of Report & Period Covered	
15. Supplementary Notes		14.	
16. Abstracts Results are summarized herein of 59 deep borings made at the locations of running tunnels, two subway stations in rock and various ancillary structures in Section A011 of Rockville Route, generally following Wisconsin Avenue between Elm Street and the Capital Beltway in Maryland northwest of the District of Columbia. The investigation include photography in a selected group of borings, special laboratory tests of rock drillability and direct shear characteristics of joint planes in addition to the usual unconfined compression and in-situ water pressure testing of the rock. The report includes a continuous geological section along the line of the tunneling tunnels through the test borings, logs of these borings, results of laboratory tests on soil samples and on rock cores and summaries of the special rock investigations performed by others.			
17. Key Words and Document Analysis. 17a. Descriptors Urban Transportation Mass Transportation Urban Planning Rock Mechanics Soil Mechanics			
17b. Identifiers/Open-Ended Terms Foundation Engineering			
17c. COSATI Field/Group			
18. Availability Statement Release Unlimited		19. Security Class (This Report) UNCLASSIFIED	21. No. of Pages
		20. Security Class (This Page) UNCLASSIFIED	22. Price

MUESER - RUTLEDGE - WENTWORTH - & - JOHNSTON  
*Consulting Engineers*

WILLIAM H. MUESER  
PHILIP C. RUTLEDGE  
PAUL M. WENTWORTH  
ROBERT C. JOHNSTON  
SALVATORE V. DESIMONE  
JAMES P. COULD  
ELMER A. RICHARDS  
EDMUND M. BURKE

*Partners*

415 MADISON AVENUE  
NEW YORK, N. Y. 10017

212 ELDORADO 5-4800

DANAROMIEL, NEW YORK

JAMES D. PARSONS  
NICHOLAS W. KOZIAKIN  
MAX BERNHEIMER  
GEORGE L. MOORE

*Senior Associates*

DOMINIC A. ZARRELLA  
PETER H. EDINGER  
PHILIP L. CALDWELL  
VYTAUTAS ANONIS  
WALTER WALCHUK  
HUGH S. LACY

*Associates*

December 30, 1974

Mr. Roy T. Dodge, Contracting Officer  
Washington Metropolitan Area Transit Authority  
600 5th Street N.W.  
Washington, D.C. 20001

Re: Report No. 117, Contract Mods. Nos. 3Z725K-003,  
-008 and -015, Section A011, Rockville Route,  
Subsurface Investigation

Dear Mr. Dodge:

In accordance with the terms of Modifications Nos. 003, -008 and -015 of Contract No. 3Z725K, we submit herewith fourteen copies of this report covering fifty nine borings made at the location of running tunnels, two subway stations and ancillary structures of Section A011 of Rockville Route. This work was done at the request of the Section Designer, Mathews-Chatelain-Beall, and one copy of the report is being sent directly to their Rockville, Maryland office. Because considerable delay had been occasioned by the rather complicated investigations contained in this study, a final draft of this report was issued on October 31, 1974 which was circulated to MCB to assist them in completing their geotechnical report. At the present time a group of supplementary borings requested by the Section Designer at the location of a planned parking facility in the Pooks Hill area is currently in progress. This work is to be covered by an additional modification and will be reported separately when field work is completed.

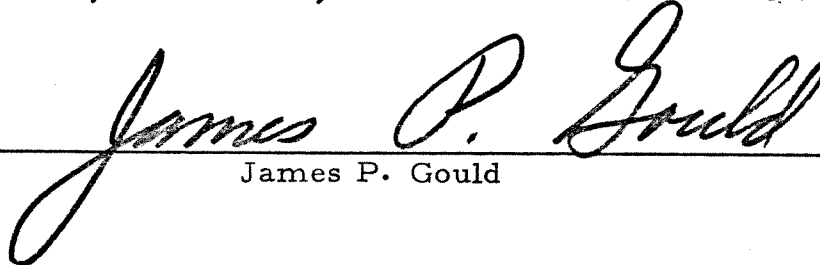
Several separate reports by special consultants have been authorized in this investigation and basic conclusions from these reports have been reprinted and assembled following the text

and tables. Each one of these reports is fairly voluminous and an original is being submitted to your office so that they may be reproduced as needed as information for bidders. We believe that this submittal completes all of the work required under the three Contract Modifications.

Very truly yours,

MUESER, RUTLEDGE, WENTWORTH & JOHNSTON

By

  
James P. Gould

JPG:ig

REPORT NO. 18  
CONTRACT MODS. NOS. 3Z725K-003, 008 & 015  
(REPORT NO. 117, MRWJ SERIES)  
SECTION A011, ROCKVILLE ROUTE  
SUBSURFACE INVESTIGATION

1. INTRODUCTION

1.1 Authorization

Contract No. 3Z725K between the Washington Metropolitan Area Transit Authority (WMA TA) and Mueser, Rutledge, Wentworth & Johnston (MRWJ) provides for boring investigations, laboratory testing and summary reports relating to certain sections of the Rapid Transit System. A series of three Modifications to that Contract covered exploration work requested by Mathews-Chatelain-Beall, designers of Section A011, to provide information for their planning and design of contract work in that section. These modifications are as follows:

1. Modification No. 003 provided for 54 borings to be made generally throughout the section to supplement earlier data and to cover changes of alignment.

2. Modification No. 008 provided for 10 borings chiefly in connection with a change in location of the planned Medical Center and its ancillary facilities.

3. Modification No. 015 provided for four borings to be made on the median strip of Wisconsin Avenue along the centerline of Medical Center Station at its final planned position.

The Section Designer stipulated that special testing of rock cores be carried out to obtain information on the practicability of drilling with a tunnel boring machine and to determine typical friction angles on joint planes along with the usual rock compression and water pressure tests.

Several earlier investigations had been made in this area which are summarized in the following MRWJ reports:

1. Preliminary Subsurface Investigation, Adopted Regional System 1968, dated February 1969 (MRWJ Series Report No. 33) covering borings of the 7-series.

2. Final Report, Subsurface Investigation, Rockville Route, Stations 221 (A009) to 590 (A013), dated March 1972 (MRWJ Series Report No. 58) covering borings of the A-series.

Borings from these earlier studies that are pertinent to the locations currently investigated have been utilized herein. In addition to the reports specifically concerned with Section A011, MRWJ Report No. 82 of April 18, 1973 covers supplementary work in Section A009 which abuts A011 on the south. Additional exploration in Section A013 which abuts A011 on the north has recently been completed and will be included in MRWJ Report No. 121.

### 1.2 Scope of Work

A total of 59 test borings of the AM-series covered by the three contract modifications were performed by Empire Soils Investigations, Inc. working continuously from September 1973 to August 1974 under the inspection of MRWJ resident engineers. While as many as four rigs were stipulated in the contract for field operations, difficulties with obtaining access permits and numerous changes of location occasioned by planning decisions caused the work to extend far beyond the expected time. At the date of this report a group of seven supplementary borings requested by the Section Designer were in progress at a planned parking facility at Pook's Hill. Five observation wells were placed in boreholes for continuing measurement of ground water levels. Casings were left in place through the overburden and seated in rock to prepare for photography in five selected borings at the two stations. Overburden sampling included the standard 1-1/2 inch diameter split-spoon and 3-inch undisturbed samples, all of which were delivered to our New York laboratory for examination, check of field classification and performance of identification and engineering properties test. Rock drilling was of NX-size with cores of 2-1/8 inch diameter, except that three borings were made with a larger barrel yielding 4-inch diameter cores. Special field investigations included borehole photography, performance of water pressure tests in a number of borings and routine falling-head permeability testing in any borings as conditions permitted. Selected rock core specimens were subjected to the following categories of tests: conventional unconfined compression, petrographic analysis of thin sections; hardness tests relating to drillability characteristics; and direct shear tests on certain rock joints.

Because of the length of time over which this investigation extended it impinged to a significant degree on the work of the Section Designer and consequently it was necessary to meet with the designer on a number of occasions to examine rock cores obtained, to review borehole photography

results and to discuss basic design criteria. These meetings were summarized in a number of letters and memoranda from our office.

## 2. BORING AND FIELD TEST PROGRAMS

### 2.1 Presentation of Boring Data

Locations of all available borings of the several series are plotted on strip maps in ten segments on four drawings, Nos. F-A-86 through -89. Information from the 59 supplementary borings is plotted on a series of ten geological section drawings Nos. F-A-90 through -99, providing a continuous longitudinal profile through Section A011. The sections commence at the north end of the line near the Capital Beltway and continue to the south limit at Elm Street with the direction of view toward the east. On two of these drawings incorporating Medical Center and Bethesda Stations two parallel longitudinal sections have been drawn, one for the east side and one for the west side of the station. In addition a group of four natural scale cross-sections at each of the two stations are presented on four drawings, Nos. F-A-100 through F-A-103. Boring and sampling data and bedrock index properties from borings of the AM-series are shown in symbol form on these sections in the manner employed in earlier investigations. Detailed information from borings of the A-series and 7-series covered by earlier referenced reports are not repeated on these sections.

Logs of all AM-series borings made in the current investigation are given on Drawings Nos. F-A-105 through F-A-122. These logs are based on our laboratory examination and identification testing of samples, and, in a number of cases, on selected review of the rock cores and they differ in certain interpretations from the original field boring logs copies of which will be placed on file with WMA TA. Coordinate locations of the borings are listed in Table No. 1. General Information Drawing No. F-1 presents notes and legends applicable to the geological section and boring log drawings.

In an effort to maximize core recovery and retain the core as much as possible in an undisturbed condition, a split-barrel diamond drill coring device was stipulated in the specifications. This comprises an outer coring barrel and an inner retaining tube both split longitudinally with the inner tube secured by tapes. After a core run is complete the barrel is removed and the outer and inner tubes opened. A semi-cylindrical cardboard holder is placed over the core and that holder with the core is turned over and transferred to the core box. In this manner the relative position of rock pieces is maintained and broken or weathered fragments are



kept as far as practicable in their original arrangement and orientation.

## 2.2 Borehole Photography

In accordance with the Section Designer's request and to provide additional details relating to joint attitude of bedrock, borehole photography was carried out within the rock sections of five of the supplementary borings, two at Medical Center Station and three within the planned Bethesda Station. This work was performed between June 6 and 8, 1974 by a field party of the U.S. Army Corps of Engineers, Southwestern Division Laboratory of Dallas, following a request to the Department of the Army for these services from WMA TA. In order to obtain an independent expert interpretation of the photographic results, MRWJ retained the services of Mr. Robert J. Nesbitt, Consulting Engineering Geologist, formerly Chief Geologist in the office of Chief of Engineers, Department of the Army, who has been instrumental in the development of the borehole photography technique. His report dated August 20, 1974 includes a summary made by Southwestern Division Laboratory which gives a brief commentary on the character of the bedrock, detailed photologs, polar diagrams of joint frequency versus attitude and plots on which the joint planes noted in the borings are projected to the tunnel cross-sections. That complete information is on file at WMA TA and should be consulted for a comprehensive record of the results. The text of Mr. Nesbitt's summary has been reprinted and included with this volume. On September 4, 1974 he reviewed his interpretation in a meeting with the Section Designer at the rock core storage in WMA TA warehouse.

Photographs were taken utilizing the Republic Engineering and Manufacturing Company NX-borehole camera. The camera is enclosed in a probe 2-3/4 inch in diameter and 30 inches long which is lowered in the hole to take a series of pictures on a 16mm color film strip at 3/4 inch vertical intervals for the full length of the borehole in rock. The pictures projected on a flat screen exhibit the series of annular sections which represent a 360° view of the 3/4 inch high portion of the walls of the hole with an arrow indicating magnetic north photographed within the interior circle. If this annulus is projected onto a cylindrical screen, using procedures developed in the office of Chief of Engineers, the strike of structural features and a dip angle can be determined. It should be noted that the summary of Southwestern Division Laboratory and Nesbitt's report refer to magnetic north as viewed directly in the photography. Declination of magnetic north at this longitude is approximately 7° west of true north which is used as a basis for the project grid and the mapping coordinates.

Three borings at Bethesda Station, Nos. AM-3U, -4 and -60 were cased through overburden down to the depth at which continuous coring started for the photography. Despite protracted cleaning, the water in AM-3U remained cloudy, probably because of sloughing of fines from weathered bedrock directly beneath the casing, and the photographs were unsatisfactory. At Medical Center Station Borings Nos. AM-68 and -70 were prepared for photography. The same difficulty with fines continuously sloughing from beneath the casing was encountered in No. AM-68 and its pictures were unsatisfactory.

The significance of the data obtained from the borehole photography is discussed in later sections on subsurface conditions.

### 2.3 Falling-Head Permeability Tests

During the supplementary exploration a total of 58 falling-head permeability tests were performed in the field in cased boreholes and in standpipes of the observation wells. Information regarding conditions of the test and computed permeability coefficients are listed on two sheets of Table No. 3. Tests were performed by raising the water level and observing the time rate of return to an equilibrium position. The differential head thus obtained plotted on a logarithmic scale against elapsed time on an arithmetic scale should form a straight line. A coefficient of permeability is computed by dividing the straight line slope by a shape factor whose value depends on the flow pattern to the borehole. The principal stratum opposite the opening in the borehole, which is assumed to control inflow to the opening during the test, is listed in Table No. 3. These tests were chiefly performed in portions of the bedrock which were open below the casing which had been carried through overburden.

The median value of all these tests equals  $2 \times 10^{-4}$  ft. per minute which indicates the generally tight nature of the rock encountered. This result is similar to median values obtained in similar tests in adjacent Section A009. There are several instances where water could not be added to the borehole fast enough to create an observable rise in the water level and in these instances the permeability is estimated to be greater than  $1 \times 10^{-2}$  ft. per minute. Borings exhibiting unusually high permeability included Nos. AM-13, AM-23 and AM-46 in the deeply weathered upper portions of bedrock where a series of zero RQD values were obtained in the coring. It should be noted that the permeability values are not necessarily a realistic indication of the quality of rock since in the falling head tests fine particles can move outward into voids and joints, reducing their apparent overall permeability. Joints which are weathered to the extent that they contain a coating of soil may exhibit a low permeability and little flow potential in the tests but they would respond in quite a different manner to inflow of seepage after the disturbance

of the joint system during excavation.

#### 2.4 Water Pressure Tests

A total of 53 individual water pressure tests were performed below drill casing within bedrock sections of the boreholes and basic information and permeability values computed from these tests are noted on seven pages of Table No. 4. The purpose of such a test was to investigate variations in bedrock permeability to ascertain if highly pervious jointed zones existed where intense inflow of seepage might occur. It was expected that zones exhibiting high permeability might also be a location of poor quality rock which could lead to structural difficulties in construction. After the NX-size borehole was cased to the top of rock and completed to the planned bottom elevation, the hole was cleaned until wash water appeared to be clear of fines. Then a pipe containing two hard rubber packers with a perforated pipe section about 5 feet long between packers was lowered to a predetermined depth below top of bedrock. The test was performed by pumping water into the space between packers through the pipe which extended through the upper packer to the surface. Gage pressure on the water pipe was build up to maximum values between 30 and 90 psi in the tests. After reaching this pressure the total quantity of inflow into the hole was measured by a calibrated water meter and recorded at 1 to 5 minute intervals over a total testing time of typically 30 minutes to one hour. From these records the average time rate of inflow into the opening was determined and permeability was computed by applying a shape factor to the physical qualities obtained during the test.

Of 53 tests a total of 22 accepted no measurable inflow of water for the duration of test. Median permeability of the other 31 tests equals  $2 \times 10^{-4}$  ft. per minute, essentially equal to the falling-head test results. Test values of coefficient of permeability in feet per minute are noted in the geological sections at a position on the boring which represents the length exposed between packers. Unfortunately, because many of the borings were made before the final grade of the rock tunnels were established, a number of the tests are for depths below the tunnel opening.

### 3. LABORATORY TESTING PROGRAM

#### 3.1 Soil Testing

In this supplementary investigation of Section A011 the character of overburden soils was primarily of interest in connection with shaft and

entrance construction and for the retained cut and cut-and-cover excavation at the north end of the section. Thin-tube sampling was confined to recovery of 3-inch undisturbed tubes all of which were transported to our New York laboratory for tests of engineering properties. These included determination of water content and occasional grain size and Atterberg limit tests, plus measurement of shear strength by unconfined compression, unconsolidated-undrained triaxial shear and direct shear tests. Results of all testing performed on undisturbed samples are summarized in Table No. 2. Split-spoon samples were examined in the laboratory, field classifications were checked and water contents determined on all fine-grained materials. Unified Classification of each soil sample and the average natural water content are noted on the geological sections at the location of the particular sample in the boring.

Overburden soils throughout A011 comprise material residual from decomposition of the bedrock and fill composed chiefly of this decomposed rock. Six Atterberg limit tests were performed on the residual soil and the fill with results summarized on a plasticity chart of Plate No. 1. Grain-size characteristics of the decomposed rock are quite regular and are portrayed by a series of 15 gradation curves in Plates Nos. 2, 3 and 4.

### 3.2 Strength of the Residual Soil Stratum of the Decomposed Rock

Unconsolidated-undrained (UU) triaxial tests are carried out in the triaxial cell, the confining pressure applied without permitting drainage, followed by shear while increasing axial load, also with no drainage. The confining pressure has the effect of restraining failure which might occur at too low stresses due to the presence of sand pockets or partings, or fractures and cracks, and it compensates in part for possible sample disturbance. Ideally undisturbed, homogeneous, saturated specimens of particularly clayey soil from a single sample sheared undrained under different chamber pressures should all exhibit the same deviator stress at failure and therefore the Mohr's envelope for such a group of tests would plot as a horizontal straight line. A total of 48 of these triaxial tests were performed in this study on samples of decomposed rock of Stratum D or, in several instances, on the overlying fill derived from this material and these tests are summarized in sixteen Mohr's envelope diagram in Plates Nos. 5 through 20. The overall average values of shear strength from these triaxial tests are as follows:

8 tests on fill: average = 2.6 ksf

40 tests on decomposed rock: average = 2.3 ksf

In a number of instances the decomposed rock samples are coarse enough to act essentially as a friction producing material in the triaxial tests, exhibiting

a significant increase in strength with confining pressure. Eight Mohr's circle diagrams with this characteristic give the following strength parameters:

$$\begin{aligned} \text{Angle } \phi &= 24^{\circ} \\ \text{Apparent cohesion} &= 0.3 \text{ tsf} \end{aligned}$$

These values suggest that small to medium positive pore water pressures are developed in the decomposed rock during shear. They are to be compared with the following results of seven drained direct shear tests recorded on Plates Nos. 21 through 27:

$$\begin{aligned} \text{Angle } \phi &= 30^{\circ} \\ \text{Cohesion} &= 0.2 \text{ tsf} \end{aligned}$$

It must be recalled that the thin-tube sampling could recover only the upper thoroughly decomposed residual soil. In fact no undisturbed samples were obtained below 24-foot depth. Below some such level the material becomes denser, coarser and increasingly rock-like. The samples tested are undoubtedly softer than the lower, less thoroughly decomposed, material. In the residual soil above about 20-foot depth the shear strength may be expressed by an equivalent friction angle of  $32^{\circ}$ . In the transition zone between the decomposed rock and weathered rock a friction angle of  $36^{\circ}$  is suggested.

### 3.3 Unconfined Compression Tests on Rock Cores

Results of 76 unconfined compression tests performed on specimens selected from NX-rock cores of the AM-series borings are summarized in Table No. 5. These specimens were taken from a range of elevations to represent material that would appear within or directly above the tunnel opening and since the choice was made in later stages of the boring program the core positions correspond fairly well with final tunnel locations. NX cores of 2-1/8 inch diameter were cut in lengths of 4-1/4 to 4-1/2 inches and tested without confining pressure in a controlled stress rate machine wherein stresses were increased at an average rate of 35 psi per second until failure. Test values of peak compressive strength range from 500 psi to 27,500 psi and reflect both differences in basic rock type and in the degree of weathering or alteration of the rock. Individual values in kips per sq. in. are noted at the position of the core tested on the geological sections within a rectangular symbol. The average of all 76 compressive strength tests equals 8,700 psi. To put these data in perspective they may be considered with results of similar compression testing on earlier designs of Rockville Routes.

These results are generally as follows:

Section	Boring Series	No. of Tests	Average Compressive Strength, psi	Average RQD of Tested Cores
A006	III-Series	127	6,400	not taken
A006	RP-Series	45	7,500	75
A009	A-Series	82	7,500	70
A011	A-Series	34	9,600	65
A011	AM-Series	76	8,700	79

The unusually high values that were obtained in certain tests apply to indistinctly foliated quartz-hornblende diorite and the lower values apply to schistose rock with strong foliation and comparatively high percentage of hornblende or chloritic minerals. It should be recognized that the rock compression test is primarily an index or classification test, indicating relative quality of rock from various locations and depths. Even the weaker test strengths cannot be applied directly in design since they would show that the rock has a quality of mediocre strength concrete. While foliation and weakening of inter-particle bonds by weathering undoubtedly influences many test values of compressive strengths, the crucial material which controls loads applied to the temporary or permanent supports of the underground structures has been broken in the coring and cannot be tested or is missing entirely in the recovered cores.

#### 3.4 Tests Relating to Drillability Characteristics of Rock

To evaluate the feasibility of using a tunnel boring machine on A011 contract work, the Section Designer requested a special series of tests relating to rock drillability be undertaken at the University of Illinois. A group of 38 core specimens were selected for drillability tests from the AM-series borings and delivered to the University of Illinois laboratory. A preliminary report by P.J. Tarkoy and E.J. Cording was submitted on September 17, 1974 and their final report, entitled, "Feasibility of Using a Tunnel Boring Machine on Project A-11, METRO, Washington, D.C." was dated December 10, 1974.

Since tunnel boring machines have been utilized in the running tunnel contract of Section A006, an important body of field experience is available which may be relevant to other sections of Rockville Route. In order to be able to utilize this experience in comparison with Section A011, we asked P.J. Tarkoy to select a series of samples from the A006 core boxes, 22 of which were collected by our field staff and delivered to their laboratory.

A complete series of their index properties tests consist of determination of rock unit weight, unconfined compressive strength, sonic velocity, and four parameters relating to hardness. A combination of two of these latter parameters in the "total hardness" appears to be the most useful factor in establishing an empirical relationship with TBM rates of penetration. Total hardness has been determined for all drillability samples from A006 and A011 and the complete set of index properties performed on certain samples selected from both groups. The locations of all core pieces selected for drillability tests in A011 are not noted by the symbol "DR" on the geological sections. Results of these tests are noted in Tables IIa, IIb and III of their December 10 report. In general the rock types of Section A011 exhibit more uniform and a somewhat higher average total hardness value than the samples of Section A006. Overall characteristics are similar enough to expect a TBM operation in A011 comparable to the rather favorable experience in A006. For identification, the title page and several summary pages of their text are bound in this volume at the end of our report. Their full report will be available for bidder's information. Included with their report as Appendix A are a series of petrographic analyses for sixteen rock chips from cores taken chiefly in the Medical Center Station area. These were performed by R. Kern of the University of Illinois in connection with A011 rock studies and that data was provided to assist in evaluation of drillability characteristics.

### 3.5 Rock Joint Shear Test

Pursuant to the Section Designer's request, a group of seven rock core specimens were selected from the AM-series borings for direct shear tests on natural rock joint planes to be performed at University of Illinois. These specimens each consisted of two adjacent core pieces meeting at a prominent joint. The joints are principally in diorite or meta-diorite with smooth but not slickensided surface containing little or no clay gouge but thin talc coatings. These were chosen because of the difficulty of preserving joint specimens with appreciable clay gouge and in consideration of the fact that gouge-filled joints had been tested previously by University of Illinois in Section A006. Their preliminary report of September 27, 1974 summarized test results on joint specimens taken previously in Sections A004b and A006. A final report by A.S. Nieto and E.J. Cording, entitled, "Direct Shear Tests on Natural Fractures in NX Core from Project A-11, Washington, D.C. METRO,"

of December 11, 1974 gives details on the results of direct shear tests on 5 joint specimens from Section A011. The title page and certain summary pages from each of these two reports are bound in this volume. The complete data will be available for bidder's information. They conclude that a conservative estimate of the shear strength for shear zones could be obtained with a total friction angle of  $15^{\circ}$ .

#### 4. OVERALL SUBSURFACE CONDITIONS AND GEOLOGIC SETTING

##### 4.1 Principal Rock Types

The igneous and metamorphic bedrock complex in the Washington, D.C. area originated in the upgrading by metamorphism of broad areas of clayey and sandy sediments of lower Paleozoic Wissahickon formation. This material is part of an extensive band of ancient rocks extending from New England to Alabama and represented by such materials as the Manhattan group of New York City and Wissahickon schist in Philadelphia. The principal types represented in Section A011 are as follows:

- (1) Typical schistose gneiss of the country rock comprising chiefly quartz and mica with lesser hornblende and including related chloritic rocks.
- (2) Quartz-diorite gneiss, generally comprising metamorphics derived from coarser parent materials than the schistose gneiss. These also include meta-diorites and diorites of indistinct foliation which exhibit less metamorphism than the diorite gneiss but comprise similar mineral aggregates.
- (3) A group of amphibolites including hornblende schist or schistose gneiss and chlorite schist ordinarily represents the weakest materials derived from the finest grained sediment.

A geologic study of a nearby area was recently completed by F.D. Patton, consulting engineering geologist, for Sections B009, B010 and B011 of Glenmont Route and presented in his report to WMA TA dated July 29, 1974. That report included a geologic map of an area adjacent to Glenmont Route north of the District boundary. A portion of his data has been utilized in our map of Plate No. 28 which illustrates the general banding of rock types and principal topographic features between Glenmont and Rockville Routes.



#### 4.2 Attitude of Rock Structure

Plate No. 28 shows that Section A011 following Wisconsin Avenue lies on the west of the drainage pattern to Rock Creek, at a position near the head of a number of streams draining in echelon on lines directed N60°E to N40°E, separated by low ridges similarly oriented. The diorite gneiss group is the dominant rock within Section A011. Schistose gneiss and amphibolites cover much less of the length of section but represent weaker and more difficult materials. Locations of poorer bedrock quality or deeper weathering typically correspond with topographic lows.

In general the bedrock structure mapped by Patton in the Glenmont area has a strong north-northeast to south-southwest grain in the Piedmont north of the District boundary. Foliation and the principal faults shears and joints on Glenmont Route correspond to this lination. Available mapping in the literature and borehole photography in this study indicate foliation and principal joints in Section A011 strike a few degrees west of north and dip steeply west. Information from several sources on attitudes of the joint sets in the general area or specifically within Rockville Route is summarized in Table No. 6. The order of the joint sets is that established by University of Illinois investigations in the rock tunnels of A004.

#### 4.3 Conditions of Weathering

Section A011 in the upper reaches of Rock Creek drainage exhibits a substantial cover of materials that comprise the products of weathering in-situ. In an effort to avoid misunderstandings as to the character of these materials, a system of designations has been adopted for the overburden which is described in the following subsections:

4.301 Decomposed Rock. Overburden above bedrock in the profile of weathering has been grouped in the boring logs in one broad category as "decomposed rock" which is derived from decay of parent rock in-situ. Decomposed rock is primarily silty sand with scattered rock fragments in its upper portion, becoming coarser, fresher, denser and more rock-like with increasing depth. Borings were advanced in this overburden by driving a casing and washing and chopping inside the casing or in advance of the casing as the materials became more compact; or by rotary drilling with a roller bit for which casing was lowered in the hole to the level at which caving was no longer a threat. A borehole was carried by these methods to the level at which practical refusal was reached which is traditionally designated as the "top of bedrock". In the geological sections and the text of this report certain alterations have been made in these conventional interpretations.

4.302 Residual Soil. In this study the decomposed rock is divided into two categories, an upper "residual soil" which is expected to consist almost entirely of soil-like material and a lower "transition" zone which is expected to contain chiefly rock-like remnants surrounded and separated by soil. This distinction reflects the fact that weathering progresses downward on joint planes and zones of weakness, expanding in all directions from these loci as seeping water and oxidation has weakened the crystalline matrix. In the transition zone "core stones" may be surrounded by soil but the percentage of these core stones increases with depth as the thickness of soil between them decreases. The division between residual soil and the transition zone is important for any excavations for shafts or entrance structures which extend down to the rock tunnels. The estimated approximate dividing line between the residual soil and transition zone is plotted as a long dash-dot line in the sections and is noted by symbol on the boring logs and is distinguished as follows:

1. If a casing can be advanced by ordinary washing and chopping either within the casing or several feet in advance of casing, the material penetrated is still residual soil. Where casing reaches practical refusal and an open hole can be continued with a roller bit without caving the transition zone may have been entered.

2. Standard sampler penetration resistance at the base of the residual soil passes from a fairly high blow count to a very high blow count, changing rather abruptly from less than 100 blows per foot to much more than 100 blows per foot. Deere and Patton have suggested a penetration resistance of 100 blows per foot as the single criterion for the base of residual soil. We feel that this is perhaps the most important single indicator but the judgement should be weighed with other factors.

3. A split-spoon sample recovered from the lower residual soil consists of a solid chunk of very compact silty sand, usually deep red-brown in color, and showing much relic rock structure. Generally the sample will have natural moisture content exceeding 20 per cent but in some cases as low as about 15 per cent. The sample can be crumbled in the hand without great difficulty, the materials molded by hand pressure, and the sample is generally free of hard rock fragments. Spoon samples which can be obtained in the transition zone generally have moisture contents below 15 per cent but in some cases as high as about 20 per cent. Color of the sample may be gray or slightly greenish with mica fragments and iron staining less extensive than in the residual soil. Often the sample cannot be broken easily by hand pressure but requires a considerable exertion. When a roller bit is utilized the spoon sample may consist of cuttings and debris that have accumulated in the bottom of the hole before the sampler is driven.

4. While an attempt has been made to core with the diamond drill as high as practicable in the transition zone, often there is no core recovery and usually the core consists of only a few friable pieces. However the presence of these hard fragments does indicate that the transition zone was reached.

4.303 Transition Zone. From the base of residual soil, as defined above, the transition zone extends to a level designated on the geological sections as the: "approximate top of 'weathered bedrock'". Below this level diamond drill coring is essentially continuous and soil is much the lesser constituent. Core recovery at this division line is required to exceed about 50 per cent and RQD about 10 per cent. In some cases this corresponds to the level of top of bedrock conventionally taken where continuous coring commences. Deere has suggested that the base of the transition zone be taken at RQD exceeding 25 per cent. However, in Section A011 this would result in abrupt dips in the interpreted bedrock surface in many locations.

4.304 Bedrock Zone WR: Weathered and Jointed Bedrock. In this exploration and in earlier METRO investigations zone WR comprises the uppermost portion of the bedrock and is distinguished by pronounced effects of weathering and by relatively closely spaced joints. This is intended to include cores in which weathering has had a visible effect on the mineral fabric of the rock, weakening mineral contacts and leading to markedly lower compressive strengths than the original intact material. Rock quality designation (RQD) values generally are less than 50 per cent in materials included in WR materials. Significantly weathered rock extends to depths of 20 to 40 feet beneath nominal top of bedrock. Generally the effects of weathering and consequent weakening of the mineral fabric decreases continuously with depth. Evidence of intense weathering sometimes appears at greater depths in shear zones or in locations rich in chlorite or hornblende constituents.

4.305 Bedrock Zone J: Moderately Jointed to Jointed Bedrock. Those bedrock zones which are distinguished by joints spaced closer than about 1 foot apart are grouped in Stratum J on the geologic sections. Weathering in these materials is confined largely to staining or talc deposits in joint plans and has not visibly altered the mineral fabric of the rock mass. RQD values generally are in the range of about 50 to 75 per cent. Where the rock is essentially unweathered but joints are more closely spaced and RQD values are less than 50 per cent the bedrock is designated as "highly jointed".

4.306 Bedrock Zone R: Relatively Sound, Occasionally Moderately Jointed Bedrock. Stratum R material is that portion of the bedrock below the obviously weathered and extensively jointed upper zones with joints usually spaced at vertical intervals greater than 1 foot. Rock

quality designation ordinarily exceeds 75 per cent and occasionally reaches values of 90 or 95 per cent. Unconfined compressive strengths tend to be highest in core specimens taken from these materials because there is the least deterioration of mineral contacts. The term relatively sound is not intended to imply that the material is free of joints and weathering effects. It must be recognized that the methods of tunnel excavation can significantly alter the jointing condition where planes of weakness are incipient, sealed or cemented in the recovered core.

#### 4.4 Ground Water Conditions

As Plate No. 28 indicates, Section A011 lies in the upper reaches of drainage directed northeast to Rock Creek and consequently the line crosses a series of highs and lows formed by the drainage channels. These drainage channels are now marked by culverts passing beneath Wisconsin Avenue in areas where comparatively thick fill has been placed to smooth street grades. Water levels observed in the borings are portrayed by symbol on the geological sections and the estimated average ground water table for the date of field operations is shown as a short dashed line on the sections. The water table generally forms a subdued reflection of surface topography and always intersects the drainage structures beneath Wisconsin Avenue. Reliable measurements of water levels in these deep borings could generally be obtained since there was ample opportunity to make equilibrium overnight readings. There is no consistent indication of a distinct perched water condition in the overburden or depressed or artesian conditions in the bedrock. However, in any built-up area such as borders Wisconsin Avenue it is inevitable that streets and drainage structures can concentrate runoff and return it to the ground at positions many feet above the underlying permanent water table. Thus in shafts or open excavations continuously flowing seepage related to this shallow recharge may be encountered and no amount of systematic drawdown of the lower water table will effect flow from these perched water levels.

### 5. SUMMARY OF DESIGN AND CONSTRUCTION PROBLEMS

#### 5.1 General Considerations

It is expected that Section A011 construction will include a contract for continuous running tunnels from the south limit of the line at Elm Street to a portal near Pooks Hill. Bethesda and Medical Center Stations will be let in a separate contract. The circular tunnels will be approximately 20 feet in diameter with cast-in-place reinforced concrete lining and presumably will be bid on a lineal foot price without distinction as to type of

support or ground conditions. A series of vertical fan or vent shafts and station entrance structures are included.

In discussions with the Section Designer it has been agreed that both station arches will be designed at normal working stresses for an average vertical pressure of 3 kips per sq. ft., distributed in the form of a tent-shaped diagram as shown in basic design criteria. That loading has been checked by evaluating the stability of a number of discreet overbreak blocks bounded by typical conjugate joints with friction angles of  $15^{\circ}$  which apply relatively high pressures over limited portions of the station arch in directions not normal to the arch line. These analyses have been processed by computer in a two-dimensional treatment of the stations. We have expressed the opinion that this design is probably exacting enough for the poorest conditions revealed in the borings and is distinctly conservative for the more favorable conditions of the stations' roof rock. Circular running tunnels have been designed for an average vertical load of 1 kip per sq. ft. at normal working stresses. We understand that the lining chosen will accommodate double this loading with both steel and concrete below yield point stresses throughout the section. Under a 3 ksf load yield in tension would occur on the outside of the concrete and thus it was concluded that the design contained a substantial allowance for local conditions much poorer than those assumed.

In the geological sections the top of tunnel opening has been plotted as well as the top of rail so that conditions within the roof rock may be better visualized. It must be recalled that except at the stations these borings have been projected from either side of Wisconsin Avenue onto a common plane and there are obvious distortions in an attempt to plot average profile conditions on a median plane. Under these circumstances the delineating of various qualities of rock by straight lines connecting borings on the sections is simply a device to visualize overall conditions and reliance cannot be placed upon the accuracy of the rock zone lines. It has been found to be excessively complicated to distinguish most of the detailed changes of lithology on the drawings so the overall rock type is merely noted at the bottom of the drawings.

In the following text the particular rock conditions relating to design and construction are discussed within successive lengths of Section A011, commencing at the north end near the Capital Beltway on Drawing No. F-A-90 and proceeding to the south limit of the line.

## 5.2 Stations 515 to 510, Retained Cut

The bottom of the retaining wall falls almost everywhere within fill except near Station 510. This fill is made largely from residual soil without

an admixture of refuse or organic materials and it is anticipated that the walls may be placed on spread footings on the fill at bearing intensities not to exceed about 1-1/2 tons per sq. ft. Specifications should provide for undercut below nominal subgrade level, replacement with select granular material compacted in place, and proof-rolling of the entire subgrade to eliminate erratic soft spots beneath the bearing surface. At the south end of these walls, near Boring No. AM-26 weathered rock will be reached at subgrade and bearing pressures could be taken conservatively at 3 tsf.

5.3 Stations 510 to 502+50, Cut-and-Cover Boxes

The base of these structures will be in weathered hornblende schistose gneiss on the north grading to jointed and slightly weathered quartz-diorite gneiss near Station 504. The boxes could be designed with wall footings at bearing capacities from about 5 tsf on the north to 15 tsf on the south with a pressure-relieved slab between footings. It is anticipated that cofferdam soldier piles would have to extend to depths of typically 4 feet below subgrade at the north end of the box to 2 feet below subgrade at the south. Boring No. AM-22 suggests the sort of difficulty that will be encountered in pre-coring soldier pile holes. Bedrock exposed behind the soldier piles is mixed in quality between country rock and amphibolite and the characteristic foliation and principal jointing pattern will strike roughly parallel to the cofferdam walls and dip steeply toward the west so that the east face will be distinctly less stable. Bidders should be warned that cofferdam design must consider pressures generated by failure on westward dipping planes of weakness.

5.4 Stations 502+50 to 450, Single-Track Rock Tunnels

The portal commences in moderately to highly weathered rock at a depth of 45 feet below the surface and 22 feet below the water table. The portal design must assume that full overburden pressures are applied to the tunnel tops. If support by the intermediate pillar is disregarded, as has been suggested by the Section Designer, it probably would be satisfactory to accept 25 or 30 per cent elevation of stresses beyond normal working values. Between Borings Nos. AM-20 and AM-45U, near the location of Bellevue Drive Fan Shaft, a culvert crosses in an old drainage channel where the surface of residual soil is at the lowest level in the area. Apparently the presence of this channel reflects a significant weakness in the bedrock evidenced by a band of chlorite schist near the base of running tunnel. This body appears in all of the west-side borings from No. AM-21 through AM-19 and does not appear in No. AM-45U east of the line. It may be presumed that this body follows the foliation strike and dip and passes through the tunnels low on the west and high on the east. Its presence makes particularly suspect the condition of the intermediate pillar and the rock

which will be exposed on the portal face.

Near Station 498 tunneling conditions are expected to improve toward the south and continue in relatively sound cover to about Station 478. At this point the tunnel passes beneath another old drainage channel marked by a shallow topographic low which now contains a box culvert draining to Rock Creek. While there is no abrupt change in the bedrock character the rock generally evidences a higher percentage of hornblende between Stations 478 and 469, then grading to diorite and meta-diorite that dominates at Medical Center Station.

Because the recent AM-series borings showed consistently poorer quality rock than borings of the A-series in this area, a special review was made of cores of both sets of borings that resulted in some relatively minor alterations in assigned RQD values of borings of the A-series between Nos. A-89 and A-92. The most striking contrast is between Nos. AM-15 and A-90 where AM-15 was advanced by rotary drilling methods with a driven spoon sampler to Elev. 180 before rock coring commenced. It was concluded that this procedure had broken through material which should have been categorized as weathered bedrock. A check boring, AM-74, was made in December 1974 within several feet of the questionable Boring No. AM-15. This appeared to confirm the supposition that weathered rock in AM-15 had been broken by rotary drilling. In any case weathered and jointed rock extends almost to the position of top of rail at AM-15 and AM-74.

In the entire length between Stations 477 and 468 the roof rock appears moderately to highly weathered. Deep weathering may be associated with a shear zone that could connect Borings Nos. AM-14, AM-15, AM-74 and A-90, striking at a flat angle east of north. Extremely unfavorable conditions are evidenced in Boring No. AM-14 at a level just beneath the tunnel. This could connect with the deep rotary drilling in AM-15, intersecting the tunnels between these borings on a steep dip to the west. Unfavorable cover at AM-17 might be associated with this shear zone although it is not in evidence in Boring No. A-91.

Continuing south of Station 468 bedrock grades to quartz-diorite gneiss or meta-diorite with lesser amphibolites. Rock cover generally ranges from jointed to relatively sound except broken and possibly sheared zones develop between Borings Nos. AM-36 and AM-40. The position of the more prominent shears are noted on Drawing No. F-A-94. These generally exhibit the characteristic steep dip associated with the foliation and thus presumably strike on a flat angle west of north. This attitude and the boring arrangement makes a considerable distinction between east side and west side conditions so that two profiles have been drawn for the top of relatively sound rock on either side of Wisconsin Avenue. Although rock is unweathered at the top of tunnel there probably will be a pattern of shears crossing the tunnels between AM-36 and AM-38. Badly broken

rock should appear in the tunnels west of AM-41 and A-87 that may be part of this movement pattern. While there is little evidence of weathering of the mineral fabric markedly low compressive strengths are obtained in cores comparatively high in hornblende and chlorite within this disturbed area.

#### 5.5 Medical Center Station

Conditions at Medical Center Station are portrayed on two longitudinal sections, for west side borings in Drawing No. F-A-95A and for the east side on Drawing No. F-A-95B. Four natural scale sections across the long axis of the station looking north are presented on Drawings Nos. F-A-100 and -101. A plan of principal features on a horizontal plane at Elev. 200 is shown in the upper panel of Drawing No. F-A-104. The interpretation of Medical Center conditions was assisted by R.A. Robinson, formerly of the University of Illinois field staff, who undertook a review of cores with our project geologist, V.V. Tepordei, in an effort to apply techniques for correlation of cores with field evidence which were developed at Dupont Circle Station. Rock at Medical Center Station consists chiefly of hard and brittle quartz-hornblende-biotite diorite and meta-diorite indistinctly foliated. Penetration of weathering from the surface is relatively shallow and the cover is unusually thick of rock unweathered except for staining and alteration of joints. Compression strengths are distinctly higher than average. Because the igneous origin of the diorite was not encountered in previous exploration, samples for drillability, thin section petrographic analysis and joint shear tests were concentrated to some extent in this material. Nevertheless the brittle diorite is at places highly jointed and blocky, as evidenced on Section B-B, and rock cover quality over a portion of the northern half of the station is distinctly unfavorable. Because of the appearance of this highly jointed zone, a series of four supplementary borings were requested to be drilled in the median strip of Wisconsin Avenue to document conditions directly above the tunnel.

Evidence of shear in the diorite is scattered and discontinuous, only one shear zone being correlated through several borings with characteristic strike and dip. This is shown west of the opening in the plan of Drawing No. F-A-104. Robinson notes that it is characteristic of tunnels now opened and exposures along Rock Creek that the rock is blockier where shear zones are the least important and that large well defined blocks bounded by planar, smooth joints become more common. The predominate lithology through the station is a light to dark color, fine to medium grained diorite varying in composition. The range of mineral content is illustrated by the thin section reports in Part 1 of the drillability report. The only distinct lithologic variations are the xenoliths of schistose



gneiss of the Wissahickon country rock plus a granodiorite dike crossing the station apparently on a line that intersects Borings Nos. AM-64, -65 and -70. The diorite is divided into a lighter colored zone relatively high in quartz on the south and a darker zone high in hornblende on the north with a complex contact near the center of the station and a mixed zone crossing from east to west. The mineral composition, texture and lithologic arrangement all suggest an igneous intrusive origin and while the contacts appear to be of no great structural significance the rock quality and the degree of jointing may be a function of this general pattern. The intensely broken material at the north third point of this station may be related to stresses produced by the intrusion of the granodiorite dike. At least five distinct sets of joints could be seen in the cores, three of these steeply dipping and corresponding to the principal sets of Table No. 6 and two near horizontal sets.

Nesbitt in his comments on the borehole photography makes particular note of potential difficulties from the horizontal jointing pattern in Borings Nos. AM-68 and AM-70. Photography in No. AM-70 exhibited a primary joint set striking  $N55^{\circ}$  to  $85^{\circ}W$  with a  $65^{\circ}$  to  $75^{\circ}NE$  dip and this corresponds to sets 3 or 3-4 distinguished in Section A004. It may be postulated that Medical Center Station will provide problems in the excavation somewhat similar to blocky quartz-diorite gneiss in the northern stations of A006.

#### 5.6 Stations 442 to 396, Single-Track Rock Tunnels

Conditions are depicted on Drawings Nos. F-A-96, -97 and -98. Throughout this length the cover rock directly above the opening is essentially unweathered except for alteration and staining on joint planes and a substantial proportion is jointed to relatively sound with typical RQD values in the range of 70 to 85. The bedrock is predominately quartz-diorite gneiss except that it grades generally toward meta-diorite at the north at Medical Center Station and it includes bands and zones of schistose gneiss of the country rock which in places are high enough in hornblende and chlorite to be categorized with the amphibolite group. Relatively unfavorable conditions first appear in a band between Stations 430 and 435 where moderate jointing in borings east of Wisconsin Avenue reaches the tunnel opening. This condition does not appear in Boring No. AM-30 directly opposite on the west side and it seems likely that this jointed zone includes chiefly steeply west dipping planes. A highly broken zone directly above the tunnel top appears in Boring No. AM-42 which may mark a boundary between quartz-diorite gneiss on the south and materials high in hornblende in the jointed zone immediately north.

South of A-73 cover rock continues in hard, brittle pre-dominately quartz-diorite gneiss to Station 410 at which point materials relatively high in hornblende begin to appear above the tunnel top commencing in the space between Borings Nos. A-68 and A-69. In the reach between Borings Nos. AM-10 on the east and AM-11 on the west zones of badly broken or highly jointed material with a comparatively high proportion of chlorite schist appear with evidence of shear on the characteristic strike and dip of the primary joint system. The shears appear to pass off to the west north of AM-12. While evidence of shear in this location is not as strong as in several less favorable sections north of Medical Center Station the lithology is highly variable and the rock quality here is the poorest in the length between the two subway stations.

#### 5.7 Bethesda Station

Conditions at Bethesda Station are portrayed on two longitudinal sections, for west side borings in Drawing No. F-A-99A and on the east side for Drawing No. F-A-99B. Four natural scale cross-sections perpendicular to the long axis of the station and looking north are presented on Drawings Nos. F-A-102 and -103. A plan view of principal features evidenced on a horizontal plane at Elev. 220 is shown on the lower panel of Drawing No. F-A-104. Rock at Bethesda Station consists of two of the principal classes of material in Section A011, quartz-diorite gneiss on the south and east and a band of hornblende schist and gneiss with chlorite inclusions crossing above the northwest corner of the station on a near north-south strike with a contact deeply dipping to the west at angles between  $60^{\circ}$  and  $75^{\circ}$ . The presence of this softer rock in the northwest quadrant of the station appears to have produced substantially deeper weathering in this quadrant and inferior conditions at the locations of the north vent shaft and the station entrance. In addition to deep weathering in this corner, weathered rock approaches as near as about 10 feet to the top of the station opening on the line between Nos. AM-3U and A-63U. In other locations the cover rock provides 10 or 15 feet of relatively sound material with RQD values above 75.

The borehole photography record indicates primary and secondary joint sets corresponding closely to the two identified in tunnels on lower Connecticut Avenue. Several of the particular features pointed out in Nesbitt's commentary have been identified on the cross-sections. In contrast to Medical Center Station, the foliation is pronounced and on the typical regional attitude, conforming to the primary joint set. A fairly systematic group of broken zones with some slick joints and evidence of gouge is noted at similar elevations on the east side borings, generally following the foliation. One zone in particular indicates continuity that

intersects the entire station low on the east wall and may continue past AM-10 and AM-11. This is plotted in the plan of Drawing No. F-A-104.

We judge that conditions at Bethesda Station may be somewhat superior to those at Dupont Circle Station because the total depth of rock is substantially greater. However the minimum cover of unweathered rock is roughly of the order of that encountered at Dupont Circle Station. In places rock bolt anchorage will fall in moderately weathered materials and there undoubtedly will be special problems with roof support where weathering approaches close to the opening.

#### 5.8 Ancillary Structures

Ancillary structures include seven vent and fan shafts which have been identified in Table No. 7. Each shaft will be carried by braced excavation through the complete profile of weathering and therefore an effort has been made to interpret the elevation of the base of residual soil and the top of weathered bedrock which have been summarized in Table No. 7. In general we expect the following performance characteristics in the rock zones to be common to each one of these shaft locations:

1. A compact H-pile can be driven deeply into the residual soil by a high energy hammer but near the base of the residual soil layer it could meet practical refusal. It certainly cannot be driven a substantial distance into the transition zone and should not be expected to reach the top of weathered bedrock by impact driving. To pre-core a hole below the depth of refusal would doubtless require churn drills or other percussion or pneumatic drilling equipment and could not be accomplished with the ordinary caisson auger for overburden.

2. Depending on the size and location of the shaft it may be necessary to require that soldier piles be carried through the transition zone to bottom in weathered bedrock. Whatever requirement is set up on this point should be made absolutely clear to the bidders or it will doubtless be a subject of later misunderstandings.

3. The decomposed and weathered rock in the transition zone very likely would need close support by lagging or ring beams between soldier piles. Rock bolting in this transition zone would be an uncertain and unpredictable means of stabilizing the wall but might be effective in conjunction with shotcrete or gunite and wire mesh. The same may be said of the upper part of the weathered bedrock which could be equally unreliable for support by rock bolts alone. Specifications should make perfectly clear to bidders what loading assumptions are to be made in analyzing stability of the cofferdam.

If soldier piles are not carried to the base of shaft the potential failure of wedges of rock with unfavorable dip should be evaluated including an allowance for the effect of vertical loads transmitted to such a wedge by the soldier piles.

4. While the lines dividing the different bedrock zones on the geological sections may suggest well defined boundaries between materials, the profile of weathering represents gradual and not entirely consistent changes with depth and no abrupt or fixed dividing lines separating materials with different performance characteristics should be envisioned.

TABLE NO. 1, SURVEY DATA FOR BORINGS

Sheet No. 1

BORING NUMBER	HORIZONTAL CONTROL COORDINATES:		VERTICAL CONTROL: GROUND SURFACE ELEVATION (FT)	STATUS OF OBSERVATION WELLS		
	EAST	NORTH		INSTALLED	TYPE	PRESENT CONDITION
AM-1	E 773,461	N 418,560	+350.3			
AM-2	E 773,333	N 418,595	+346.9			
AM-3U	E 773,410	N 418,707	+347.9			
AM-4	E 773,280	N 418,736	+347.6			
AM-5	E 773,351	N 418,888	+346.8			
AM-6U	E 773,193	N 418,908	+344.2			
AM-7	E 773,275	N 419,122	+348.3			
AM-8	E 773,161	N 419,134	+348.3	2-22-74	1-1/2"	F
AM-9U	E 773,068	N 419,332	+350.6			
AM-10	E 773,148	N 419,752	+355.5			
AM-11	E 772,998	N 419,890	+352.7			
AM-12	E 772,930	N 420,259	+351.1			
AM-13	E 772,383	N 426,070	+266.1			
AM-14	E 772,225	N 426,260	+265.2			
AM-15	E 772,346	N 426,511	+260.6			
AM-16	E 772,195	N 426,716	+251.5			
AM-17	E 772,311	N 427,099	+234.0			
AM-18	E 772,188	N 428,766	+283.6			
AM-18A	E 772,049	N 429,152	+253.4			
AM-19	E 771,977	N 429,303	+243.0			
AM-20	E 771,907	N 429,505	+242.3			
AM-21	E 771,827	N 429,653	+247.2			
AM-22	E 771,753	N 429,835	+251.6			
AM-23	E 771,718	N 429,921	+247.4			
AM-24	E 771,655	N 430,068	+241.6			
AM-25	E 771,585	N 430,213	+237.7			

TABLE NO. 1, SURVEY DATA FOR BORINGS

Sheet No. 2

BORING NUMBER	HORIZONTAL CONTROL COORDINATES:		VERTICAL CONTROL: GROUND SURFACE ELEVATION (FT)	STATUS OF OBSERVATION WELLS		
	EAST	NORTH		INSTALLED	TYPE	PRESENT CONDITION
AM-26	E 771,593	N 430,415	+235.4			
AM-27	E 771,510	N 430,573	+234.1			
AM-28	E 771,395	N 430,770	+230.7			
AM-29	E 771,308	N 430,890	+235.2			
AM-30	E 772,498	N 422,888	+310.4			
AM-34	E 772,574	N 423,630	+312.3			
AM-35	E 772,406	N 423,962	+322.4			
AM-36	E 772,476	N 424,753	+320.5	11-6-73	1-1/2"	F
AM-37U	E 772,458	N 425,080	+311.6			
AM-38	E 772,304	N 425,239	+313.6			
AM-40	E 772,278	N 425,579	+290.4			
AM-41U	E 772,402	N 425,773	+274.0	3-19-74	1-1/2"	F
AM-42	E 772,685	N 422,537	+329.0			
AM-43U	E 772,305	N 427,624	+250.7			
AM-44U	E 772,164	N 428,097	+285.7	6-12-74	1-1/2"	F
AM-45U	E 772,083	N 429,421	+237.5			
AM-46U	E 772,451	N 426,177	+264.9			
AM-47	E 772,327	N 424,607	+318.8			
AM-48	E 773,256	N 418,472	+343.6			
AM-58U	E 772,603	N 423,868	+328.0			
AM-59U	E 772,858	N 421,490	+353.2			
AM-60	E 773,053	N 419,166	+346.0			
AM-61	E 772,551	N 423,854	+320.8			
AM-62	E 772,520	N 424,143	+322.4			
AM-63	E 772,395	N 424,225	+320.3			
AM-64	E 772,515	N 424,298	+321.2			
AM-65	E 772,382	N 424,375	+321.6	2-5-74	1-1/2"	

TABLE NO. 1, SURVEY DATA FOR BORINGS

Sheet No. 3

BORING NUMBER	HORIZONTAL CONTROL COORDINATES:		VERTICAL CONTROL: GROUND SURFACE ELEVATION (FT)	STATUS OF OBSERVATION WELLS		
	EAST	NORTH		INSTALLED	TYPE	PRESENT CONDITION
AM-66	E 772,505	N 424,461	+320.7			
AM-67	E 772,345	N 424,745	+325.7			
AM-68	E 772,482	N 423,967	+321.5			
AM-69	E 772,455	N 424,183	+322.8			
AM-70	E 772,443	N 424,337	+323.5			
AM-71	E 772,429	N 424,489	+323.5			
AM-74	E 772,341	N 426,500	+259.0			
<p>NOTE:</p> <p>Borings Nos. AM-72-73 and -75 to -79 were in progress in December 1974 and are not included in this report.</p> <p>Legend: F = still functioning in 1974</p>						

**TABLE NO.2**  
**SUMMARY OF LABORATORY TEST DATA**

SAMPLE IDENTIFICATION				CLASSIFICATION PROPERTIES					PHYSICAL PROPERTIES																		
									STRENGTH					CONSOLIDATION													
BORING NO.	SAMPLE NO.	DEPTH FT.	STRATUM DESIGNATION	NATURAL WATER CONTENT % (W) AVERAGE OF ENTIRE SAMPLE	LIQUID LIMIT (WL)	PLASTICITY INDEX (I <sub>p</sub> )	NATURAL WATER CONTENT OF LIMIT SAMPLE % (W)	SPECIFIC GRAVITY OF SOLIDS (G)	UNIFIED CLASSIFICATION SYSTEM			UNCONFINED COMPRESSION			TRIAXIAL COMPRESSION				NATURAL WATER CONTENT %	EXISTING OVERBURDEN STRESS TSF	ESTIMATED PROBABLE PRECONSOLIDATION STRESS - TSF	COMPRESSION INDEX C <sub>c</sub>	SWELLING INDEX C <sub>s</sub>	VOID RATIO AT START OF SWELL, e <sub>r</sub>			
									SOIL TYPE	% SAND (<#4, >#200 SIEVE)	% FINES (<#200 SIEVE)	COMPRESSIVE STRENGTH TSF	WATER CONTENT AT END OF TEST %	STRAIN AT FAILURE %	TYPE OF TEST	DEVIATOR STRESS (σ <sub>1</sub> - σ <sub>3</sub> ) TSF	CONFINING PRESSURE (σ <sub>3</sub> ) TSF	NATURAL WATER CONTENT %							WATER CONTENT AT END OF TEST %		
AM-3U	2U	3.6	D	15					SM				Q	1.36	0.5	15	15										
	4U	8.2	D	10					SM	72	28	Direct Shear Test φ = 31.8° C = 0.1 TSF	Q	1.51	0.5	10	10										
	6U	13.7	D	20					SM				Q	2.45	1.0	10	9										
	8U	19.0	D	21					SM	74	26		Q	3.88	2.0	10	10										
	10U	24.0	D	26					SM			0.52	32	4	Q	1.63	0.5	21	21								
															Q	3.16	2.0	-	-								
AM-6U	2U	3.2	F	29	48	16	29		CL&ML			0.90	23	4	Q	1.81	1.0	30	30								
	3U	5.5	F	27					ML			1.53	25	5	Q	2.01	0.5	29	29								
	4U	7.8	D	22					SM						Q	1.65	0.5	21	21								
	6U	11.8	D	24	35	4	24		SM			Direct Shear Test φ = 24° C = 0.45 TSF	Q	1.87	1.0	24	23										
	7U	14.2	D	17					SM	72	28		Q	2.90	2.0	23	22										
	9U	18.1	D	25					SM						Q	2.05	1.0	24	23								
	10U	20.5	D	26					SM						Q	3.04	2.0	24	23								
															Q	1.69	0.5	17	17								
															Q	2.22	0.5	16	15								
															Q	2.00	1.0	22	22								
														Q	2.81	2.0	23	22									
AM-9U	1U	1.5	D	19					SM						Q	1.26	0.5	23	24								
	2U	3.6	D	12					SM	72	28	Direct Shear Test φ = 37° C = 0.3 TSF	Q	2.68	2.0	20	19										
															Q	1.44	0.5	12	12								
														Q	2.60	1.0	12	11									
AM-25U	2U	6.0	D	10					SM						Q	2.57	1.0	8	-								
AM-37U	2U	4.0	D	15					ML						Q	1.88	0.5	17	16								
														Q	4.63	2.0	19	18									

MUESER, RUTLEDGE, WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
415 MADISON AVENUE  
NEW YORK, N.Y. 10017

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
DE LEUW CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANTS



**TABLE NO. 2**  
**SUMMARY OF LABORATORY TEST DATA**

SAMPLE IDENTIFICATION				CLASSIFICATION PROPERTIES							PHYSICAL PROPERTIES															
BORING NO.	SAMPLE NO.	DEPTH FT.	STRATUM DESIGNATION	NATURAL WATER CONTENT % (W) AVERAGE OF ENTIRE SAMPLE	LIQUID LIMIT (W <sub>L</sub> )	PLASTICITY INDEX (I <sub>p</sub> )	NATURAL WATER CONTENT OF LIMIT SAMPLE % (W)	SPECIFIC GRAVITY OF SOLIDS (G)	UNIFIED CLASSIFICATION SYSTEM		UNCONFINED COMPRESSION			TRIAXIAL COMPRESSION				CONSOLIDATION								
									SOIL TYPE	% SAND (<#4, >#200 SIEVE)	% FINES (<#200 SIEVE)	COMPRESSIVE STRENGTH TSF	WATER CONTENT AT END OF TEST %	STRAIN AT FAILURE %	TYPE OF TEST	DEVIATOR STRESS (σ <sub>1</sub> - σ <sub>3</sub> ) TSF	CONFINING PRESSURE (σ <sub>3</sub> ) TSF	NATURAL WATER CONTENT %	WATER CONTENT AT END OF TEST %	NATURAL WATER CONTENT %	EXISTING OVERBURDEN STRESS TSF	ESTIMATED PROBABLE PRECONSOLIDATION STRESS - TSF	COMPRESSION INDEX C <sub>c</sub>	SWELLING INDEX C <sub>s</sub>	VOID RATIO AT START OF SWELL, e <sub>r</sub>	
AM-41U	2U	3.0	D	23					SM				Q	2.38	0.5	22	25									
	3U	6.0	D	24					SM				Q	3.10	1.0	22	22									
													Q	2.71	2.0	27	27									
AM-44U	2U	3.0	F	29					ML				Q	1.88	1.0	29	29									
	4U	9.0	D	27					ML	41	59	Direct Shear Test φ = 26° C = 0.3 TSF	Q	2.64	2.0	29	29									
	6U	16.0	D	38					ML				Q	2.49	0.5	25	25									
												Q	1.82	0.5	29	28										
												Q	1.13	1.0	40	40										
												Q	2.21	2.0	35	35										
AM-45U	4U	15.8	D	26					ML			0.75	26	5												
AM-46U	2U	1.6	F	18					CL				Q	3.32	0.5	20	20									
													Q	3.35	1.0	17	17									
													Q	3.91	2.0	16	16									
AM-58U	4U	13.0	D	18					SM	72	28	Direct Shear Test φ = 36° C = 0	Q	2.84	0.5	17	17									
	5U	16.0	D	15					SM	74	26		Q	2.55	1.0	18	18									
													Q	4.22	2.0	15	15									
AM-59U	2U	4.0	D	12					SM & ML	58	41	Direct Shear Test φ = 27° C = 0.2 TSF	Q	3.75	2.0	12	12									
	4U	11.0	D	19					SM				Q	1.64	0.5	18	18									
													Q	3.16	1.0	20	20									

**NOTES**

- ALL TESTS SUMMARIZED ABOVE WERE PERFORMED IN THE SOILS LABORATORY OF MUESER, RUTLEDGE, WENTWORTH & JOHNSTON.
- THE SAMPLE DEPTH LISTED ABOVE IS THE AVERAGE DEPTH OF THE SAMPLE RECOVERED.
- FOR GROUND SURFACE ELEVATIONS AT THE BORINGS SEE TABLE NO. 1. FOR GENERALIZED STRATA DESCRIPTIONS SEE DRAWING NO. F-1.
- "NATURAL WATER CONTENT OF ENTIRE SAMPLE" IS A WEIGHTED AVERAGE OF ALL MATERIAL TYPES RECOVERED.
- THE TRIAXIAL COMPRESSION TESTS PERFORMED WERE:  
Q - QUICK TESTS (UU - UNCONSOLIDATED UNDRAINED TESTS)  
Q<sub>c</sub> - CONSOLIDATED QUICK TESTS (CU - CONSOLIDATED UNDRAINED TESTS)
- STRENGTH TESTS WERE PERFORMED ON PISTON TYPE SAMPLES (U) APPROXIMATELY 2.9 INCHES IN DIAMETER AND ON SHELBY TYPE SAMPLES (S) APPROXIMATELY 1.8 INCHES IN DIAMETER. THE RATIO OF HEIGHT TO DIAMETER OF ALL STRENGTH TEST SPECIMENS WAS APPROXIMATELY 2.0.
- THE TRIAXIAL COMPRESSION TESTS WERE PERFORMED AT A RATE OF STRAIN OF APPROXIMATELY 1 PER CENT PER MINUTE.
- THE DIRECT SHEAR TESTS WERE PERFORMED AT A CONSTANT RATE OF STRAIN EQUAL TO A HORIZONTAL DISPLACEMENT OF 0.02 INCHES PER HOUR. THE SPECIMENS WERE OF APPROXIMATELY 1/2 INCH THICKNESS.
- COMPRESSION INDEX C<sub>c</sub> - STRAIGHT LINE PORTION OF THE VIRGIN CURVE OF CONSOLIDATION TEST: e = e<sub>0</sub> - C<sub>c</sub> LOG P/P<sub>0</sub>
- SWELLING INDEX C<sub>s</sub> - STRAIGHT LINE PORTION OF THE REBOUND CURVE OF CONSOLIDATION TEST: e = e<sub>0</sub> - C<sub>s</sub> LOG P/P<sub>0</sub>

MUESER, RUTLEDGE, WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
416 MADISON AVENUE  
NEW YORK, N.Y. 10017

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
DE LEUW CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANTS

TABLE No. 3, SUMMARY OF FALLING HEAD PERMEABILITY TESTS IN BORE HOLES AND OBSERVATION WELLS

(1)

Boring No.	AM-2	AM-4	AM-5	AM-7	AM-8	AM-9U	AM-10	AM-11	AM-12	AM-12	AM-13	AM-13	AM-13	AM-14	AM-16
Elevation of top and bottom of opening tested	+301.1 +185.9	+302.6 +186.3	+311.8 +207.3	+303.3 +191.3	+288.3 +188.3	+289.6 +190.6	+345.5 +185.5	+312.7 +181.7	+245.1 +211.6	+290.1 +191.1	+241.1 +186.5	+225.1 +159.2	+241.1 +219.3	+210.2 +149.2	+196.5 +119.0
Length of opening, feet	115.2	116.3	104.5	112	100	99	160	131	133.5	99	54.6	45	21.8	36.4	77.5
Computed permeability, feet/minute	$6.7 \times 10^{-4}$	$4.5 \times 10^{-4}$	$1.3 \times 10^{-4}$	$2.5 \times 10^{-4}$	$1.5 \times 10^{-4}$	$3.9 \times 10^{-4}$	$1.3 \times 10^{-4}$	$2.4 \times 10^{-4}$	$1.1 \times 10^{-4}$	$1.9 \times 10^{-4}$	$> 1 \times 10^{-2}$	$2 \times 10^{-3}$	$> 1 \times 10^{-2}$	$4.5 \times 10^{-4}$	$5.1 \times 10^{-4}$
Stratum tested	WR, J & R	WR J & R	WR, J & R	WR, J & R	WR, J & R	WR, J & R	D, WR J & R	D, WR J & R	J & R	WR, J & R	D, WR & J	WR & J	D & WR	WR, J & R	WR, J & R

Boring No.	AM-17	AM-18	AM-18A	AM-20	AM-21	AM-21	AM-21	AM-21	AM-22	AM-23	AM-23	AM-24	AM-26	AM-30	AM-34
Elevation of top and bottom of opening tested	+186.0 +149.6	+242.6 +235.1	+198.4 +153.4	+204.3 +152.3	+235.4 +207.7	+228.2 +185.7	+228.2 +181.2	+228.2 +167.2	+242.6 +181.6	+227.4 +212.4	+227.4 +177.4	+221.6 +177.9	+235.4 +196.9	+300.4 +140.4	+287.3 +147.3
Length of opening, feet	36.4	7.5	45	52	11.8	42.5	47	61	61	15	50	43.7	38.5	160	140
Computed permeability, feet/minute	$4.5 \times 10^{-4}$	$1.0 \times 10^{-3}$	$2.7 \times 10^{-3}$	$3.9 \times 10^{-4}$	$6.4 \times 10^{-5}$	$3.9 \times 10^{-4}$	$3.3 \times 10^{-4}$	$9.6 \times 10^{-5}$	$8.6 \times 10^{-5}$	$2.2 \times 10^{-3}$	$1.0 \times 10^{-3}$	$2.1 \times 10^{-3}$	$7.1 \times 10^{-5}$	$1.6 \times 10^{-4}$	$3.3 \times 10^{-5}$
Stratum tested	WR, J & R	D & WR	WR, J & R	WR & J	D & WR	D & WR	D, WR	D, WR & R	D, WR & J	D, WR	D, WR & J	D, WR & J	FD & WR	D, WR & R	WR, J & R

TABLE No. 3, SUMMARY OF FALLING HEAD PERMEABILITY TESTS IN BORE HOLES AND OBSERVATION WELLS

Boring No.	AM-34	AM-42	AM-42	AM-43	AM-44U	AM-44U Observ. Well	AM-45U	AM-46U	AM-47	AM-58U	AM-59U	AM-60	AM-61	AM-61	AM-62
Elevation of top and bottom of opening tested	+287.3 +133.8	+314.0 +289.0	+289.0 +232.0	+209.7 +121.3	+235.7 +125.7	+164.2 +150.7	+199.5 +143.6	+213.9 +165.3	+278.3 +159.8	+283.0 +188.0	+296.2 +185.6	+247.0 +181.0	+265.8 +176.8	+265.8 +168.4	+277.4 +173.8
Length of opening, feet	153.5	25	57	88.4	110	13.5	55.9	48.6	118.5	95	110.6	66	89	97.4	103.6
Computed permeability, feet/minute	$3.5 \times 10^{-5}$	$2.7 \times 10^{-4}$	$1.3 \times 10^{-4}$	$4.3 \times 10^{-5}$	$2.5 \times 10^{-5}$	$1.4 \times 10^{-4}$	$3.2 \times 10^{-4}$	$> 1 \times 10^{-2}$	$1.4 \times 10^{-4}$	$3.0 \times 10^{-4}$	$3.0 \times 10^{-4}$	$4.1 \times 10^{-5}$	$7.6 \times 10^{-5}$	$9.7 \times 10^{-5}$	$1.3 \times 10^{-4}$
Stratum tested	WR, J & R	D, WR	WR, J & R	WR, J & R	D, WR & R	D, WR & R	WR, J & R	WR, J & R	J, R	WR, J & R	J, R	J, R	R	R	J & R

-4

Boring No.	AM-62	AM-63	AM-63	AM-64	AM-65	AM-65	AM-66	AM-66	AM-67	AM-68	AM-69	AM-70	AM-71		
Elevation of top and bottom of opening tested	+277.4 +137.4	+280.3 +190.8	+280.3 +135.3	+301.2 +173.1	+297.6 +170.6	+297.6 +148.6	+300.7 +180.7	+300.7 +128.7	+291.8 +186.3	+279.5 +131.5	+288.3 +134.3	+300.5 +133.9	+280.5 +134.0		
Length of opening, feet	140	89.5	145	128.1	127	149	120	172	105.5	148	154	166.6	146.5		
Computed permeability, feet/minute	$1.2 \times 10^{-4}$	$3.2 \times 10^{-4}$	$2.4 \times 10^{-4}$	$1.0 \times 10^{-4}$	$2.6 \times 10^{-5}$	$1.8 \times 10^{-5}$	$3.5 \times 10^{-5}$	$8.6 \times 10^{-5}$	$1.9 \times 10^{-4}$	$1.4 \times 10^{-4}$	$1.5 \times 10^{-4}$	$4.0 \times 10^{-4}$	$1.4 \times 10^{-4}$		
Stratum tested	J & R	D, WR, J & R	D, WR, J & R	D, WR, J & R	D, WR & J	D, WR J & R	D, WR J & R	D, WR J & R	D, WR & R	WR, J & R	WR, J & R	D, WR J & R	WR, J & R		

Table No. 4 - Summary of Water Pressure Tests in Rock

Sheet No. 1

Boring No.	AM-1	AM-1	AM-1	AM-1	AM-4	AM-4	AM-4	AM-8
Test No.	1	2	3	4	1A	1	2	1
Elev. Bott. of Top Packer	200.3	210.3	220.3	238.3	234.6	202.6	226.6	208.3
Elev. Top of Bott. Packer	195.3	205.3	215.3	233.3	229.6	197.6	221.6	203.3
Maximum Pressure Gage Reading, PSI	60	40	85	45	90	90	40	60
Total Net Head on Opening, Ft. of Water	139	92	196	104	208	208	92	139
Average Flow G. P. M.	1.72	3.74	2.47	4.49	2.47	0.75	8.23	0
Computed Permeability Ft. /Min.	$1.3 \times 10^{-4}$	$3.9 \times 10^{-4}$	$1.4 \times 10^{-4}$	$4.2 \times 10^{-4}$	$2.0 \times 10^{-4}$	$4.1 \times 10^{-5}$	$8.7 \times 10^{-5}$	Very Low $< 1 \times 10^{-7}$
Average RQD Value of Surrounding Rock	80	82	86	85	85	95	0	95

Table No. 4 - Summary of Water Pressure Tests in Rock

Boring No.	AM-8	AM-8	AM-8	AM-18	AM-18	AM-19	AM-19	AM-19
Test No.	2	3	4	3	4	1	2	3
Elev. Bott. of Top Packer	213.3	218.3	223.3	212.6	193.6	169.5	174.5	179.5
Elev. Top of Bott. Packer	208.3	213.3	218.3	207.6	188.6	164.5	169.5	174.5
Maximum Pressure Gage Reading, PSI	75	75	60	60	40	90	30	90
Total Net Head on Opening, Ft. of Water	173	173	139	139	92	208	68	208
Average Flow G.P.M.	0	0	0	1.50	0.07	13.46	22.44	17.20
Computed Permeability Ft./Min.	Very Low $< 1 \times 10^{-7}$	Very Low $< 1 \times 10^{-7}$	Very Low $< 1 \times 10^{-7}$	$1.0 \times 10^{-4}$	$6.6 \times 10^{-6}$	$7.5 \times 10^{-4}$	$3.2 \times 10^{-3}$	$9.6 \times 10^{-4}$
Average RQD Value of Surrounding Rock	91	95	85	23	59	32	50	55

Table No. 4 - Summary of Water Pressure Tests in Rock

Sheet No. 3

Boring No.	AM-19	AM-20	AM-20	AM-20	AM-20	AM-35	AM-35	AM-35
Test No.	4	1	2	3	4	1	2	3
Elev. Bott. of Top Packer	184.5	172.3	182.3	192.3	202.3	158.4	163.4	168.4
Elev. Top of Bott. Packer	179.5	167.3	177.3	187.3	197.3	153.4	158.4	163.4
Maximum Pressure Gage Reading, PSI	90	66	50	46	45	90	90	85
Total Net Head on Opening, Ft. of Water	208	152	115	106	104	208	208	196
Average Flow G. P. M.	11.97	0.37	8.23	9.72	11.59	0	0.15	0
Computed Permeability Ft. /Min.	$6.7 \times 10^{-4}$	$2.7 \times 10^{-5}$	$7.6 \times 10^{-4}$	$9.6 \times 10^{-4}$	$1.2 \times 10^{-3}$	Very Low $< 1 \times 10^{-7}$	$8.4 \times 10^{-6}$	Very Low $< 1 \times 10^{-7}$
Average RQD Value of Surrounding Rock	35	45	74	80	20	98	95	92

Table No. 4 - Summary of Water Pressure Tests in Rock

Boring No.	AM-35	AM-36	AM-36	AM-36	AM-36	AM-38	AM-38	AM-38
Test No.	4	1	2	3	4	1	2	3
Elev. Bott. of Top Packer	173.4	146.5	151.5	161.5	166.5	140.4	145.4	150.4
Elev. Top of Bott. Packer	168.4	141.5	146.5	156.5	161.5	135.4	140.4	145.4
Maximum Pressure Gage Reading, PSI	85	80	80	30	90	90	90	80
Total Net Head on Opening, Ft. of Water	196	185	185	68	208	208	208	185
Average Flow G. P. M.	0	5.24	2.69	5.39	2.39	0.45	0.30	0.75
Computed Permeability Ft. /Min.	Very Low $<1 \times 10^{-7}$	$3.3 \times 10^{-4}$	$1.7 \times 10^{-4}$	$7.8 \times 10^{-4}$	$1.3 \times 10^{-4}$	$2.5 \times 10^{-5}$	$1.6 \times 10^{-5}$	$4.5 \times 10^{-5}$
Average RQD Value of Surrounding Rock	98	22	20	10	73	83	87	90

Table No. 4 - Summary of Water Pressure Tests in Rock

Sheet No 5

Boring No.	AM-38	AM-40	AM-40	AM-40	AM-41U	AM-41U	AM-41U	AM-41U
Test No.	4	1	2	3	1	2	3	4
Elev. Bott. of Top Packer	155.4	141.4	146.4	151.4	124.0	137.0	150.0	164.0
Elev. Top of Bott. Packer	150.4	136.4	141.4	146.4	119.0	132.0	145.0	159.0
Maximum Pressure Gage Reading, PSI	85	90	90	90	50	40	42	70
Total Net Head on Opening, Ft. of Water	139	208	208	208	116	92	97	162
Average Flow G.P.M.	0.15	0	0	0	0	0	0	0
Computed Permeability Ft./Min.	$1.2 \times 10^{-5}$	Very Low $< 1 \times 10^{-7}$	Very Low $< 1 \times 10^{-7}$	Very Low $< 1 \times 10^{-7}$	Very Low $< 1 \times 10^{-7}$	Very Low $< 1 \times 10^{-7}$	Very Low $< 1 \times 10^{-7}$	Very Low $< 1 \times 10^{-7}$
Average RQD Value of Surrounding Rock	95	65	89	88	39	28	35	50



Table No. 4 - Summary of Water Pressure Tests in Rock

Boring No.	AM-62	AM-62	AM-63	AM-63	AM-66	AM-66	AM-66	AM-67
Test No.	1	2	1	2	1	2	3	1
Elev. Bott. of Top Packer	174.9	182.9	158.9	171.9	157.4	164.4	182.4	162.3
Elev. Top of Bott. Packer	169.9	177.9	153.9	167.9	152.4	159.4	177.4	157.3
Maximum Pressure Gage Reading, PSI	80	80	80	80	90	90	90	90
Total Net Head on Opening, Ft. of Water	185	185	185	185	208	208	208	208
Average Flow G.P.M.	0	0	0.64	0	0	1.50	0	0.22
Computed Permeability Ft./Min.	Very Low -7 <1x10	Very Low -7 <1x10	4.0x10 <sup>-5</sup>	Very Low -7 <1x10	Very Low -7 <1x10	8.4x10 <sup>-5</sup>	Very Low -7 <1x10	1.2x10 <sup>-5</sup>
Average RQD Value of Surrounding Rock	60	66	45	90	75	63	60	94

Table No. 4 - Summary of Water Pressure Tests in Rock

Boring No.	AM-67	AM-68	AM-68	AM-70	AM-71			
Test No.	2	1	2	3	2			
Elev. Bott. of Top Packer	171.3	166.5	186.5	148.5	155.5			
Elev. Top of Bott. Packer	166.3	161.5	181.5	143.5	150.5			
Maximum Pressure Gage Reading, PSI	80	90	90	75	72			
Total Net Head on Opening, Ft. of Water	185	208	208	173	166			
Average Flow G. P. M.	0	0	0	1.57	0.28			
Computed Permeability Ft. /Min.	Very Low $<1 \times 10^{-7}$	Very Low $<1 \times 10^{-7}$	Very Low $<1 \times 10^{-7}$	$1.2 \times 10^{-4}$	$2.2 \times 10^{-5}$			
Average RQD Value of Surrounding Rock	92	100	100	33	89			

TABLE NO. 5 , SUMMARY OF UNCONFINED  
COMPRESSION STRENGTHS ON ROCK CORES

Sheet 1

Boring Number	Core Number	Sample Depth Feet	Peak Compressive Strength, PSI	R.O.D. Value of Core Run %
AM-1	6C	79.0	2,710	65
AM-1	9C	97.0	3,340	70
AM-1	14C	118.0	5,570	90
AM-1	16C	130.0	5,330	95
AM-3U	6C	71.0	3,820	40
AM-3U	9C	87.0	4,780	70
AM-3U	13C	102.0	5,100	75
AM-3U	18C	127.0	5,800	75
AM-4	7C	72.0	2,610	65
AM-4	8C	76.0	1,500	90
AM-4	12C	94.0	1,620	95
AM-6U	6C	70.0	2,550	65
AM-6U	11C	90.0	9,490	98
AM-6U	15C	109.0	5,480	65
AM-8	10C	78.0	860	50
AM-8	14C	96.0	1,270	90
AM-8	20C	123.0	4,590	75
AM-9U	21C	122.0	960	45
AM-9U	24C	136.0	7,040	90
AM-12	17C	121.0	5,890	80
AM-12	19C	129.0	5,100	70
AM-12	21C	139.0	5,480	60
AM-18	11C	85.0	7,040	75
AM-18	13C	94.0	3,820	70
AM-30	18C	94.0	9,240	80
AM-30	21C	106.0	8,700	80
AM-34	13C	77.0	9,300	90
AM-34	16C	88.0	13,020	85
AM-34	18C	92.0	13,180	45
AM-34	27C	133.0	10,190	90
AM-35	9C	74.0	17,200	90
AM-35	14C	92.0	11,150	55
AM-35	18C	110.0	7,170	85
AM-35	26C	143.0	10,250	85

TABLE NO. 5 , SUMMARY OF UNCONFINED  
COMPRESSION STRENGTHS ON ROCK CORES (CONT.)

Sheet 2

Boring Number	Core Number	Sample Depth Feet	Peak Compressive Strength, PSI	R.Q.D. Value of Core Run %
AM-36	16C	101.0	5,730	85
AM-36	20C	120.0	480	95
AM-38	21C	102.0	1,080	85
AM-38	24C	114.0	5,730	95
AM-38	26C	126.0	8,310	95
AM-40	18C	89.0	1,270	65
AM-40	19C	93.0	5,250	90
AM-43	13C	79.0	8,920	95
AM-43	17C	100.0	8,340	95
AM-60	9C	79.0	570	40
AM-60	16C	114.0	6,720	40
AM-61	24C	122.0	16,880	98
AM-62	15C	81.0	20,700	90
AM-62	23C	119.0	13,380	55
AM-63	25C	143.0	15,100	90
AM-64	11C	65.0	2,930	60
AM-64	14C	78.0	5,760	45
AM-64	21C	110.0	8,380	50
AM-65	28C	131.0	29,300	70
AM-65	39C	167.0	18,470	85
AM-66	13C	76.0	9,550	60
AM-66	19C	100.0	20,380	98
AM-66	22C	113.0	15,920	90
AM-66	29C	146.0	18,470	70
AM-68	14C	85.0	11,810	80
AM-68	21C	116.0	8,440	100
AM-68	26C	135.0	5,920	100
AM-69	12C	84.0	7,520	85
AM-69	20C	116.0	9,400	80
AM-69	26C	139.0	9,940	75
AM-70	19C	96.0	6,770	70
AM-70	28C	136.0	3,100	85
AM-70	33C	156.0	6,110	75

TABLE NO. 5 , SUMMARY OF UNCONFINED  
COMPRESSION STRENGTHS ON ROCK CORES

Sheet 3

Boring Number	Core Number	Sample Depth Feet	Peak Compressive Strength, PSI	R.Q.D. Value of Core Run %
AM-71	12C	75.0	11,460	100
AM-71	25C	127.0	22,290	100
AM-71	33C	164.0	14,330	90
A-79	8C	79.0	11,110	70
A-79	14C	109.0	22,290	40
A-79	18C	126.0	21,660	85
A-80	17C	101.0	9,550	95
A-80	21C	124.0	10,200	98
A-80	27C	153.0	16,240	90
<p>NOTE: Five (5) additional rock cores from Borings AM-6U, AM-9U, AM-10 &amp; AM-14 could not be tested due to damage in transit.</p>				

TABLE NO. 6, ATTITUDES OF PRINCIPAL BEDROCK JOINTS, PIEDMONT IN NORTHWEST WASHINGTON DC AND MONTGOMERY CO.

JOINT SET *	1	2	3	3 - 4	4
FELLOWS (1950)		$\frac{N 42^{\circ} E}{68^{\circ} SE}$	$\frac{N 40^{\circ} W}{68^{\circ} NE}$	$\frac{N 72^{\circ} W}{Vertical}$ ← STRIKE ← DIP	
JOHNSTON (1964)	$\frac{N - S}{60^{\circ} W}$ to Vert. (Lination)	$\frac{N 30^{\circ} \text{ to } 50^{\circ} E}{35^{\circ} \text{ to } 65^{\circ} SE}$			$\frac{N 70^{\circ} W}{N 70^{\circ} E}$ Vertical to 65°
CLOOS (1964)		$\frac{N 20^{\circ} E}{68^{\circ} SE}$		$\frac{N 70^{\circ} W}{70 NE}$	
MRWJ A 006 (1967)	$\frac{N 7^{\circ} W}{55^{\circ} W}$	$\frac{N 20^{\circ} E}{44^{\circ} SE}$		$\frac{N 73^{\circ} W}{53^{\circ} NE}$	
MRWJ A 009 (1973)	$\frac{N 5^{\circ} \text{ to } 10^{\circ} W}{60^{\circ} \text{ to } 75^{\circ} W}$	$\frac{N 50^{\circ} E}{50^{\circ} \text{ to } 70^{\circ} SE}$ (Variable)		$\frac{N 60^{\circ} W}{70^{\circ} \text{ to } 90^{\circ} N}$	
MRWJ BETHESDA STA. (1974)	$\frac{N 5^{\circ} \text{ to } 10^{\circ} W}{40^{\circ} \text{ to } 70^{\circ} W}$ (also foliation)	$\frac{N 40^{\circ} E}{30^{\circ} \text{ to } 65^{\circ} SE}$			
MRWJ MEDICAL CENTER STA. (1974)	Horizontal jointing and indistinct foliation are characteristic.			$\frac{N 60^{\circ} \text{ to } 90^{\circ} W}{65^{\circ} \text{ to } 75^{\circ} NE}$ (Primary)	$\frac{N 5^{\circ} W}{50^{\circ} \text{ to } 75^{\circ} E}$ (Secondary)

NOTES:

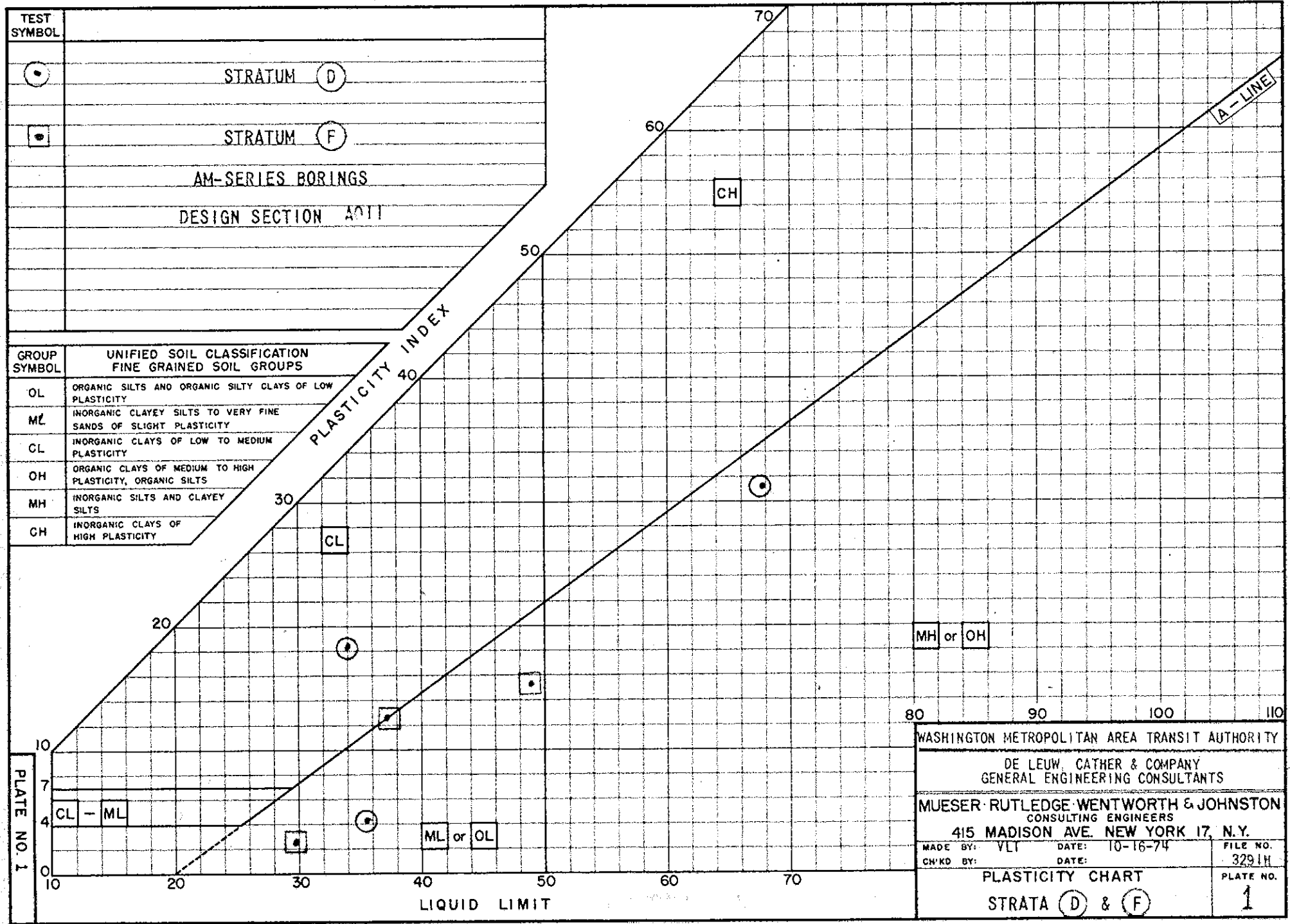
\* Joint sets are numbered in order of importance established by U of Ill. from exposures in A 004 tunnels (data from R.A. Robinson).

Determinations by Mueser, Rutledge, Wentworth & Johnston are from borehole photography investigations in sections of Rockville Route.

TABLE NO. 7

ZONES OF THE WEATHERED PROFILE AT LOCATIONS  
OF A011 SHAFTS

Shaft Designation	Applicable Boring	Elevations of Principal Zones	
		Base of Saprolite = Top of Transition	Base of Transition = Top Weathered Bedrock
Locust Hill Road Vent Shaft	AM-44	244	235
Medical Center South Vent Shaft	AM-61 (AM-58U)	298	288
Bellevue Drive Fan Shaft	AM-45	220	210
North Drive Fan Shaft	AM-46 (A-89)	236	215
West Virginia Avenue Fan Shaft	AM-59	328	320
Bethesda Station North Vent Shaft	AM-60	310	304
Medical Center North Vent Shaft	AM-47	307	287



TEST SYMBOL	
○	STRATUM (D)
□	STRATUM (F)
	AM-SERIES BORINGS
	DESIGN SECTION A011

GROUP SYMBOL	UNIFIED SOIL CLASSIFICATION FINE GRAINED SOIL GROUPS
OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
ML	INORGANIC CLAYEY SILTS TO VERY FINE SANDS OF SLIGHT PLASTICITY
CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY
OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
MH	INORGANIC SILTS AND CLAYEY SILTS
CH	INORGANIC CLAYS OF HIGH PLASTICITY

PLATE NO. 1

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

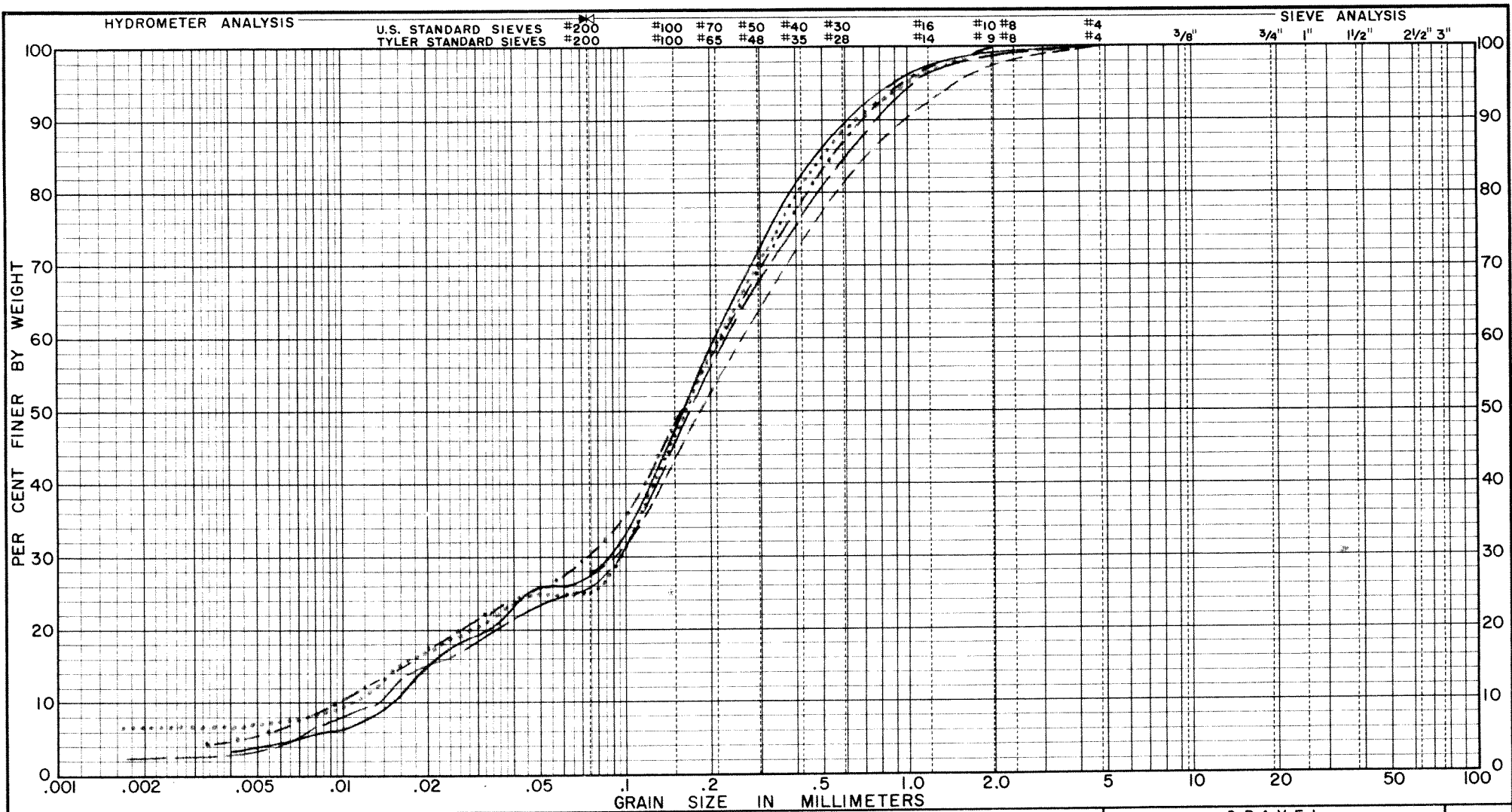
DE LEUW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANTS

MUESER, RUTLEDGE, WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N.Y.

MADE BY: VLT DATE: 10-16-74 FILE NO. 3291H  
 CH/KD BY: DATE: PLATE NO. 1

PLASTICITY CHART  
STRATA (D) & (F)





UNIFIED SOILS CLASSIFICATION ▷      CLAY OR SILT      SAND      GRAVEL      COBBLES  
 FINE      MEDIUM      COARSE      FINE      COARSE

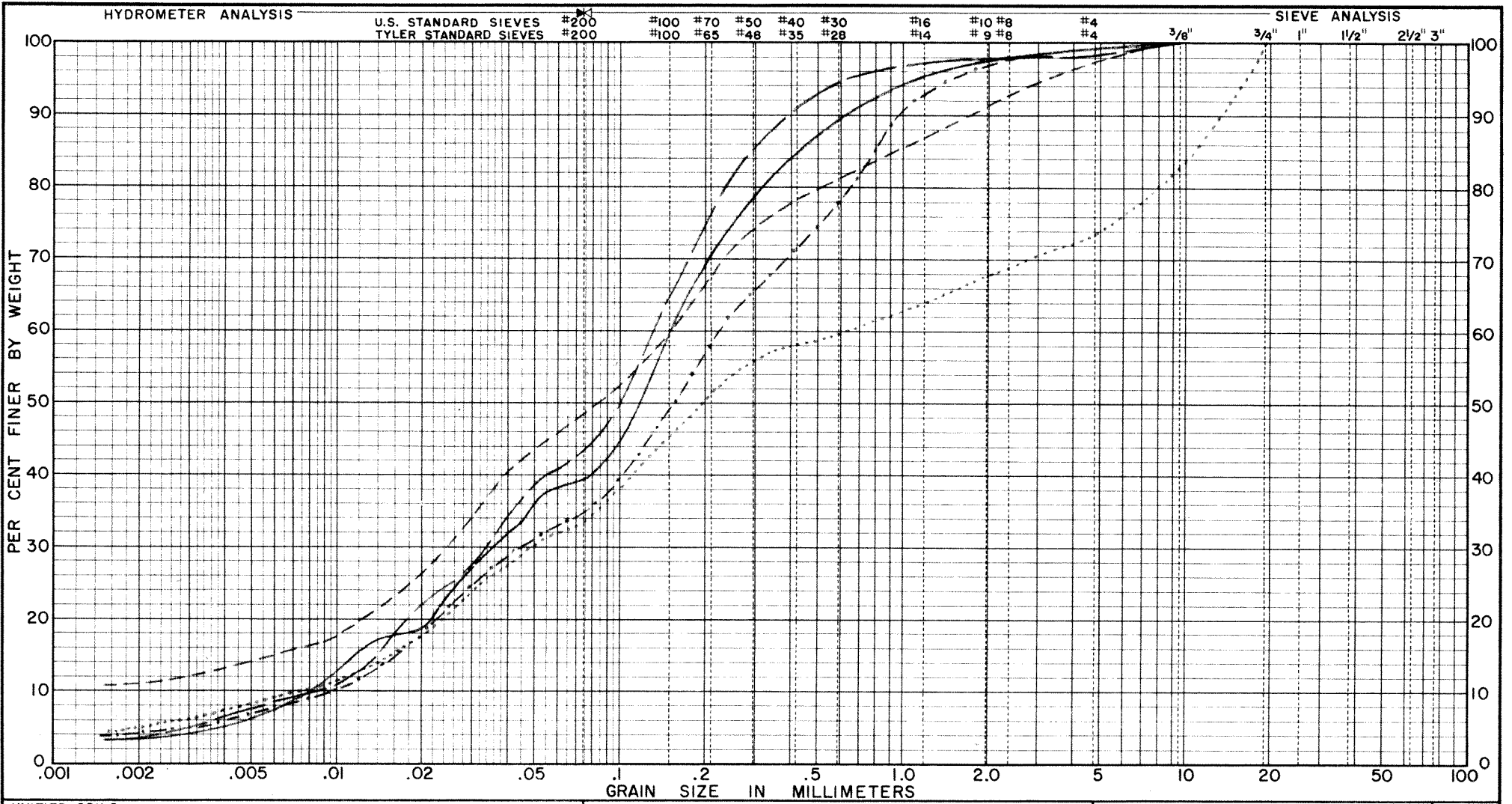
PLATE NO. 2	SYMBOL	BORING	SAMPLE	DESCRIPTION OF SAMPLE
	—	AM-3U	4U	
—	AM-3U	8U		
- - -	AM-6U	7U		
.....	AM-6U	12U		
- - -	AM-9U	2U		

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
 DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANTS

MUESER RUTLEDGE WENTWORTH & JOHNSTON  
 CONSULTING ENGINEERS  
 415 MADISON AVE., NEW YORK, N.Y. 10017

MADE BY: AR      DATE: 10-23-74      FILE NO. 3291H  
 CH'KD BY:      DATE:      PLATE NO. 2

GRADATION CURVES  
 STRATUM (D)



UNIFIED SOILS CLASSIFICATION ▷ CLAY OR SILT SAND GRAVEL COBBLES

FINE MEDIUM COARSE FINE COARSE

PLATE NO. 3	SYMBOL	BORING	SAMPLE	DESCRIPTION OF SAMPLE
	—	AM-9U	6D	
---	AM-184	3D		
----	AM-28	3D		
.....	AM-29	2D		
-.-.-	AM-30	3D		

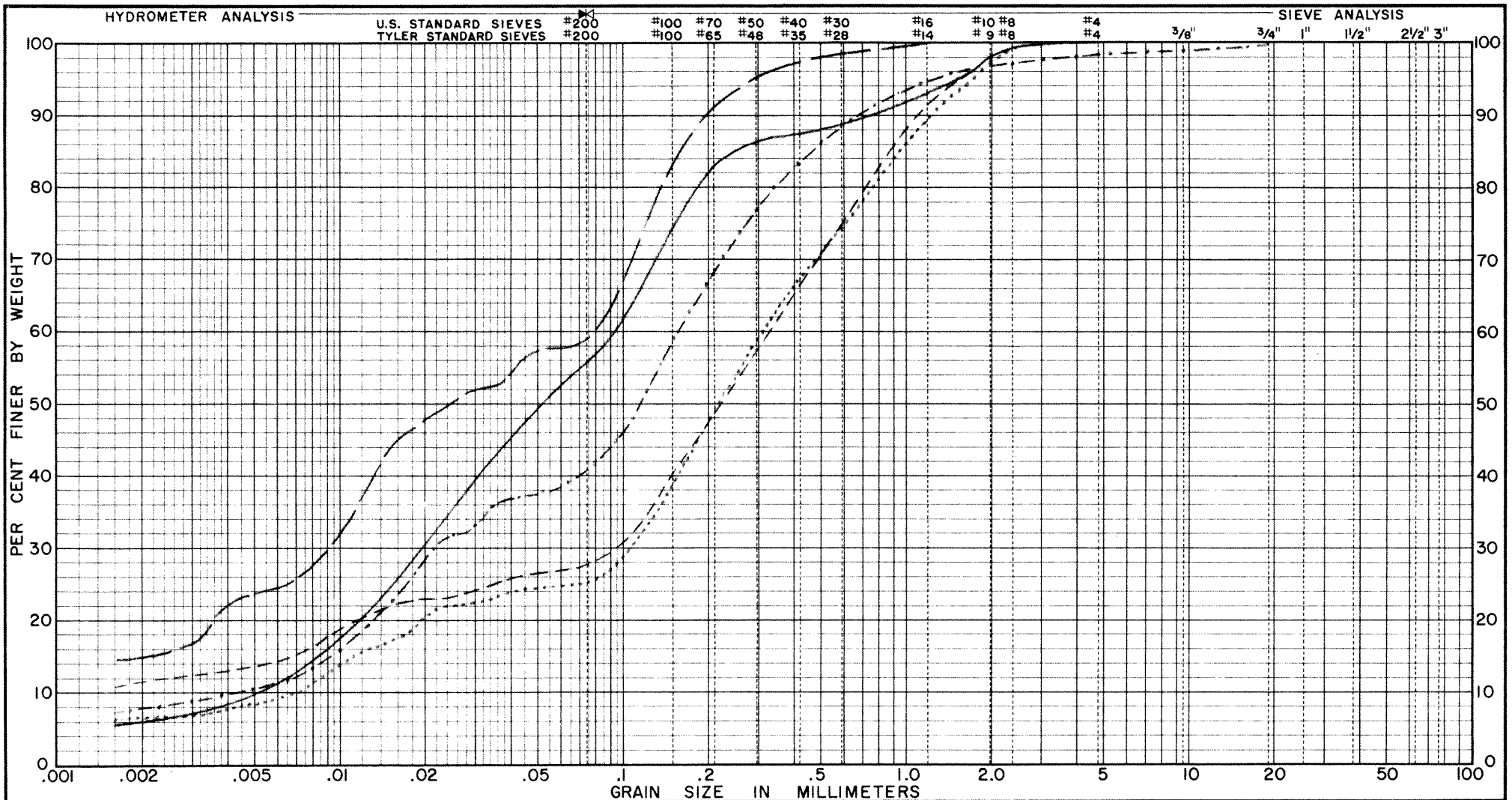
WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DE LEUW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANTS

MUESER · RUTLEDGE · WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK, N.Y. 10017

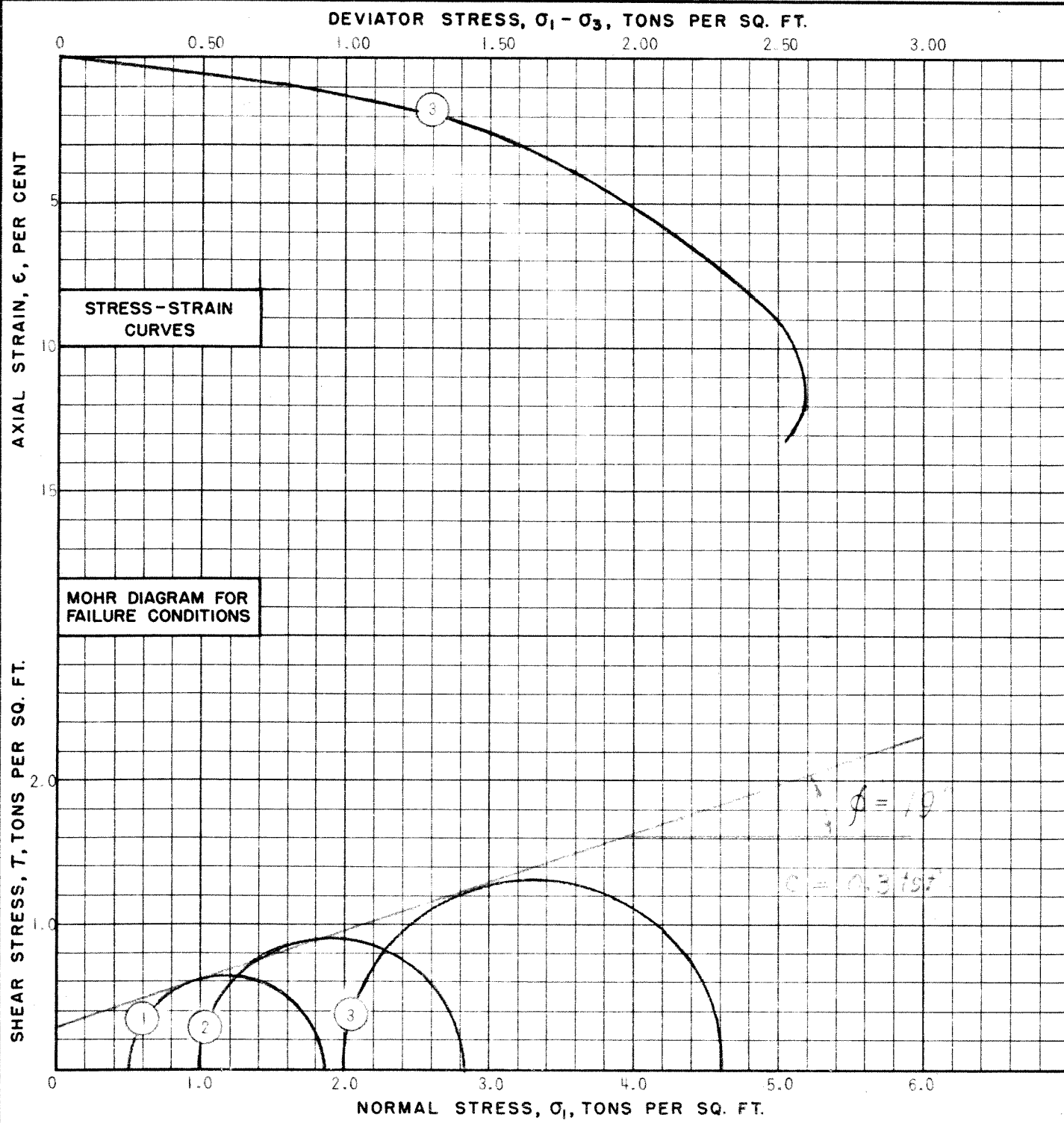
MADE BY: AR DATE: 10-29-74 FILE NO.  
CHKD BY: VLT DATE: 10-29-74 3291H

GRADATION CURVES  
STRATUM (D) PLATE NO. 3

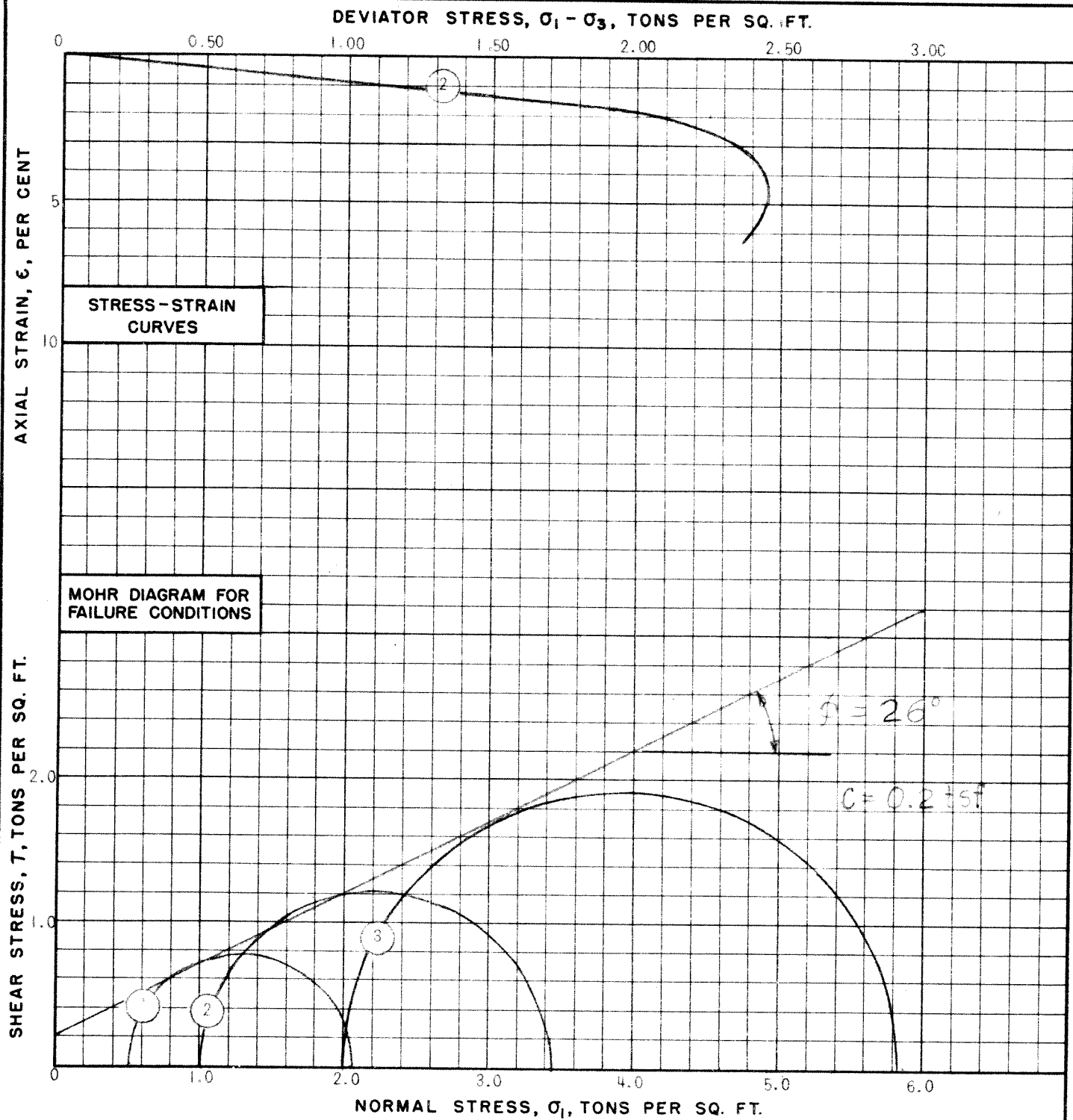


UNIFIED SOILS CLASSIFICATION ▷ CLAY OR SILT S A N D GRAVEL COBBLES  
 FINE MEDIUM COARSE FINE COARSE

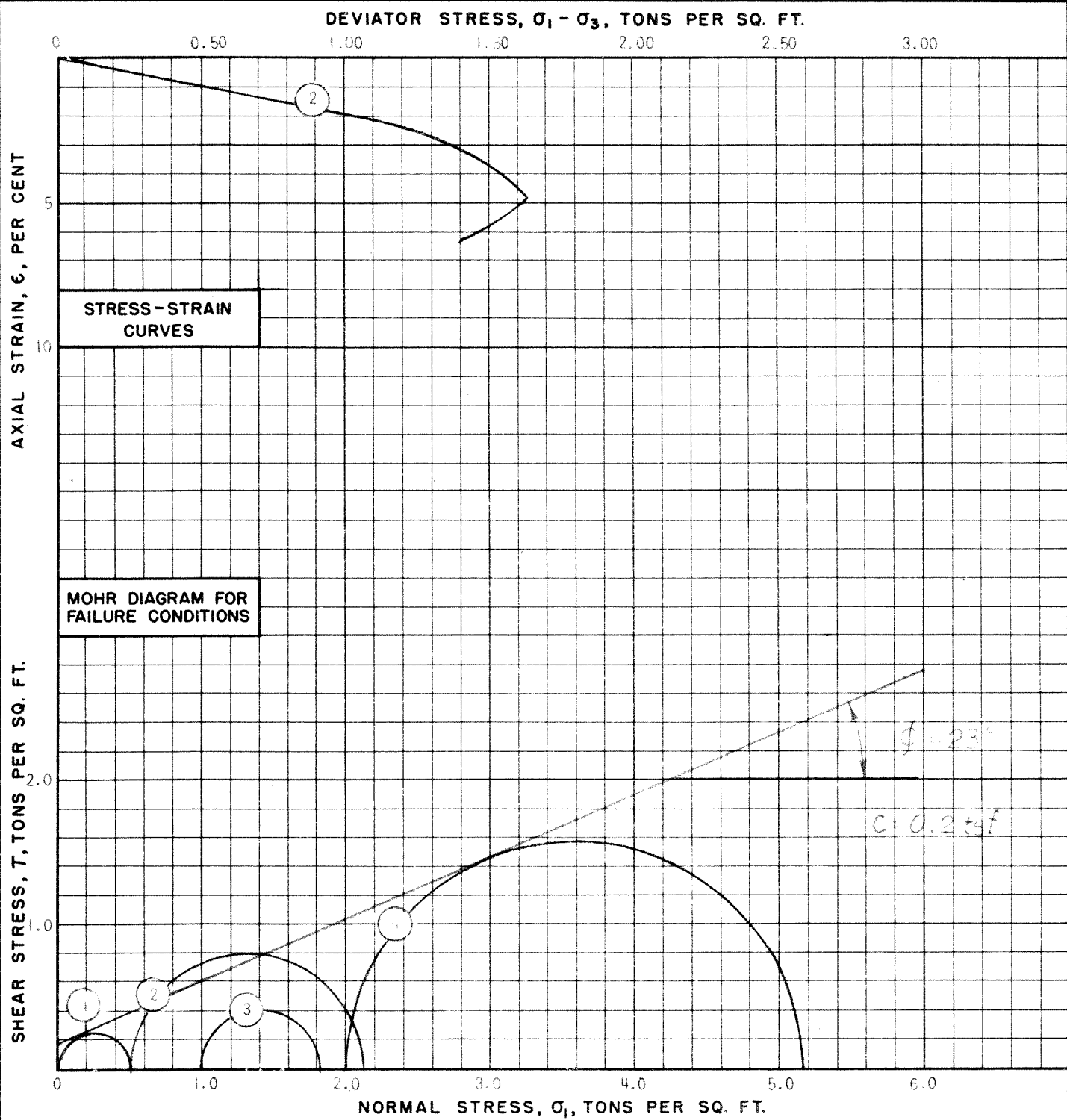
PLATE NO. 4	SYMBOL	BORING	SAMPLE	DESCRIPTION OF SAMPLE	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY		
	————	AM-36	2D	STRATUM (D)	DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS		
	-----	AM-44U	4U		MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS		
	-----	AM-58U	4U		415 MADISON AVE., NEW YORK, N.Y. 10017		
	.....	AM-58U	5U		MADE BY: AR DATE: 10-23-74 FILE NO.		
	-----	AM-59U	2U		CH'KD BY: VLT DATE: 10-24-74 329 JH		
			GRADATION CURVES		PLATE NO.		
			STRATUM (D)	4			



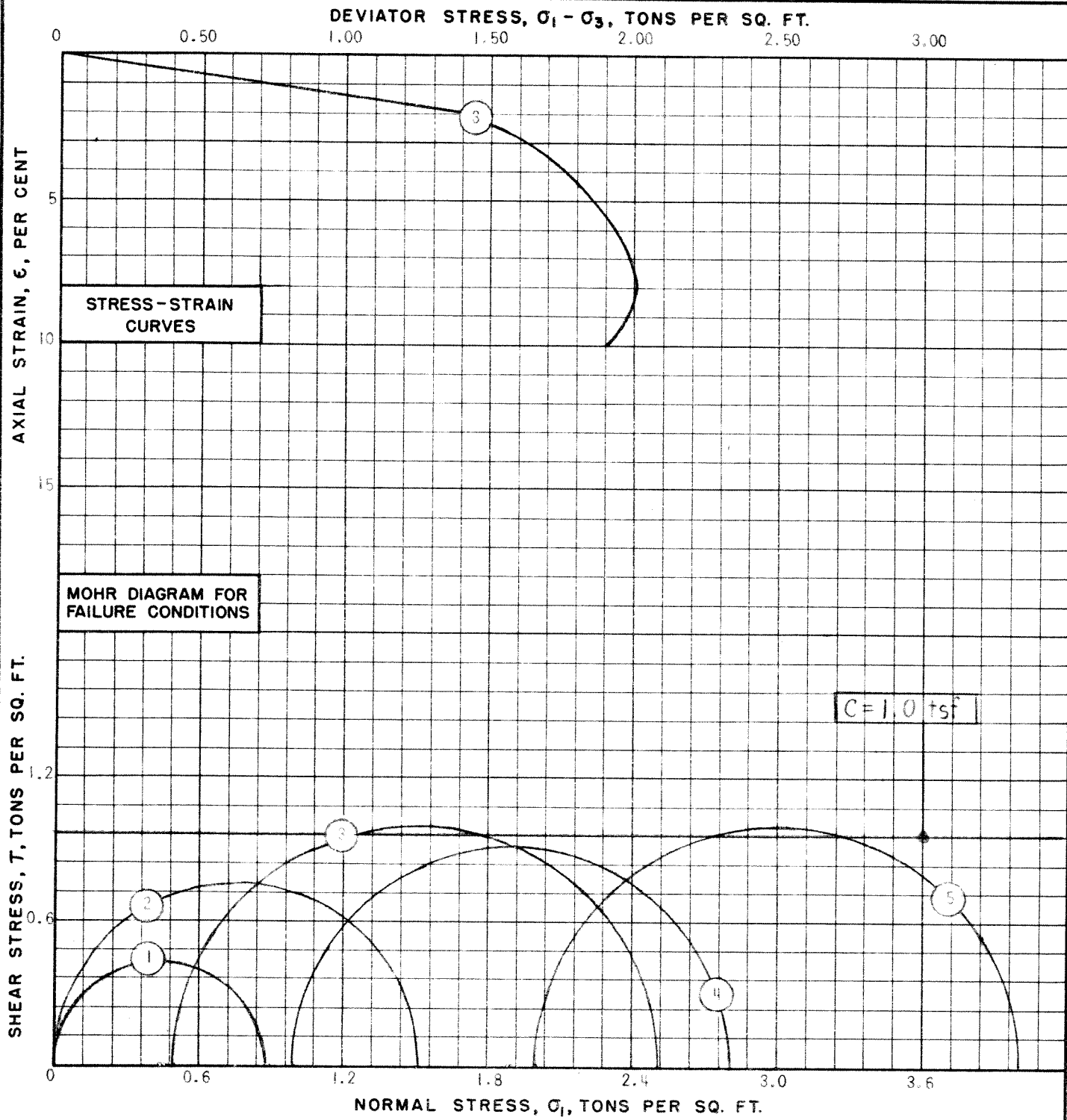
DESCRIPTION OF MATERIAL TESTED												SYMBOLS FOR TEST TYPE	
BROWN SILTY FINE SAND, TRACE MEDIUM TO COARSE SAND STRATUM (D)												Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST	
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY	
						INITIAL	FINAL	INITIAL	FINAL			DE LEUW, CATHER, & COMPANY GENERAL ENGINEERING CONSULTANT	
(1)	3U	2U	SM	Q	0.5	16	16			1.36	13	MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017	
(2)	3U	2U	SM	Q	1.0	13	13			1.83	14	MADE BY: AAK	DATE: 6-6-74
(3)	3U	2U	SM	Q	2.0	15	15			2.59	8	CH'KD BY: VLT	DATE: 10-10-74
												SUMMARY OF STRENGTH TESTS	
BORING AM-3U SAMPLE 2U												FILE NO. 3291H PLATE NO. 5	



DESCRIPTION OF MATERIAL TESTED											SYMBOLS FOR TEST TYPE	
BROWN SILTY FINE SAND, TRACE COARSE SAND STRATUM (D)											Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST	
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY
						INITIAL	FINAL	INITIAL	FINAL			
(1)	3U	4U	SM	Q	0.5	10	10			1.51	2	DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS
(2)	3U	4U	SM	Q	1.0	10	9			2.45	4	<b>MUESER · RUTLEDGE · WENTWORTH &amp; JOHNSTON</b> CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017
(3)	3U	4U	SM	Q	2.0	10	10			2.83	6	MADE BY: AAK DATE: 6-6-74 FILE NO. 3291H CH'KD BY: VLT DATE: 10-10-74 3291H
<b>SUMMARY OF STRENGTH TESTS</b>											BORING AM-3U SAMPLE 4U	
											6	

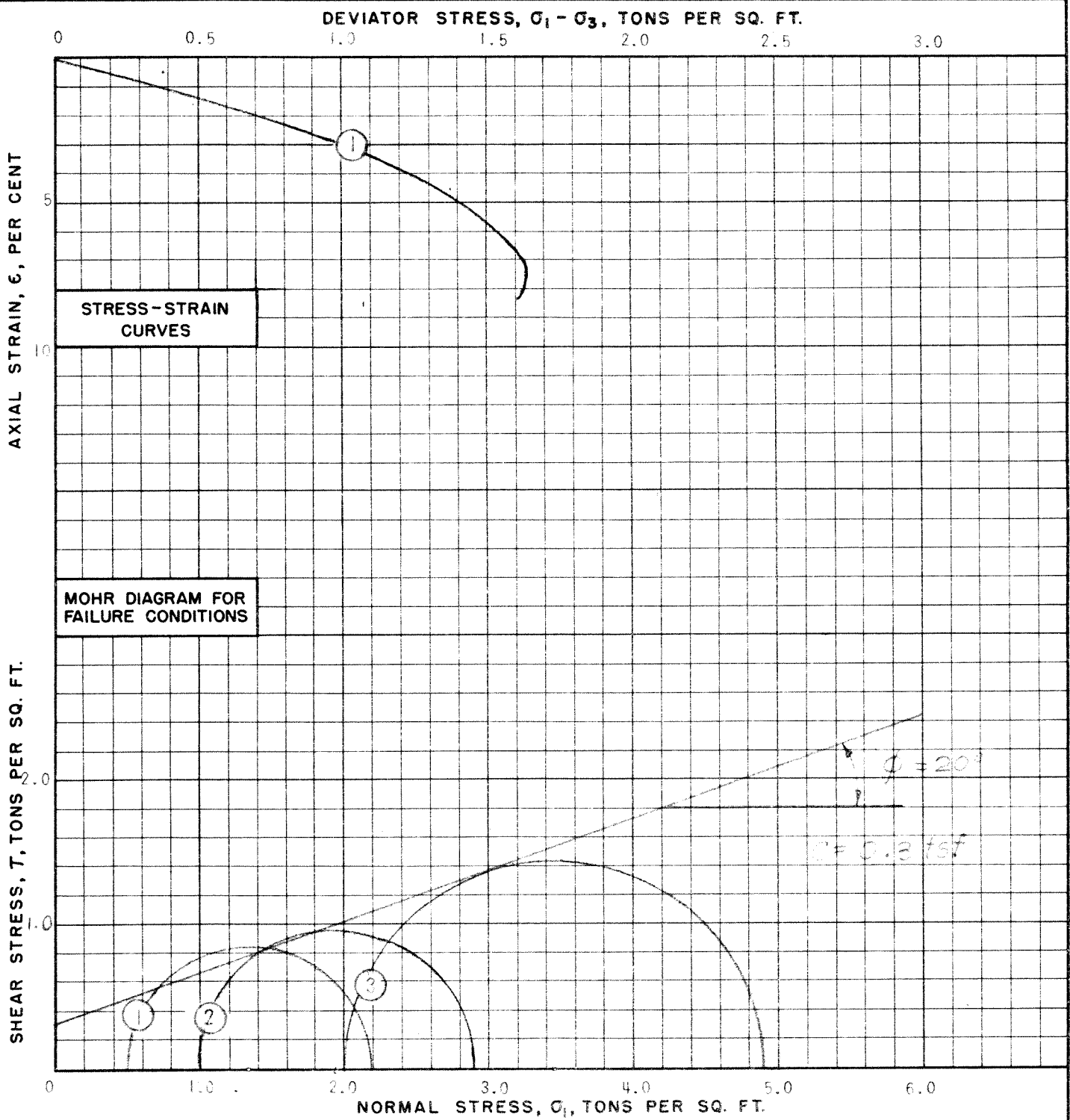


DESCRIPTION OF MATERIAL TESTED												SYMBOLS FOR TEST TYPE		
BROWN SILTY FINE TO MEDIUM SAND, TRACE COARSE SAND STRATUM (D)												Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST		
												U - UNCONFINED COMPRESSION TEST		
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY		
						INITIAL	FINAL	INITIAL	FINAL			DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS		
1	3U	10U	SM	U	-	33	32			0.52	4	MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017		
2	3U	3U	SM	Q	0.5	21	21			1.63	5	MADE BY: AAK	DATE: 6-13-74	FILE NO. 3291H
3	3U	10U	SM	Q	1.0	18	16			0.82	5	CHK'D BY: VLT	DATE: 10-10-74	
4	3U	8U	SM	Q	2.0	-	-			3.16	8	SUMMARY OF STRENGTH TESTS		
												BORING AM-3U SAMPLE 8U & 10U		
												PLATE NO. 7		



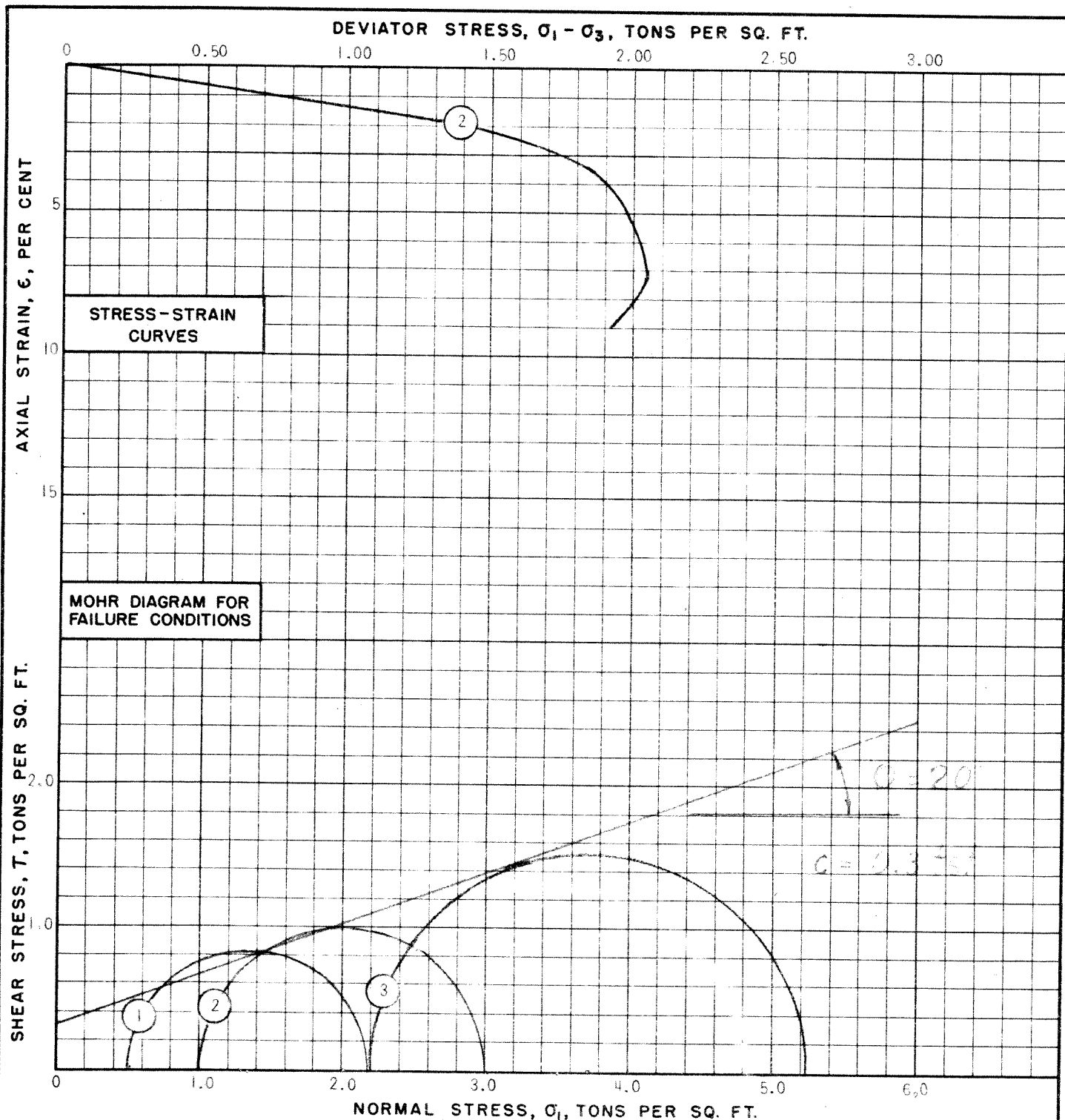
DESCRIPTION OF MATERIAL TESTED											SYMBOLS FOR TEST TYPE		
STIFF BROWN CLAYEY SILT AND SILTY CLAY, SOME FINE SAND, TRACE GRAVEL STRATUM (F)											Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST		
											U - UNCONFINED COMPRESSION TEST		
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY	
						INITIAL	FINAL	INITIAL	FINAL			DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS	
1	6U	2U	ML	U	-	23	23			0.90	4	<b>MUESER · RUTLEDGE · WENTWORTH &amp; JOHNSTON</b> CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017 MADE BY: AAK DATE: 5-28-74 FILE NO. CH'KD BY: VLT DATE: 6-3-74 3291H <b>SUMMARY OF STRENGTH TESTS</b> PLATE NO.	
2	6U	3U	ML	U	-	25	25			1.51	5		
3	6U	3U	ML	Q	0.5	29	-			2.01	8		
4	6U	2U	CL	Q	1.0	30	30			1.81	19		
5	6U	2U	CL	Q	2.0	32	32			2.00	15		
											BORING AM-6U SAMPLE 2U & 3U		

8

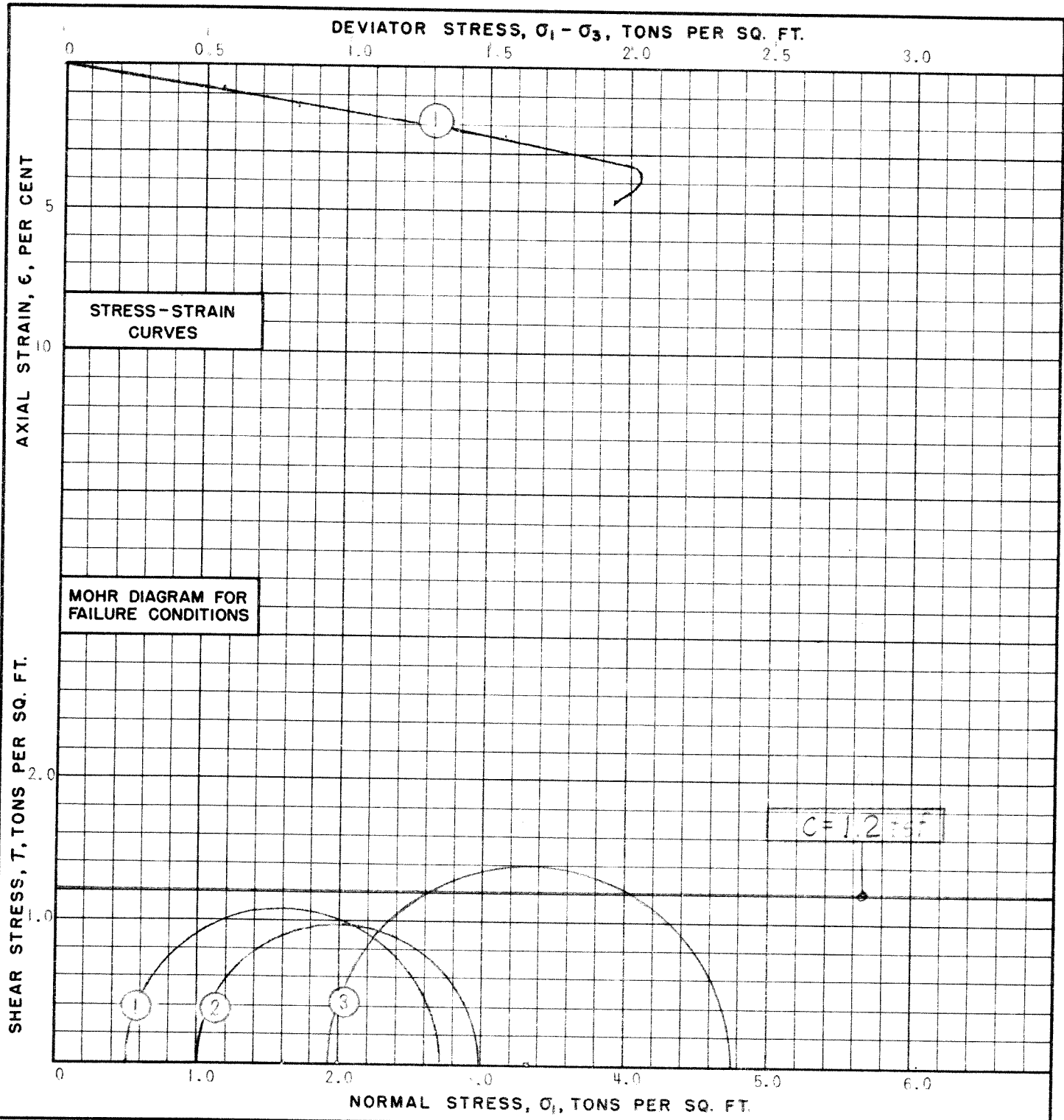


DESCRIPTION OF MATERIAL TESTED											SYMBOLS FOR TEST TYPE	
BROWN SILTY FINE SAND STRATUM (D)											Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST	
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS
						INITIAL	FINAL	INITIAL	FINAL			
1	6U	4U	SM	Q	0.5	21	21			1.65	7	
2	6U	4U	SH	Q	1.0	24	23			1.87	13	
3	6U	4U	SH	Q	2.0	23	22			2.90	10	





DESCRIPTION OF MATERIAL TESTED											SYMBOLS FOR TEST TYPE			
BROWN-GRAY SILTY FINE TO MEDIUM SAND TRACE COARSE SAND STRATUM (D)											Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST			
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY		
						INITIAL	FINAL	INITIAL	FINAL			DE LEUN, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS		
1	6U	7U	SM	Q	0.5	17	17			1.69	4	MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017		
2	6U	6U	SM	Q	1.0	24	23			2.05	7	MADE BY: AAK	DATE: 5-31-74	FILE NO. 3291H
3	6U	6U	SM	Q	2.0	24	23			3.04	6	CH'KD BY: VLT	DATE: 10-10-74	PLATE NO. 10
											<b>SUMMARY OF STRENGTH TESTS</b>			
											BORING AM-6U SAMPLE 6U & 7U			



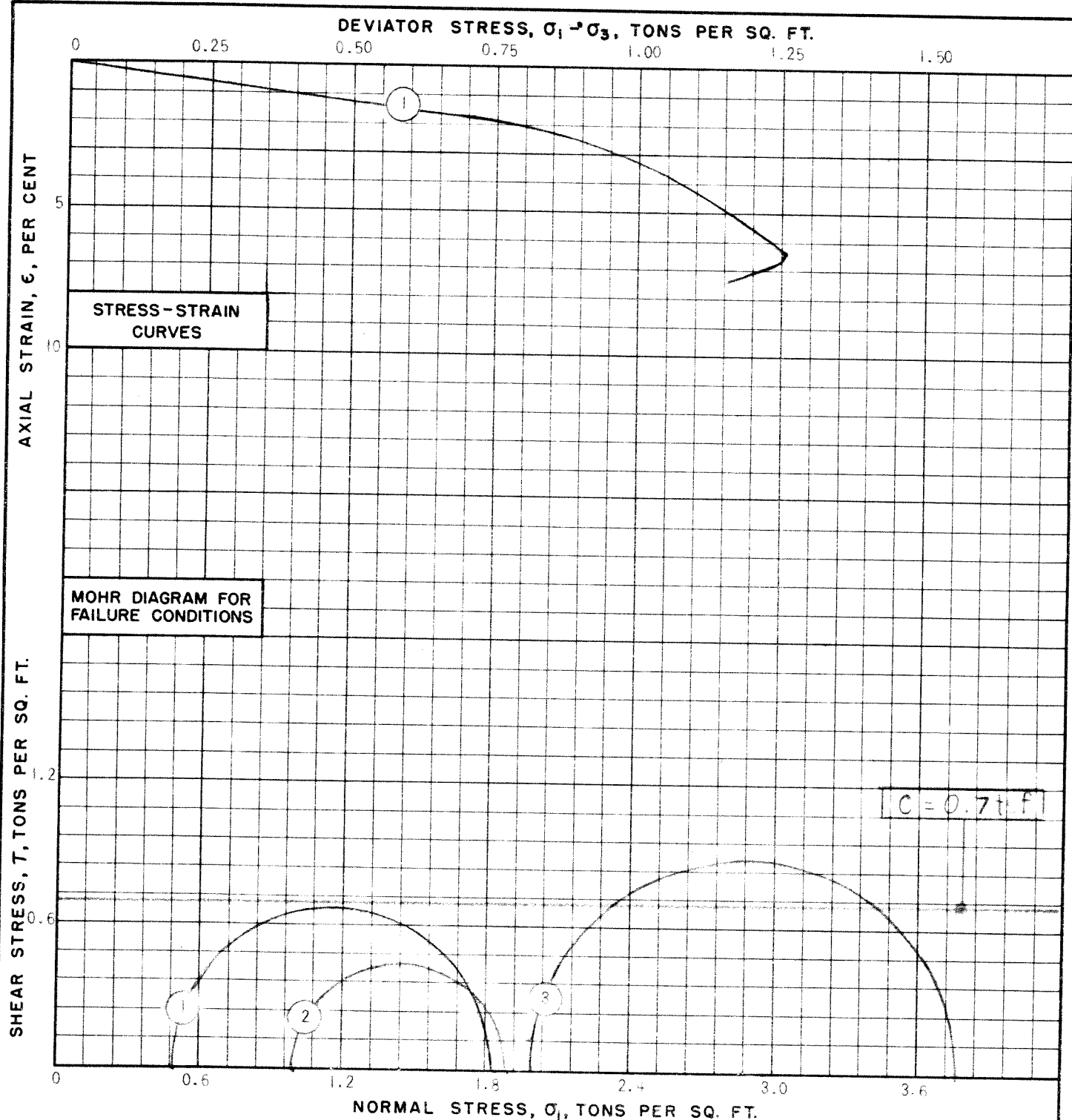
DESCRIPTION OF MATERIAL TESTED										SYMBOLS FOR TEST TYPE	
BROWN SILTY FINE SAND										Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST	
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %
						INITIAL	FINAL	INITIAL	FINAL		
1	6U	7U	SM	Q	0.5	16	15			2.22	4
2	6U	9U	SM	Q	1.0	22	22			2.00	9
3	6U	9U	SM	Q	2.0	23	22			2.81	7

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
 DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANTS

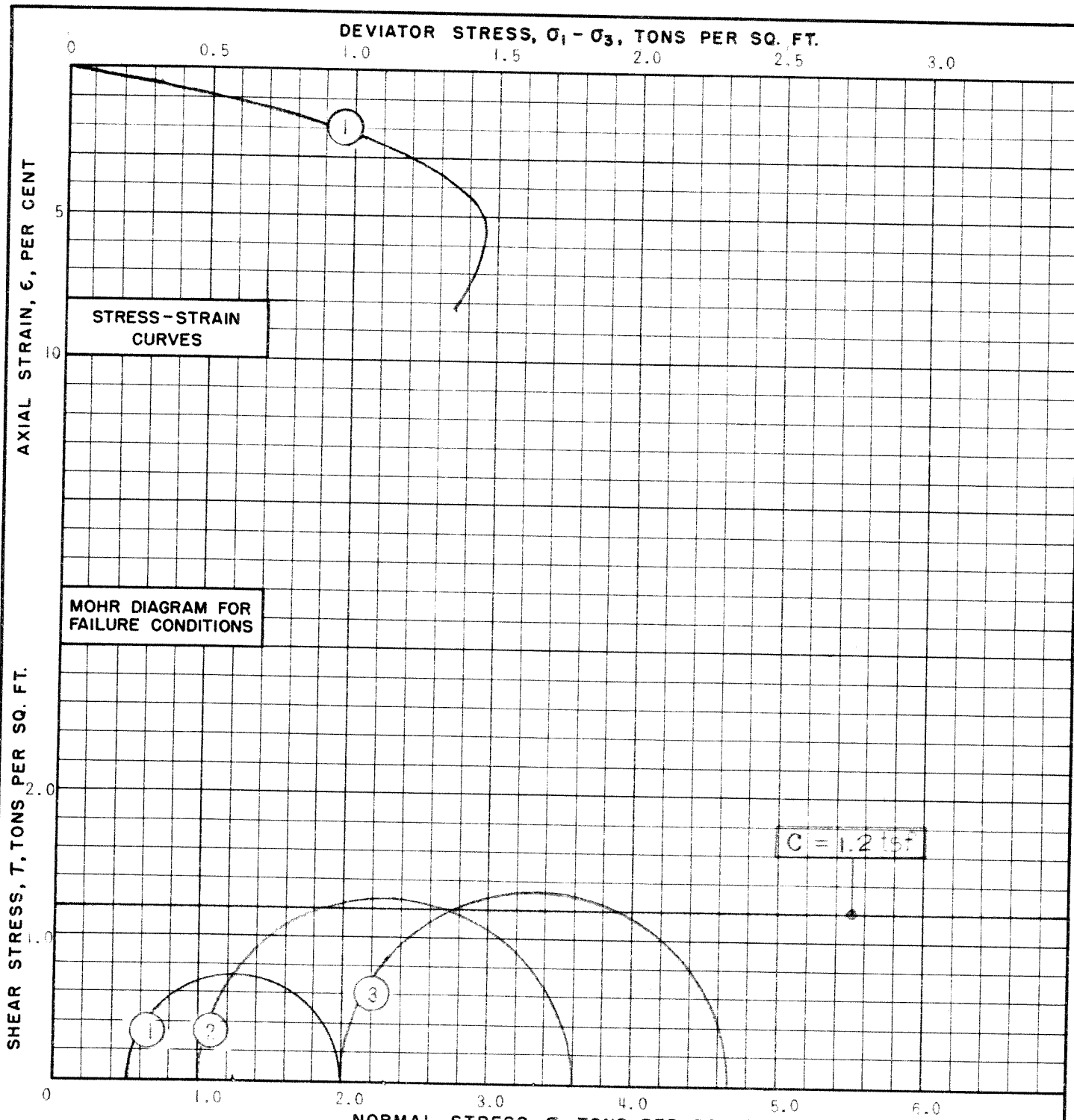
**MUESER RUTLEDGE WENTWORTH & JOHNSTON**  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK, N.Y. 10017

MADE BY: *KSW* DATE: 4-17-74 FILE NO. 32914  
 CH'KD BY: *KSW* DATE: 4-26-74 PLATE NO. 11

**SUMMARY OF STRENGTH TESTS**  
 BORING AM-6U SAMPLES 7U & 9U



DESCRIPTION OF MATERIAL TESTED											SYMBOLS FOR TEST TYPE			
BROWN-GRAY SILTY FINE TO MEDIUM SAND TRACE COARSE SAND AND GRAVEL STRATUM (D)											Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST			
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY		
						INITIAL	FINAL	INITIAL	FINAL			DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS		
1	6U	10U	SM	Q	0.50	23	24			1.26	6	MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017		
2	6U	10U	SM	Q	1.00	26	27			0.90	4	MADE BY: AAK	DATE: 5-31-74	FILE NO. 3291H
3	6U	10U	SM	Q	2.00	30	29			1.77	6	CH'KD BY: VLT	DATE: 10-10-74	PLATE NO. 12
											SUMMARY OF STRENGTH TESTS			
											BORING AM-6U SAMPLE 10U			



DESCRIPTION OF MATERIAL TESTED

BROWN SILTY FINE-MEDIUM SAND, TRACE GRAVEL  
STRATUM (D)

SYMBOLS FOR TEST TYPE

Q - UNCONSOLIDATED UNCONFINED  
TRIAXIAL SHEAR TEST

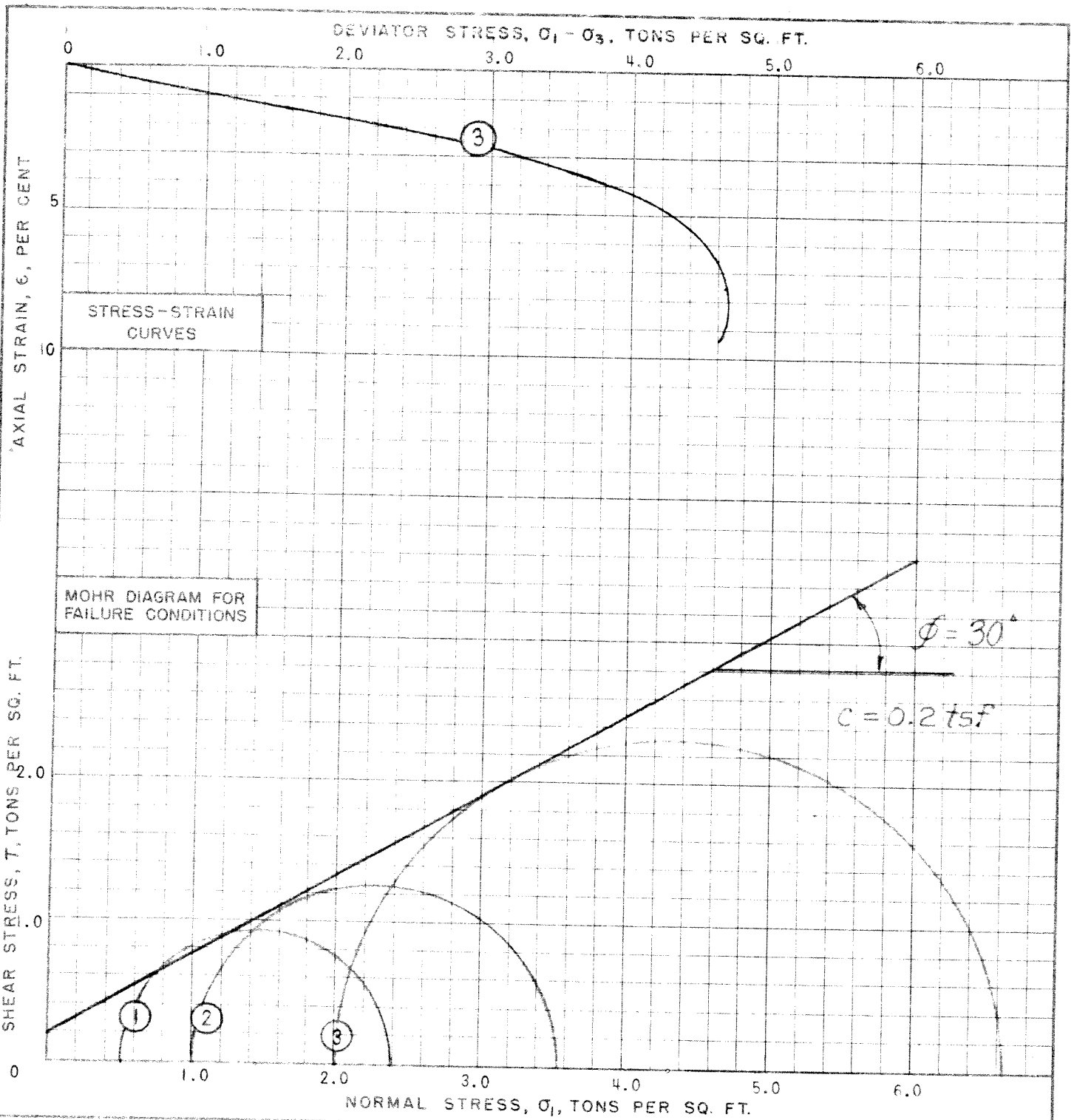
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %
						INITIAL	FINAL	INITIAL	FINAL		
1	9U	2U	SM	Q	0.5	12	12			1.44	6
2	9U	2U	SM	Q	1.0	12	11			2.60	6
3	9U	1U	SM	Q	2.0	20	19			2.68	14

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
DE LEUW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANTS

**MUESER RUTLEDGE WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK, N.Y. 10017

MADE BY: VLT DATE: 4-17-74 FILE NO.  
CHK'D BY: KSW DATE: 4-26-74 32910

**SUMMARY OF STRENGTH TESTS**  
BORING AM-9U SAMPLES 1U & 2U



DESCRIPTION OF MATERIAL TESTED										SYMBOLS FOR TEST TYPE	
BROWN SILTY FINE SAND STRATUM D										Q - UNCONSOLIDATED UNCONFINED TRIAXIAL SHEAR TEST	
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE TSF	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS TSF	AXIAL STRAIN %
						INITIAL	FINAL	INITIAL	FINAL		
1	37U	2U	SM	Q	0.5	16	16			1.88	6
2	25U	2U	SM	Q	1.0	8	-			2.57	10
3	37U	2U	SM	Q	2.0	9	18			4.65	8

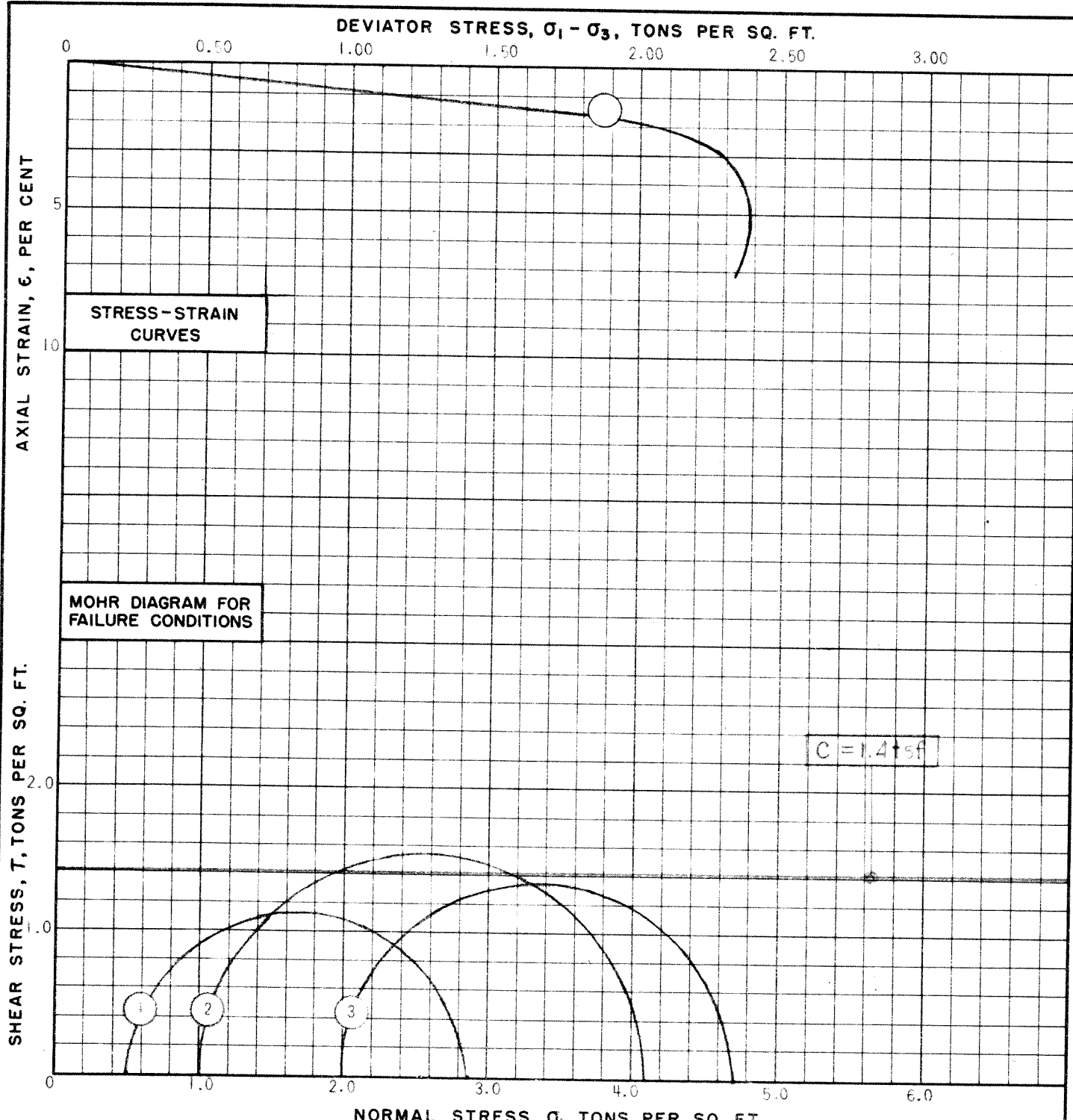
WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
 DE LEUW, CATHEW & COMPANY  
 GENERAL ENGINEERING CONSULTANTS

**MUESER RUTLEDGE WENTWORTH & JOHNSTON**  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK, N.Y. 10017

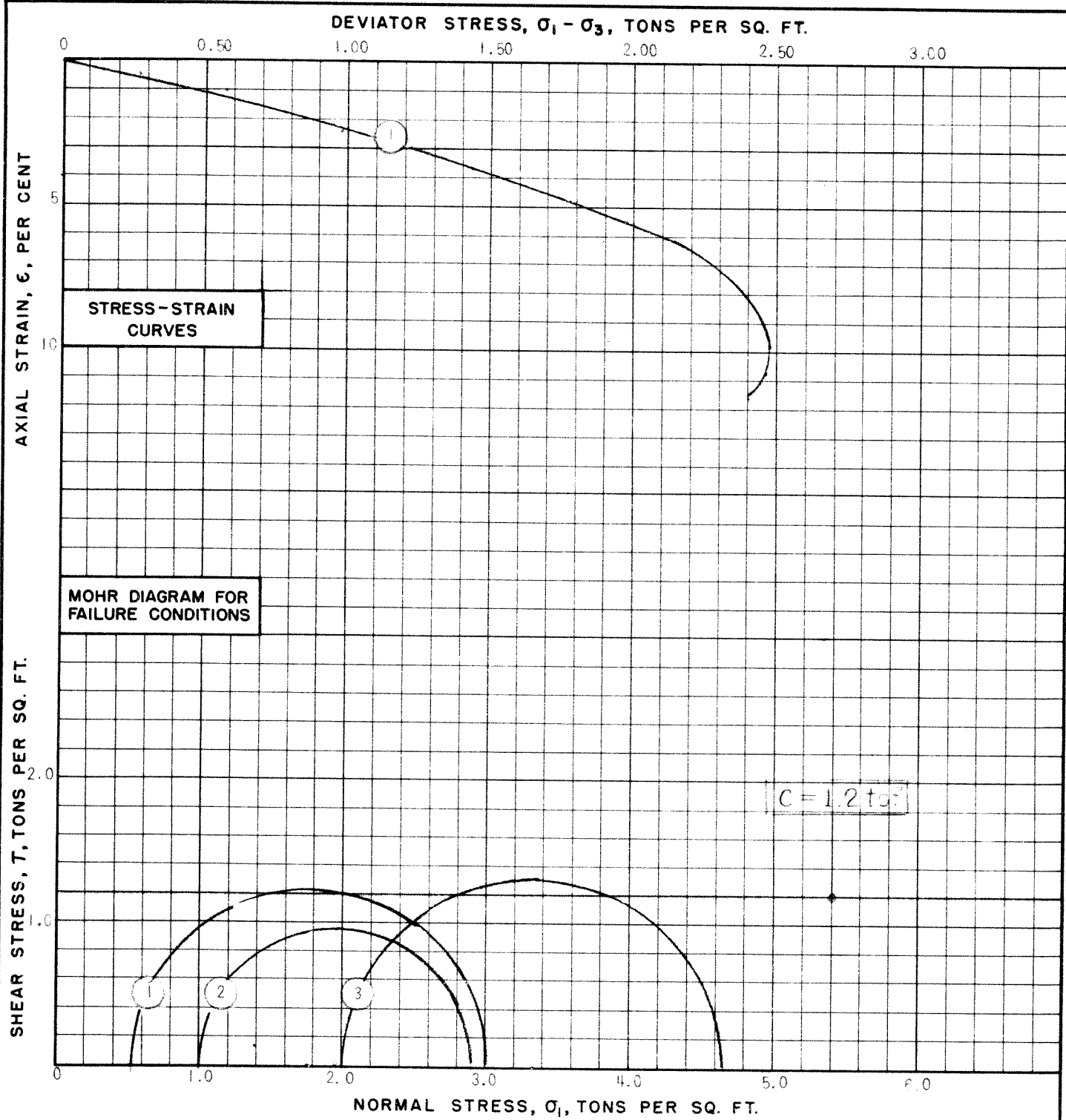
MADE BY: DG      DATE: 1-29-74      FILE NO. 3291H  
 CH'KD BY: JMC      DATE: 2-1-74

**SUMMARY OF STRENGTH TESTS**  
 BORING AM-25U SAMPLE 2U  
 BORING AM-37U SAMPLE 2U

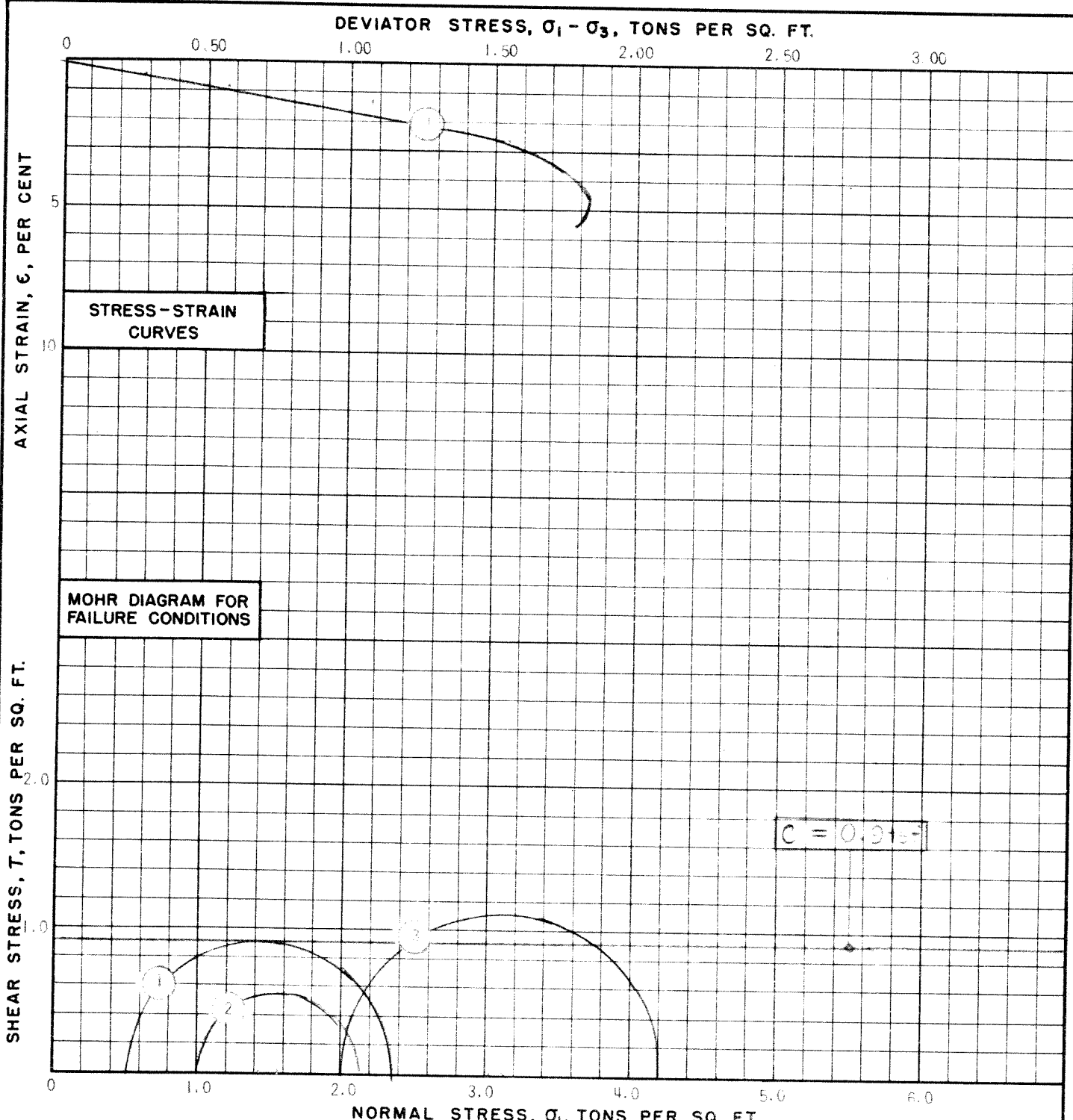
PLATE NO. **14**



DESCRIPTION OF MATERIAL TESTED											SYMBOLS FOR TEST TYPE		
BROWN SILTY FINE SAND, TRACE MEDIUM TO COARSE SAND STRATUM (D)											Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST		
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY	
						INITIAL	FINAL	INITIAL	FINAL			DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS	
1	41U	3U	SM	Q	0.5	22	21			2.38	5	MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017	
2	41U	3U	SM	Q	1.0	22	22			3.10	5	MADE BY: AAK DATE: 6-11-74 FILE NO. 32914	
3	41U	3U	SM	Q	2.0	27	27			2.71	12	CH'KD BY: V.T. DATE: 10-10-74 PLATE NO. 15	
											<b>SUMMARY OF STRENGTH TESTS</b>		
											BORING AM-41U SAMPLE 3U		

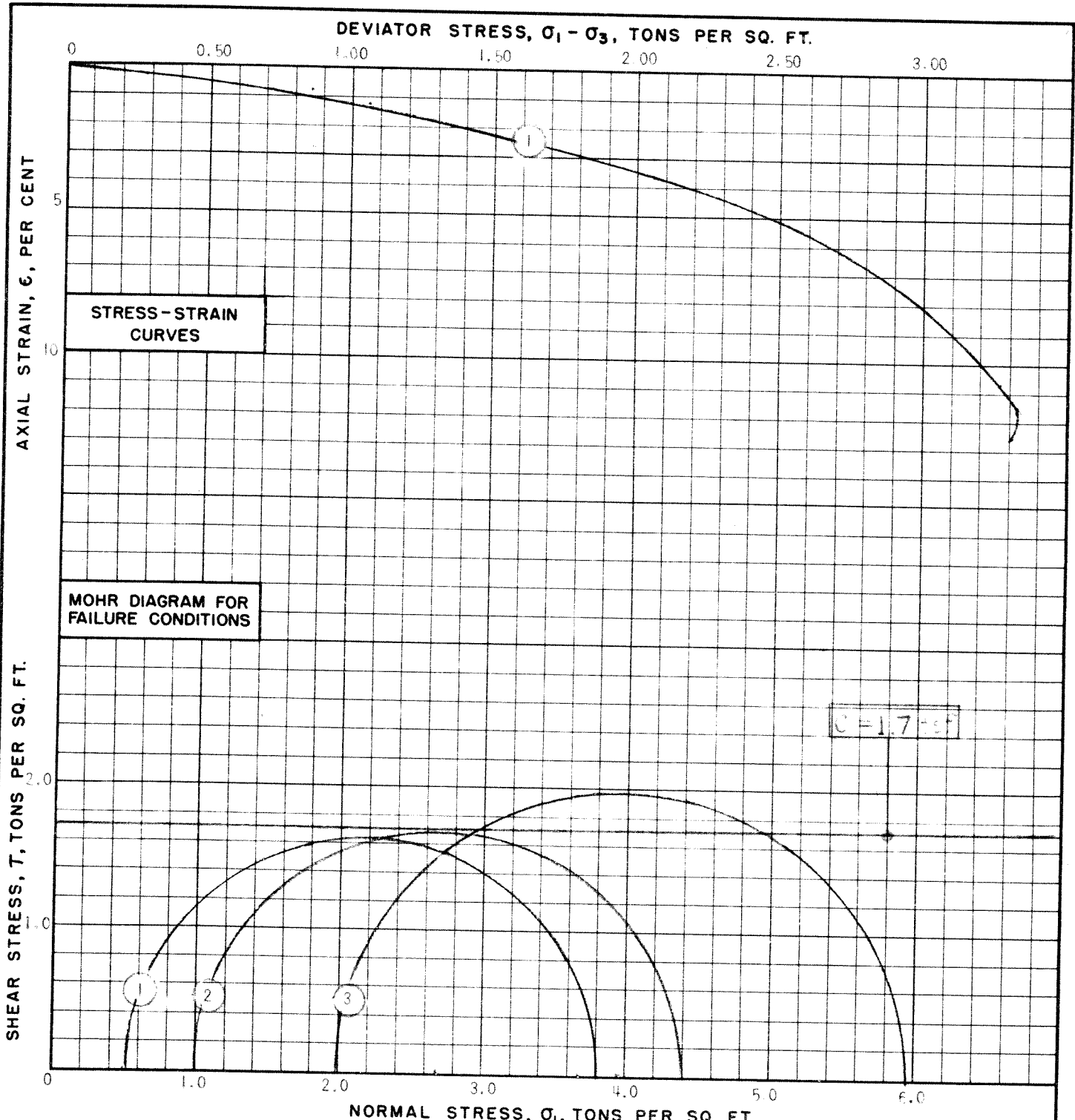


DESCRIPTION OF MATERIAL TESTED											SYMBOLS FOR TEST TYPE			
STIFF BROWN CLAYEY SILT, SOME FINE SAND AND RED-BROWN FINE SANDY SILT, SOME CLAY STRATA (D) & (F)											Q - UNCONSOLIDATED UNDRAINED TRIAxIAL SHEAR TEST			
WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY											DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS			
KEY	BORINGS NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	MUESER · RUTLEDGE · WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017		
						INITIAL	FINAL	INITIAL	FINAL			MADE BY: VLT	DATE: 10-25-74	FILE NO. 3291H
(1)	44U	4U	ML	Q	0.5	25	25			2.49	9	CH'KD BY:	DATE:	PLATE NO. 16
(2)	44U	2U	ML	Q	1.0	29	29			1.88	13	SUMMARY OF STRENGTH TESTS		
(3)	44U	2U	ML	Q	2.0	29	29			2.64	13	BORING AH-44U SAMPLE 2U & 4U		



DESCRIPTION OF MATERIAL TESTED										SYMBOLS FOR TEST TYPE			
RED-BROWN FINE SANDY SILT, TRACE COARSE SAND AND GRAVEL STRATUM (D)										Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST			
KEY	BORINGS NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY	
						INITIAL	FINAL	INITIAL	FINAL			DE LUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS	
1	44U	4U	ML	Q	0.5	29	28			1.62	5	MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017	
2	44U	6U	ML	Q	1.0	40	40			1.13	9	MADE BY: VLT DATE: 10-25-74 FILE NO. 3291H	
3	44U	6U	ML	Q	2.0	35	35			2.21	10	CH'KD BY: DATE: SUMMARY OF STRENGTH TESTS PLATE NO. 17	
												BORING AM-44U SAMPLE 4U & 6U	

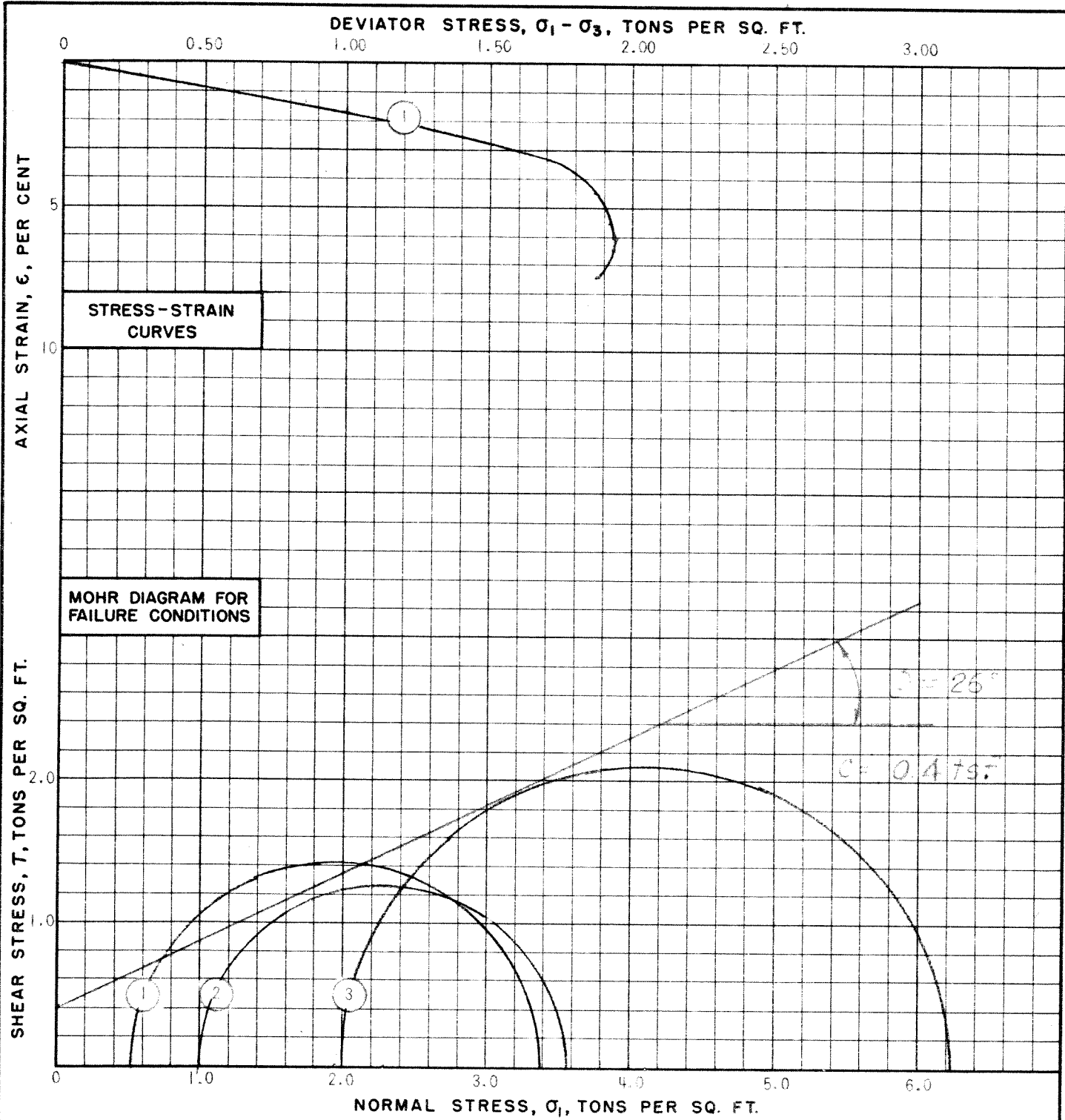




DESCRIPTION OF MATERIAL TESTED										SYMBOLS FOR TEST TYPE	
STIFF BROWN SILTY CLAY, SOME FINE SAND, TRACE ROOTS STRATUM (F)										Q - UNCONSOLIDATED UNDRAINED TRIAxIAL SHEAR TEST	
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %
1	46U	2U	CL	Q	0.5	20	20			3.32	12
2	46U	2U	CL	Q	1.0	17	17			3.35	14
3	46U	2U	CL	Q	2.0	16	16			3.91	16

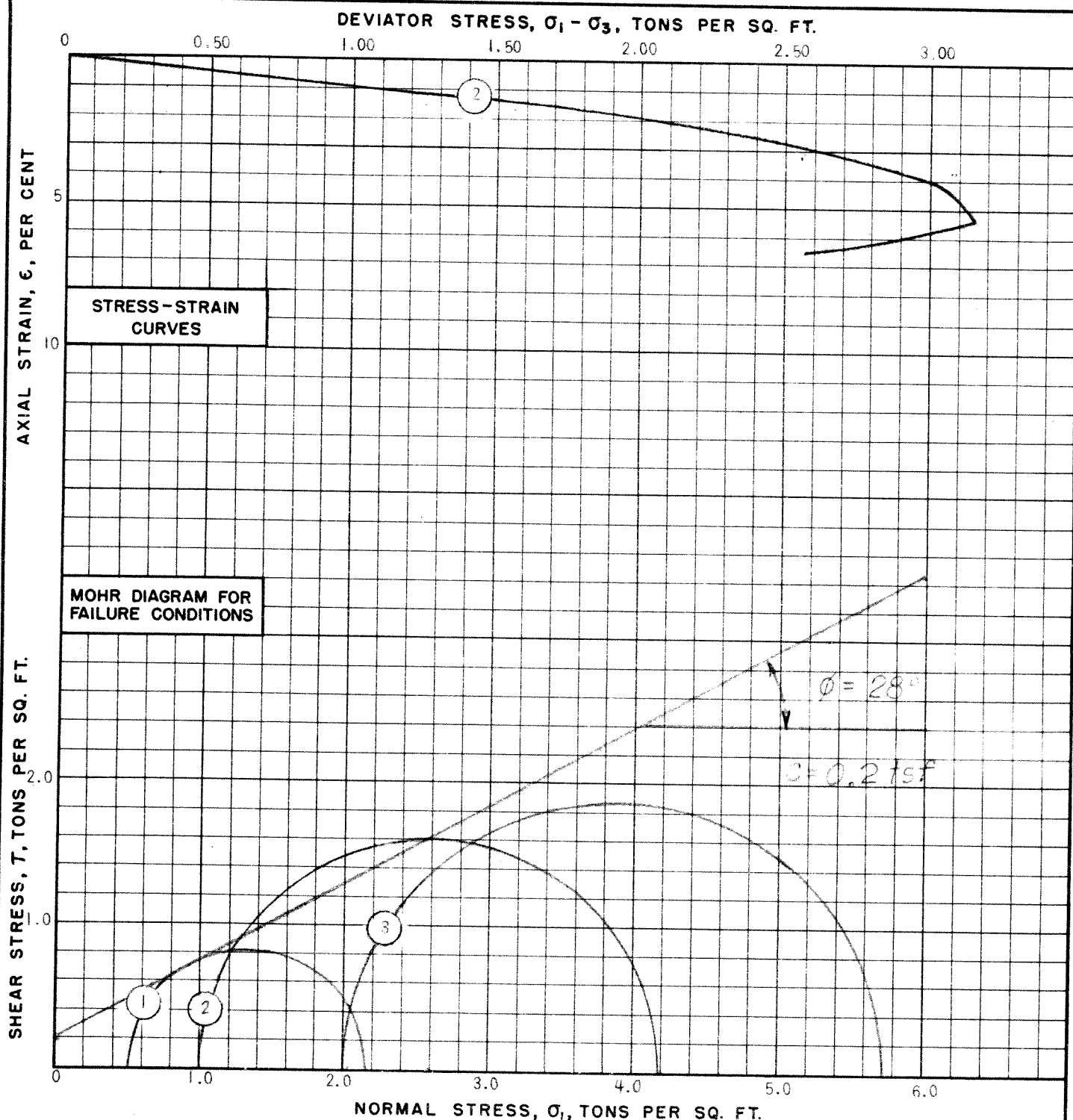
WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY		
DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS		
MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017		
MADE BY: VLT	DATE: 10-24-74	FILE NO.
CH'KD BY:	DATE:	3291H
SUMMARY OF STRENGTH TESTS		PLATE NO.
BORING AM-46U SAMPLE 2U		18



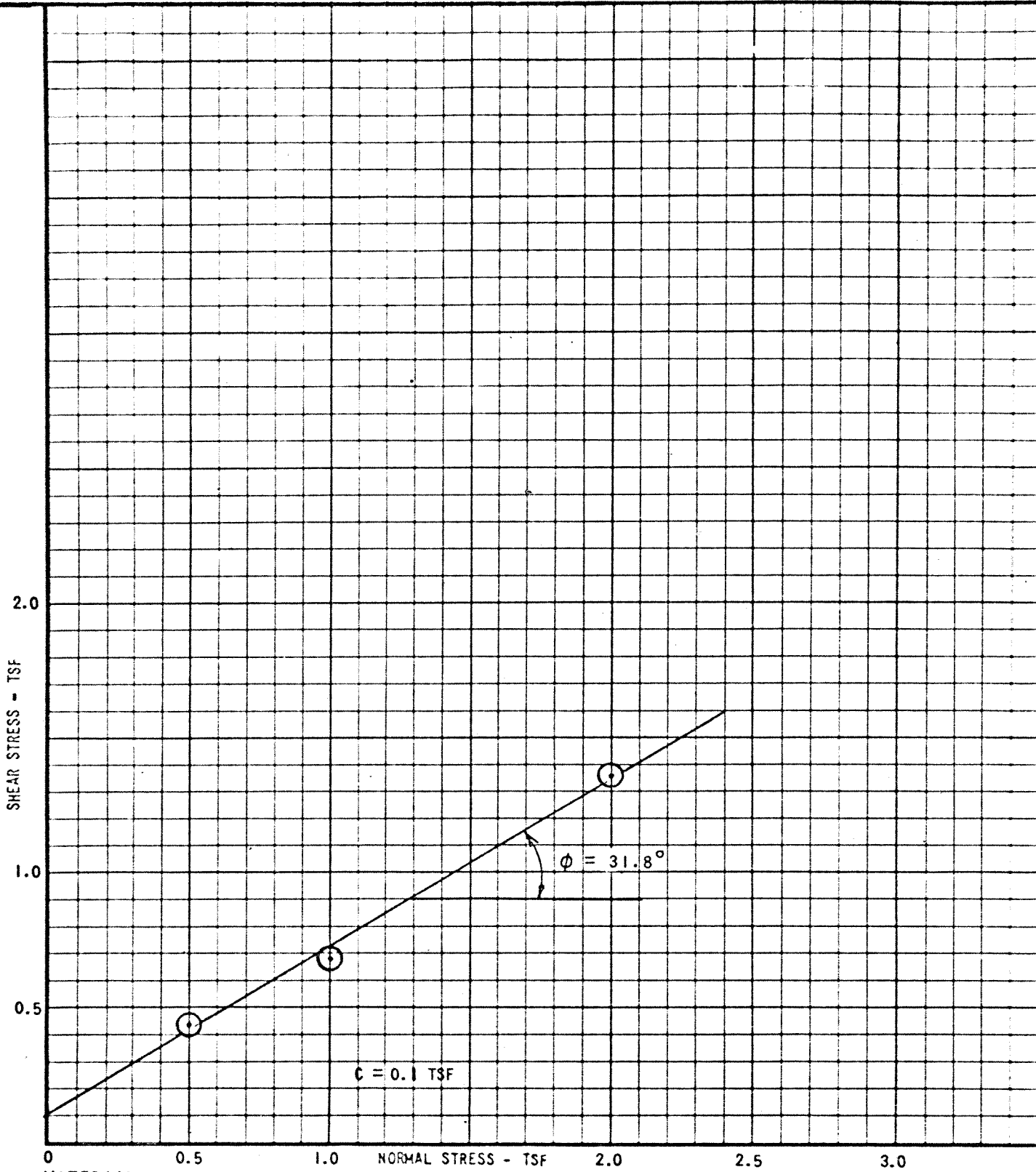
DESCRIPTION OF MATERIAL TESTED										SYMBOLS FOR TEST TYPE	
BROWN SILTY FINE TO MEDIUM SAND, TRACE COARSE SAND STRATUM (D)										Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST	
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %
						INITIAL	FINAL	INITIAL	FINAL		
(1)	58U	4U	SM	Q	0.5	17	17			2.84	6
(2)	58U	4U	SM	Q	1.0	18	18			2.55	9
(3)	58U	5U	SM	Q	2.0	15	15			4.22	5

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY			
DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS			
<b>MUESER · RUTLEDGE · WENTWORTH &amp; JOHNSTON</b> CONSULTING ENGINEERS			
415 MADISON AVE. NEW YORK, N.Y. 10017			
MADE BY: VLT	DATE: 10-25-74	FILE NO. 3291H	
CH'KD BY:	DATE:	PLATE NO. 19	
<b>SUMMARY OF STRENGTH TESTS</b>			
BORING AM-58U SAMPLE 4U & 5U			



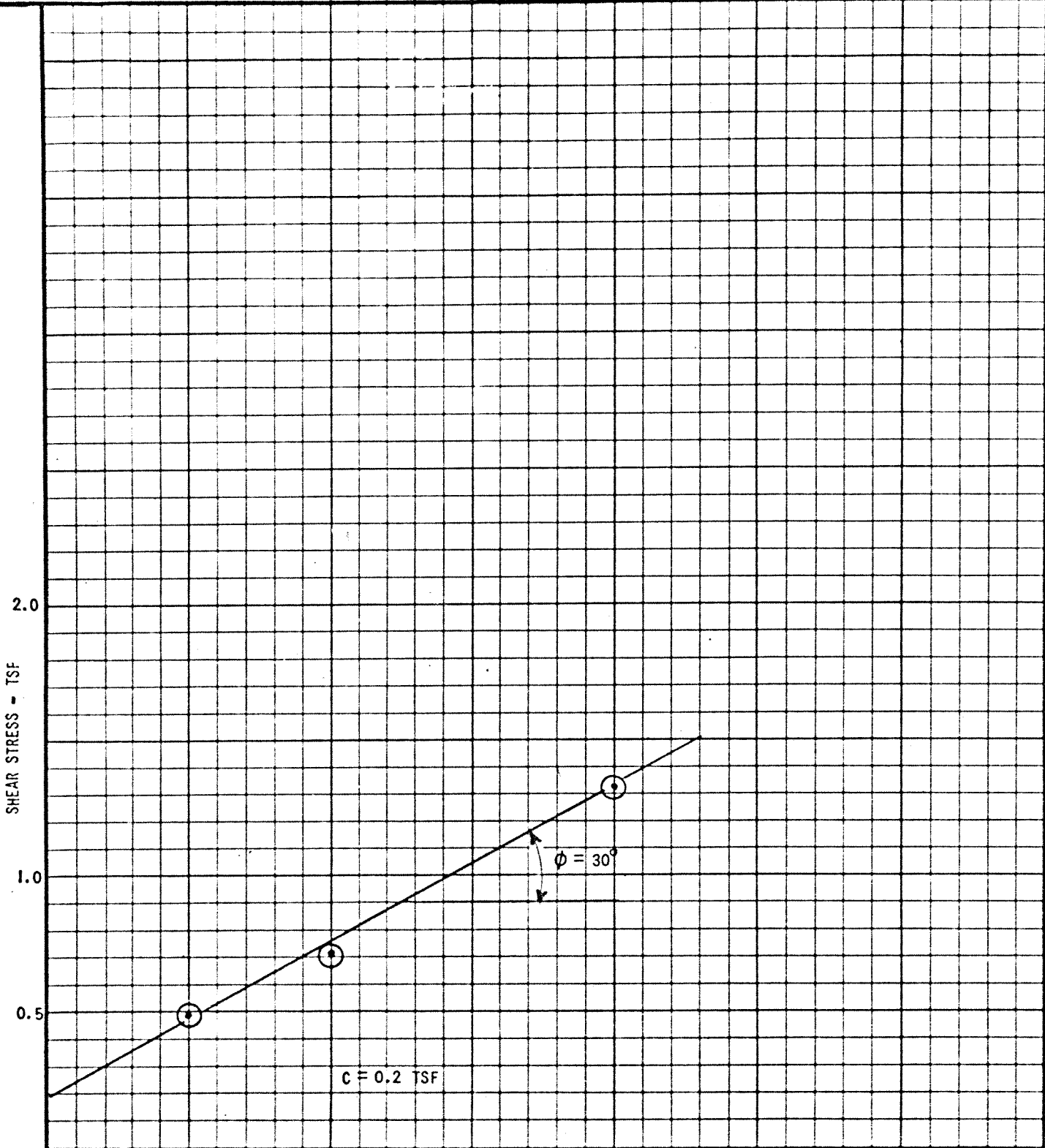
DESCRIPTION OF MATERIAL TESTED										SYMBOLS FOR TEST TYPE	
BROWN-GRAY SILTY FINE TO MEDIUM SAND TRACE COARSE SAND STRATUM (D)										Q - UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST	
KEY	BORING NO.	SAMPLE NO.	SOIL TYPE	TEST TYPE	LATERAL PRESSURE T.S.F.	WATER CONTENT - %		DEGREE OF SATURATION - %		DEVIATOR STRESS T.S.F.	AXIAL STRAIN %
					INITIAL	FINAL	INITIAL	FINAL			
(1)	59U	4U	SM	Q	0.5	18	18			1.64	4
(2)	59U	4U	SM	Q	1.0	20	20			3.16	5
(3)	59U	2U	SM	Q	2.0	12	12			3.75	9
WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY											
DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS											
MUESER RUTLEDGE WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK, N.Y. 10017											
MADE BY: AAK DATE: 8-15-74										FILE NO.	
CH'KD BY: VLT DATE: 10-16-75										3291H	
SUMMARY OF STRENGTH TESTS											PLATE NO.
BORING AM-59U SAMPLE 2U & 4U											20



MATERIAL: BROWN FINE SANDY SILT  
STRATUM (D)

BORING NUMBER	SAMPLE NUMBER	FINAL CONDITION			
		WATER CONTENT	PER CENT SATURATION	NORMAL STRESS TSF	SHEAR STRESS TSF
AM-3U	4U	25	100+	0.5	0.44
		29		1.0	0.68
		30		2.0	1.35

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
 DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANTS  
 MUESER RUTLEDGE WENTWORTH & JOHNSTON  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK 17, N.Y.  
 MADE BY: AAK DATE: 6-18-74 FILE NO.  
 CHECKED BY: VLT DATE: 10-9-74 3291H  
 DIRECT SHEAR TEST  
 BORING AM-3U SAMPLE 4U  
 PLATE NO. 21



0 0.5 1.0 2.0 2.5 3.0 NORMAL STRESS - TSF

MATERIAL: BROWN SILTY FINE SAND, TRACE COARSE SAND STRATUM (D)

FINAL CONDITION

BORING NUMBER	SAMPLE NUMBER	FINAL CONDITION			
		WATER CONTENT	PER CENT SATURATION	NORMAL STRESS TSF	SHEAR STRESS TSF
AM-3U	8U	31	100+	0.5	0.49
		30		1.0	0.71
		34		2.0	1.32

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DE LEUW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANTS

MUESER RUTLEDGE WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N.Y.

MADE BY: VLT DATE: 10-9-74

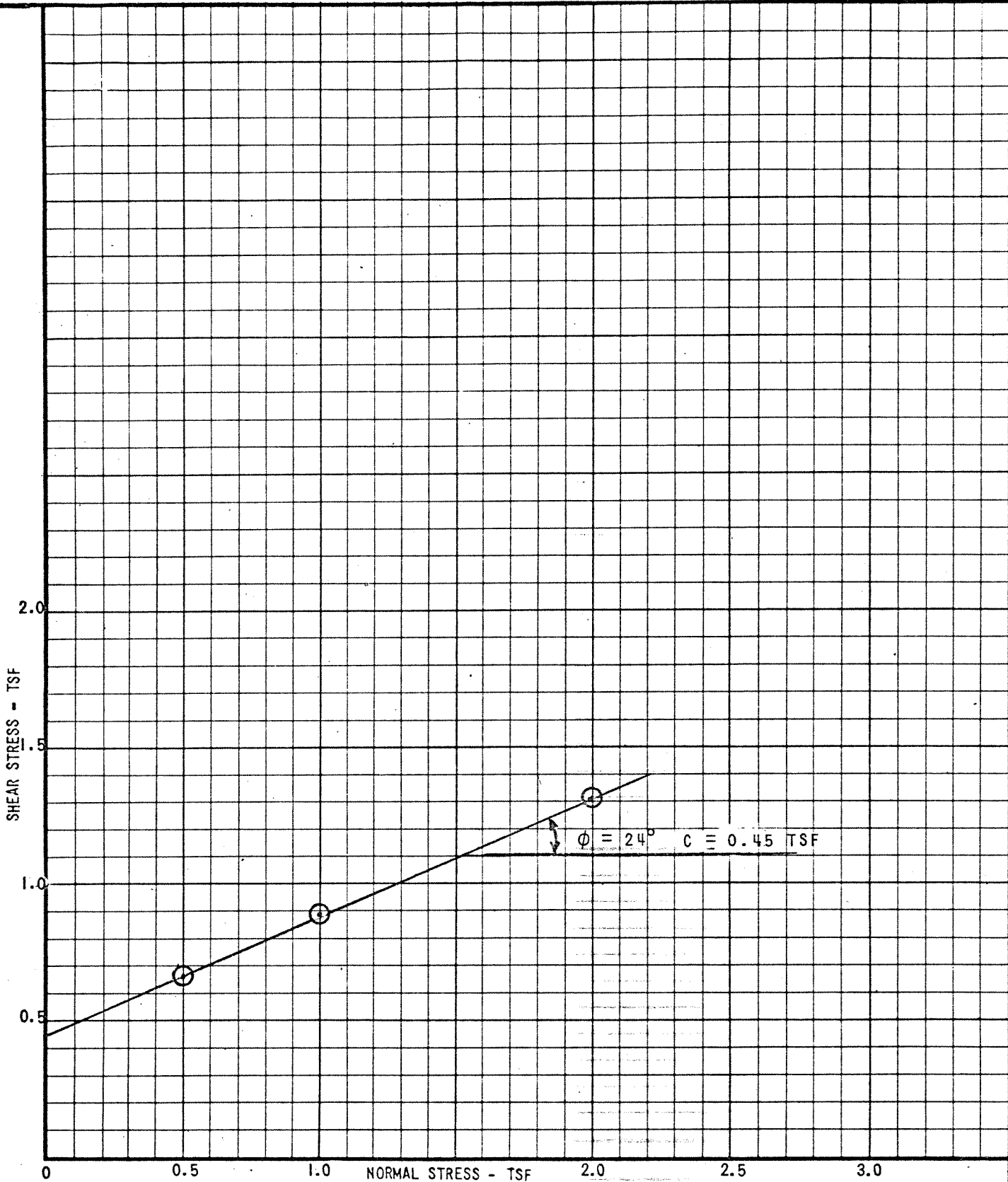
CHECKED BY: DATE:

DIRECT SHEAR TEST

BORING AM-3U SAMPLE 8U

FILE NO. 3291H

PLATE NO. 22



MATERIAL: BROWN SILTY FINE SAND, TRACE MEDIUM TO COARSE SAND STRATUM D

BORING NUMBER	SAMPLE NUMBER	FINAL CONDITION			
		WATER CONTENT	PER CENT SATURATION	NORMAL STRESS TSF	SHEAR STRESS TSF
AM-6	7U	28	100+	0.5	0.67
		27		1.0	0.89
		27		2.0	1.31

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

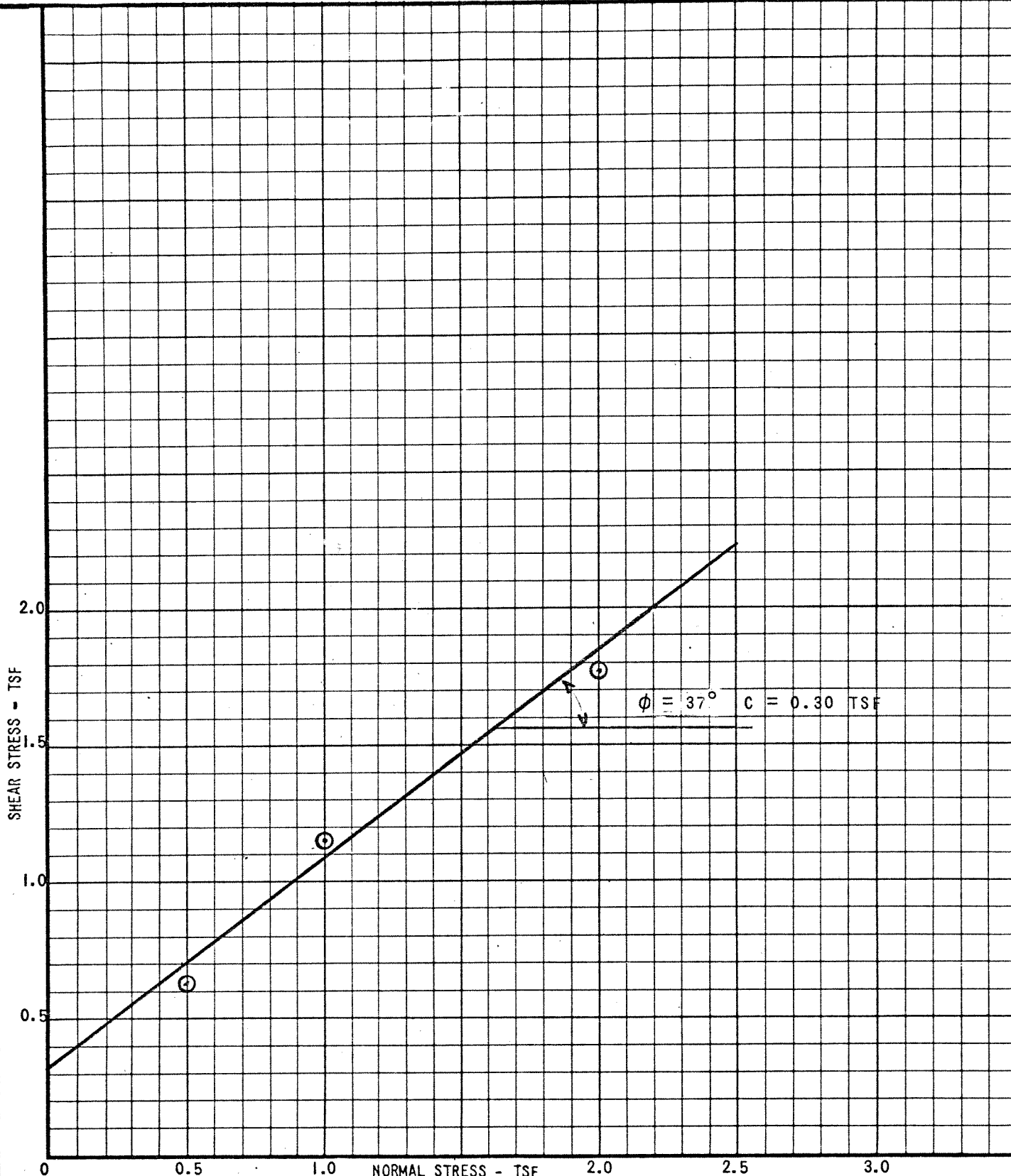
DE LEUW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANTS

MUESER RUTLEDGE WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N.Y.

MADE BY: AAK DATE: 6-4-74 FILE NO. 3291H  
CHK'D BY: VLT DATE: 6-6-74

DIRECT SHEAR TEST  
BORING AM-6 SAMPLE 7U

PLATE NO. 23



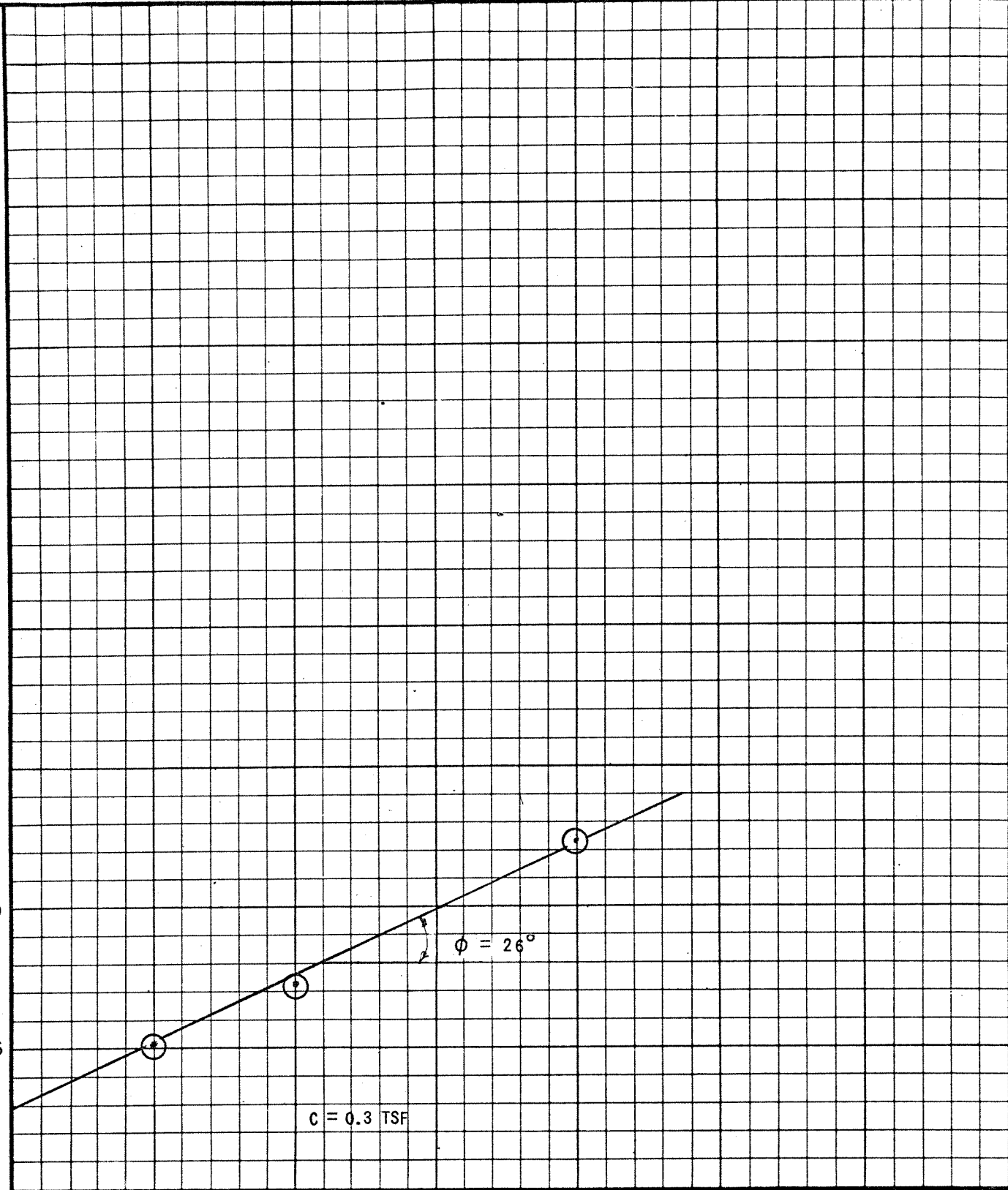
MATERIAL: BROWN FINE TO MEDIUM SAND, SOME SILT  
TRACE COARSE SAND STRATUM (D)

BORING NUMBER	SAMPLE NUMBER	FINAL CONDITION			
		WATER CONTENT	PER CENT SATURATION	NORMAL STRESS TSF	SHEAR STRESS TSF
AM-9U	2U	24	100+	0.5	0.63
		27		1.0	1.15
		25		2.0	1.77

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
 DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANTS  
**MUESER RUTLEDGE WENTWORTH & JOHNSTON**  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK 17, N.Y.  
 MADE BY: AAK DATE: 6-5-74 FILE NO. 329 IH  
 CH'KD BY: VLT DATE: 6-6-74  
**DIRECT SHEAR TEST**  
 BORING AM-9U SAMPLE 2U  
 PLATE NO. **24**

SHEAR STRESS - TSF

2.0  
1.0  
0.5



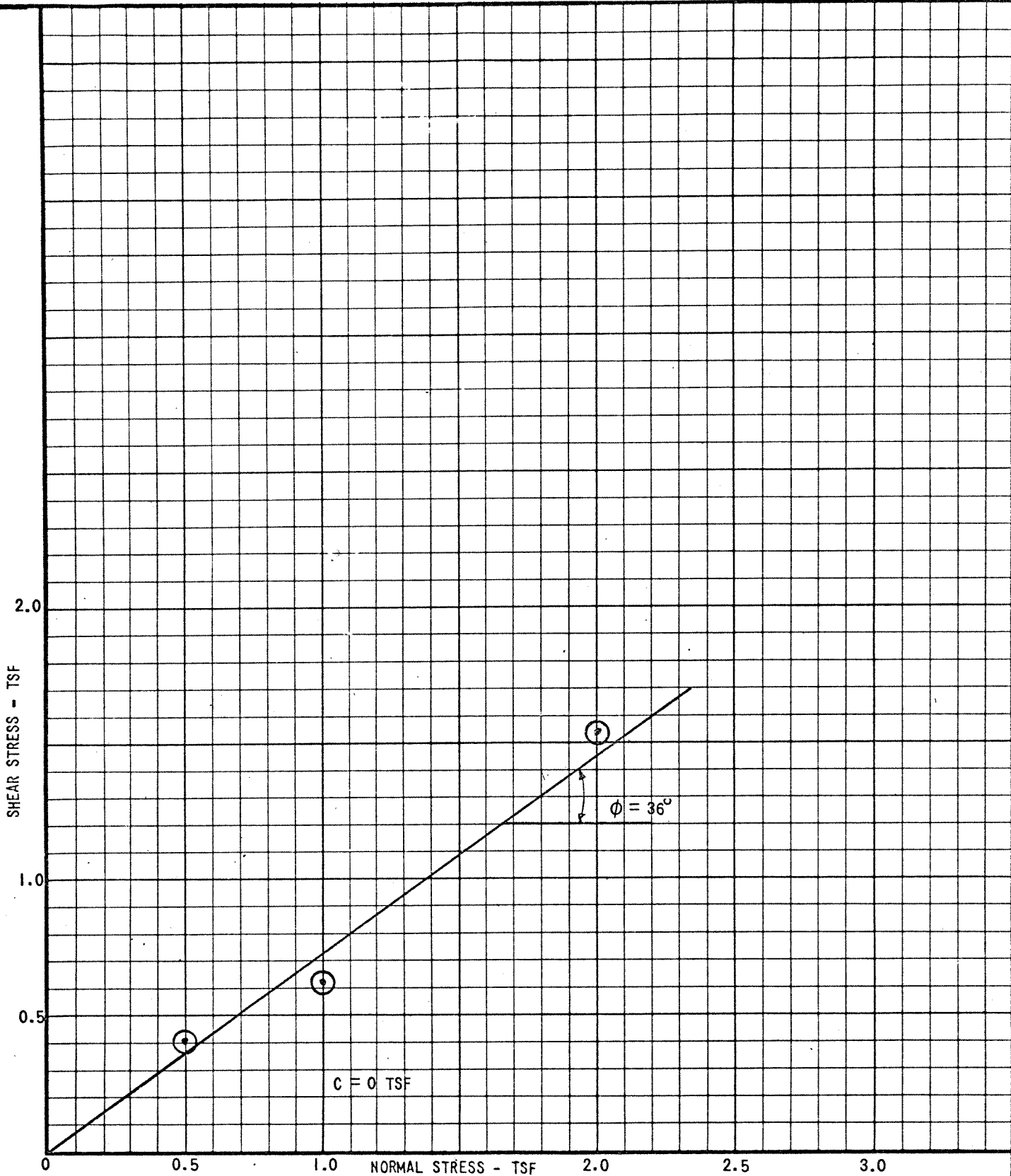
0 0.5 1.0 2.0 2.5 3.0 NORMAL STRESS - TSF

MATERIAL: RED FINE SANDY SILT  
STRATUM (D)

BORING NUMBER	SAMPLE NUMBER	FINAL CONDITION			
		WATER CONTENT	PER CENT SATURATION	NORMAL STRESS TSF	SHEAR STRESS TSF
AM-44U	4U	46	100+	0.5	0.51
		43		1.0	0.73
		42		2.0	1.24

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
 DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANTS  
**MUESER RUTLEDGE WENTWORTH & JOHNSTON**  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK 17, N.Y.  
 MADE BY: VLT DATE: 10-8-74 FILE NO. 3291H  
 CHECKED BY: DATE: PLATE NO. 25  
 DIRECT SHEAR TEST  
 BORING AM-44U SAMPLE 4U





MATERIAL: BROWN AND GRAY SILTY FINE TO MEDIUM SAND, TRACE COARSE SAND STRATUM (D)

BORING NUMBER	SAMPLE NUMBER	FINAL CONDITION			
		WATER CONTENT	PER CENT SATURATION	NORMAL STRESS TSF	SHEAR STRESS TSF
AM-58U	4U	27	100+	0.5	0.41
	4U	25		1.0	0.63
	5U	30		2.0	1.55

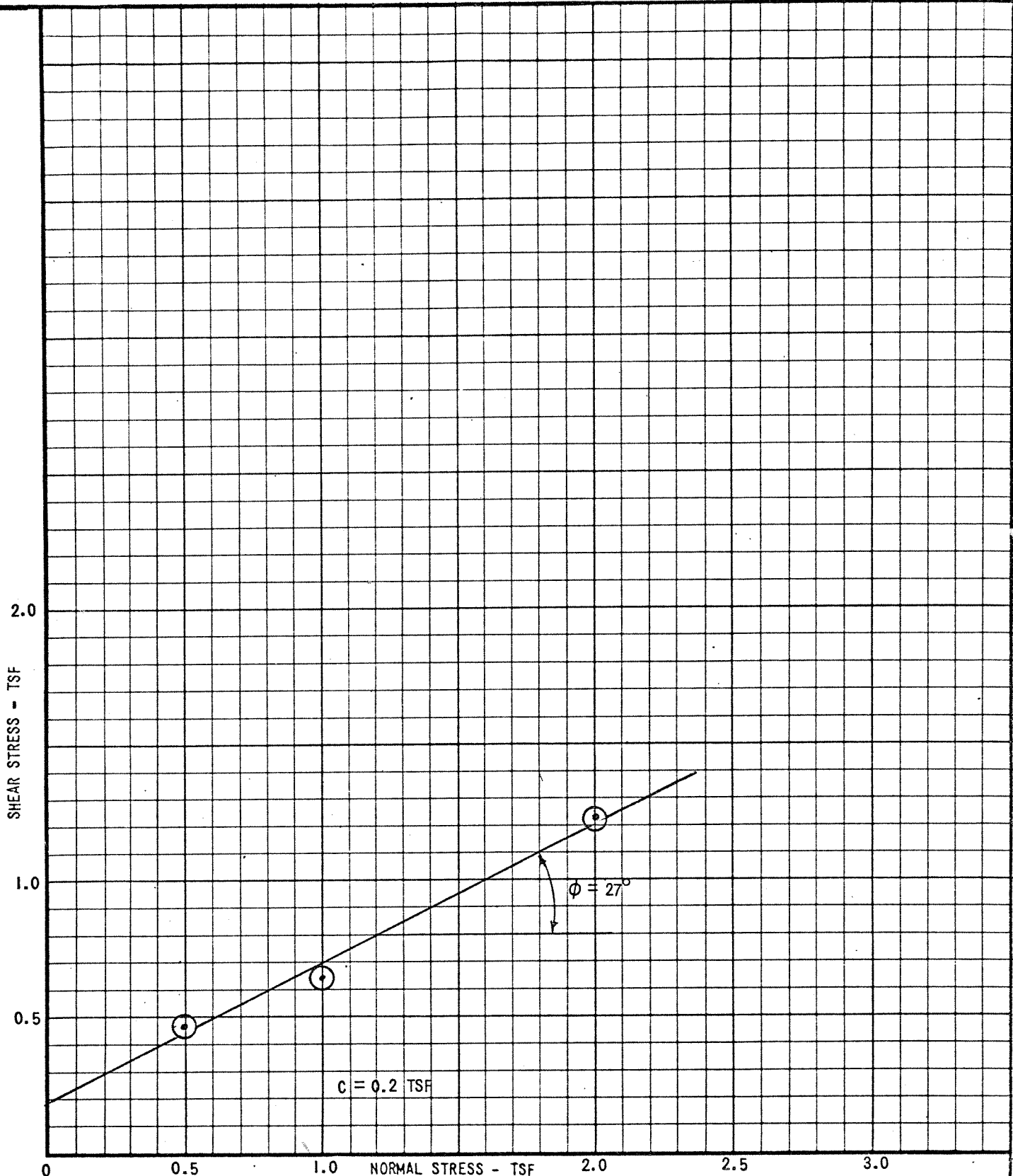
WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
 DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANTS

MUESER RUTLEDGE WENTWORTH & JOHNSTON  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK 17, N.Y.

MADE BY: KSW DATE: 8-9-74 FILE NO. 3291H  
 CH'KD BY: VLT DATE: 10-9-74

DIRECT SHEAR TEST  
 BORING AM-58U SAMPLES 4U & 5U

PLATE NO. 26



MATERIAL: BROWN SILTY FINE SAND

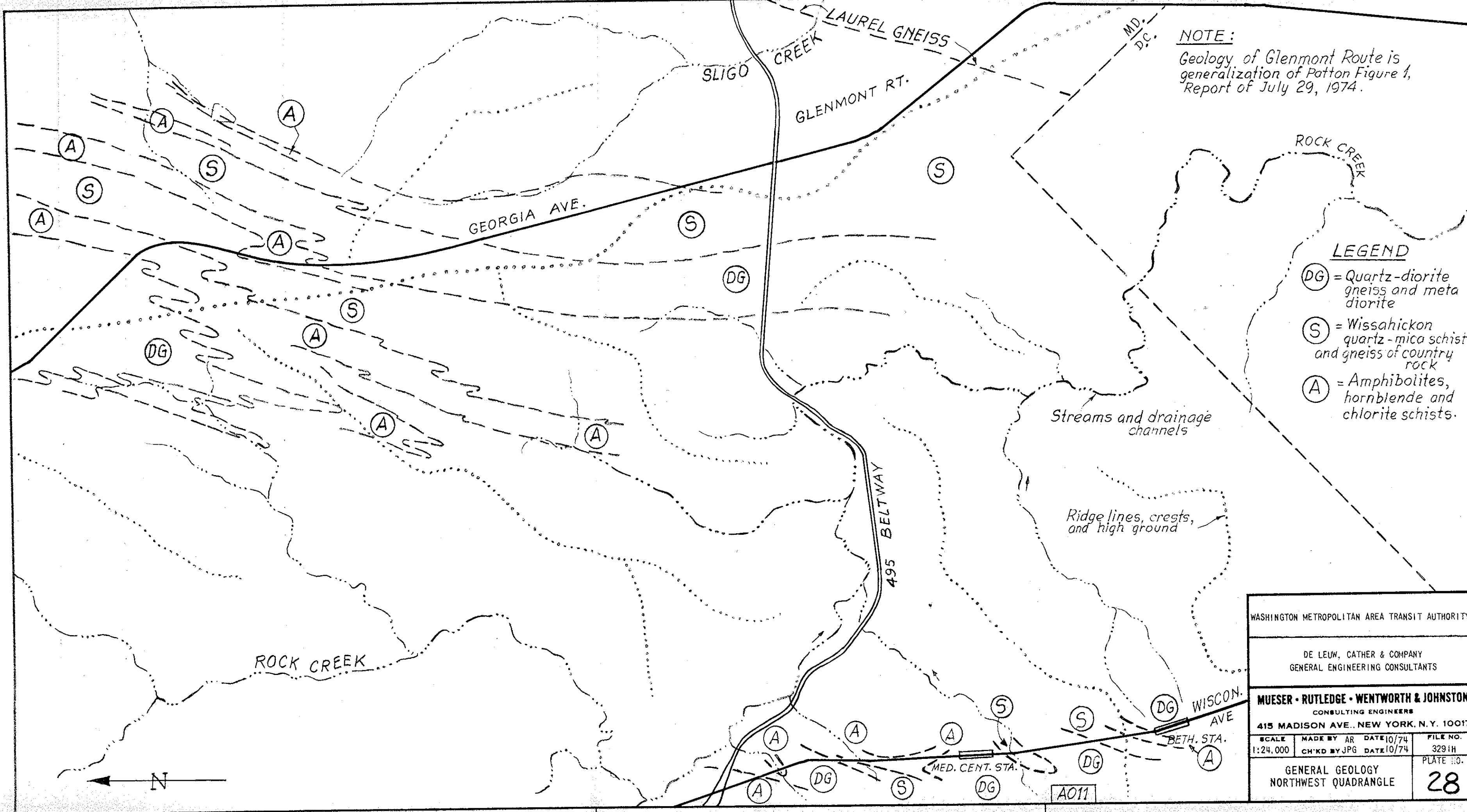
STRATUM (D)

BORING NUMBER	SAMPLE NUMBER	FINAL CONDITION			
		WATER CONTENT	PER CENT SATURATION	NORMAL STRESS TSF	SHEAR STRESS TSF
AM-59U	2U	35	95	0.5	0.48
		34		1.0	0.65
		35		2.0	1.23

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
 DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANTS

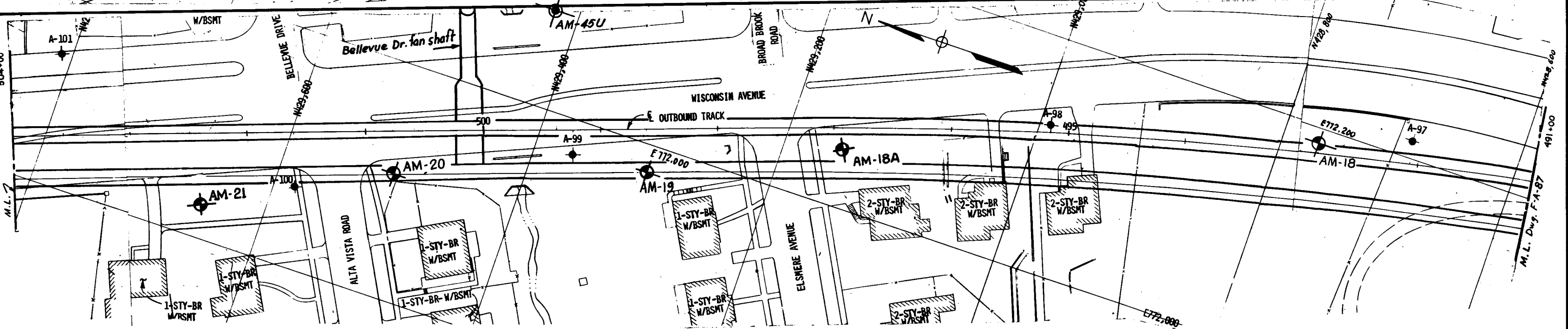
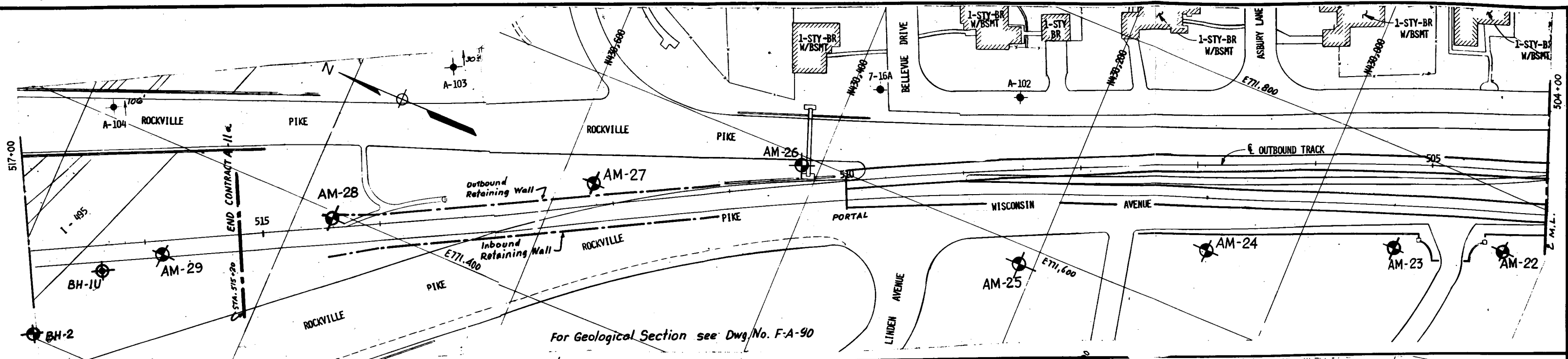
MUESER RUTLEDGE WENTWORTH & JOHNSTON  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK 17, N.Y.

MADE BY: VLT DATE: 10-9-74 FILE NO. 3291H  
 CHKD BY: DATE: DIRECT SHEAR TEST PLATE NO. 27  
 BORING AM-59U SAMPLE 2U



WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY			
DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANTS			
MUESER • RUTLEDGE • WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK, N. Y. 10017			
SCALE 1:24,000	MADE BY AR	DATE 10/74	FILE NO. 3291H
	CH'KD BY JPG	DATE 10/74	PLATE NO. 28
GENERAL GEOLOGY NORTHWEST QUADRANGLE			

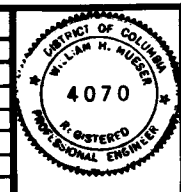




**BORING LEGEND**

- "AM" - Dry sample borings made for this investigation.
- "AM" - Undisturbed sample borings made for this investigation.
- "A" - Series borings, see MRWJ Report No. 58, March 1972.
- "7" - Series borings, see MRWJ Report No. 33, February 1969.
- BH - Series borings made for Section A013 to be shown in later Report.

DESIGNED		DATE		REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY	NUMBER	DESCRIPTION	DATE	BY
AR	General Notes and Legend	6-74		F-1			
<p>These Drawings and the stationing were provided by the Section Designer.</p>							



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER • RUTLEDGE • WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N. Y.

SUBMITTED *William M. Hiesler*

DE LEUW, CATHER & COMPANY   
GENERAL ENGINEERING CONSULTANT

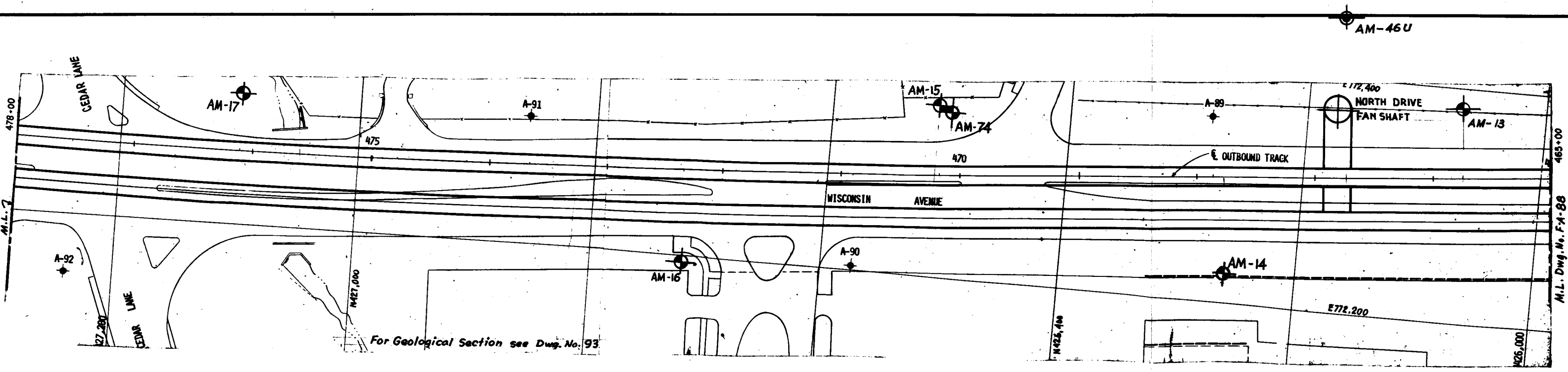
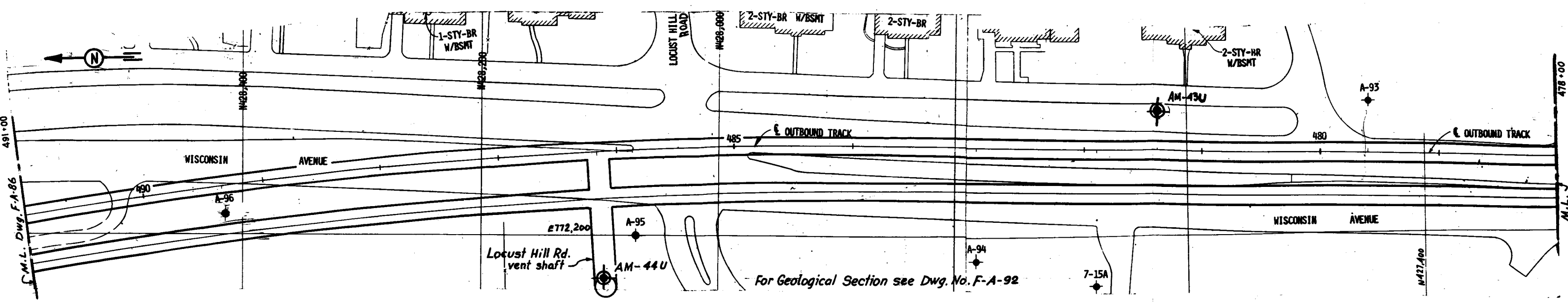
HARRY WEESE & ASSOCIATES   
GENERAL ARCHITECTURAL CONSULTANT

APPROVED \_\_\_\_\_

**ROCKVILLE ROUTE (A011)**  
**BORING LOCATION PLAN**  
STA. 491+00 to 517+00

SCALE: 1" = 50'

DRAWING NO. **F-A-86**



DESIGNED		DATE		REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY	NUMBER	DESCRIPTION	DATE	BY
AR	6-74	F-1		General Notes and Legend			
SLT. & JPS.	18-74						



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER - RUTLEDGE - WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N. Y.

SUBMITTED *William H. Mueser*

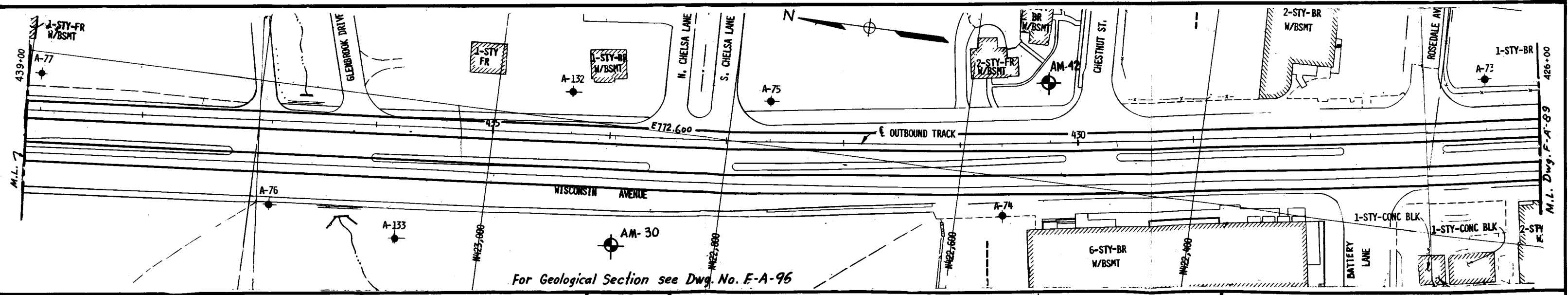
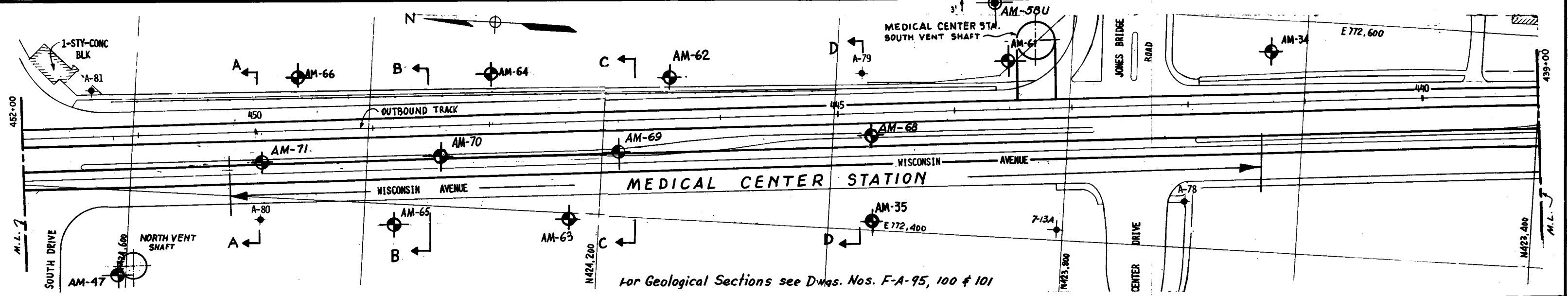
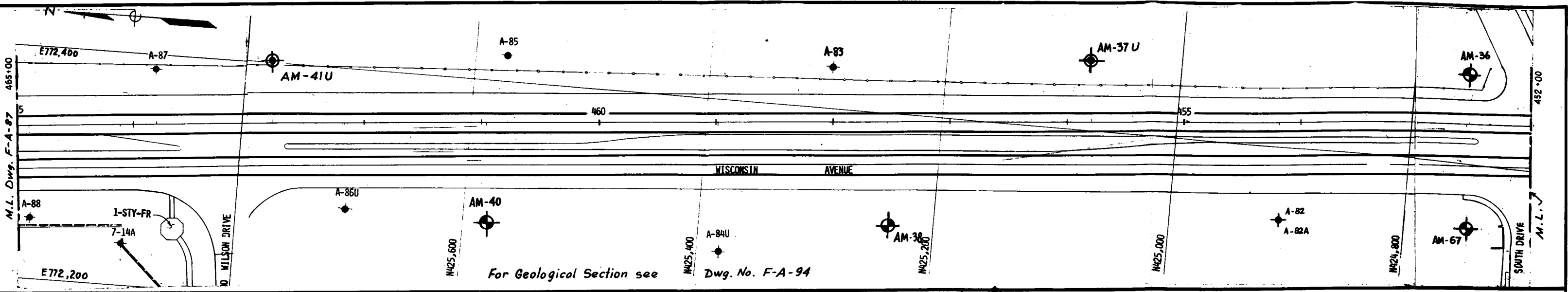
DE LEUW, CATHER & COMPANY   
GENERAL ENGINEERING CONSULTANT

HARRY WEESE & ASSOCIATES   
GENERAL ARCHITECTURAL CONSULTANT

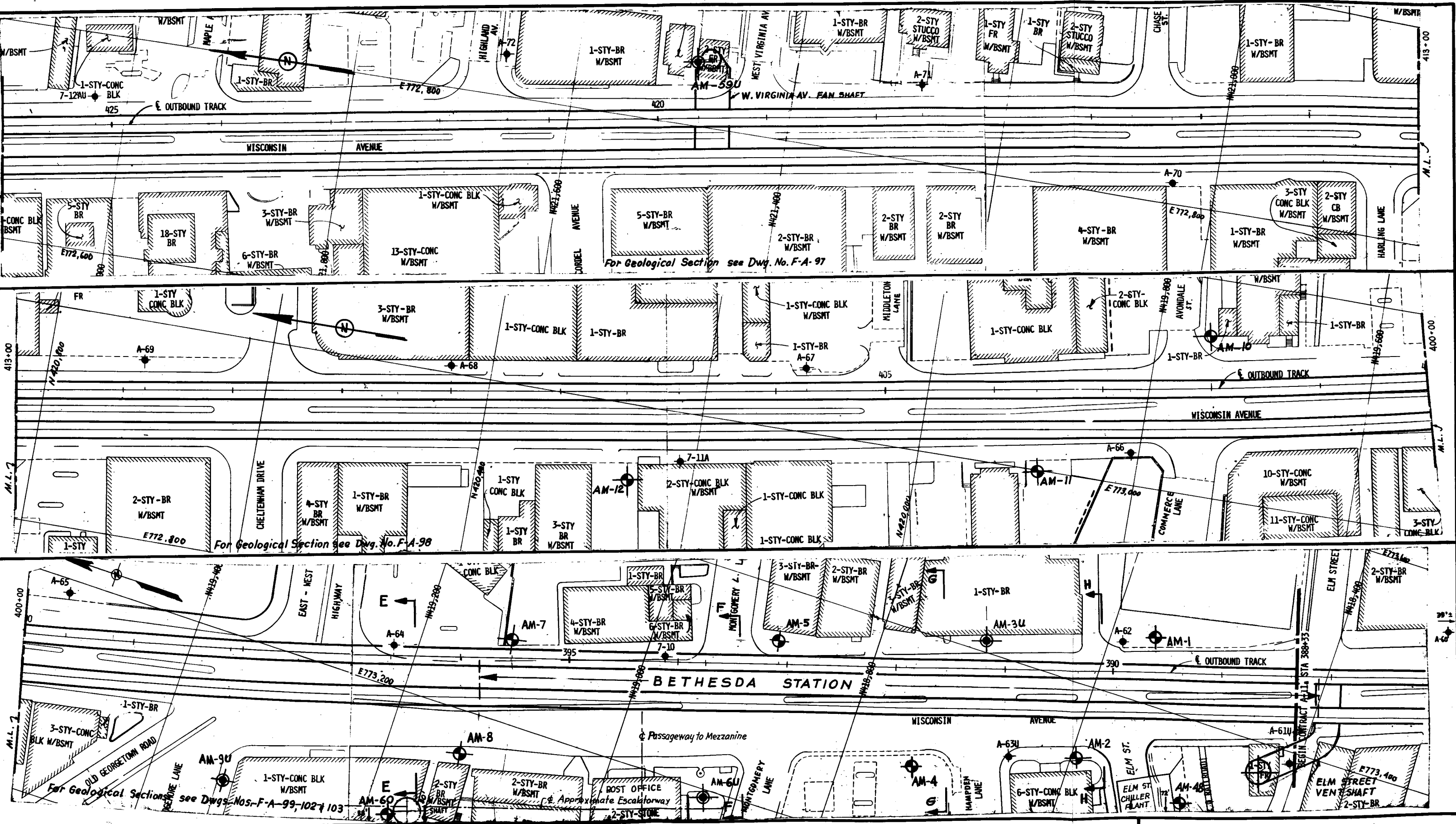
**ROCKVILLE ROUTE (A011)**  
**BORING LOCATION PLAN**  
STA. 465+00 to 491+00

SCALE

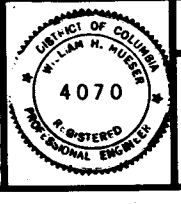
DRAWING NO. **F-A-87**



DESIGNED DRAWN CHECKED APPROVED	AR 6-74 SLT. & JPG. 12-74	<b>REFERENCE DRAWINGS</b> NUMBER DESCRIPTION F-1 General Notes and Legend	<b>REVISIONS</b> DATE BY DESCRIPTION		<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> MUESER • RUTLEDGE • WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y. SUBMITTED <i>William H. Mueser</i>	DE LEUW, CATHAR & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT	<b>ROCKVILLE ROUTE (AO11) BORING LOCATION PLAN</b> STA. 426+00 to 465+00 SCALE: 1" = 20' DRAWING NO. F-A-88
--	---------------------------------	---	---	--	---	---	--



DESIGNED	DATE	NUMBER	REFERENCE DRAWINGS	DESCRIPTION	DATE	BY	REVISIONS	DESCRIPTION
AR	6-74	F-1	General Notes and Legend					
DRAWN								
CHECKED	SLT. & JPG	12-74						
APPROVED								

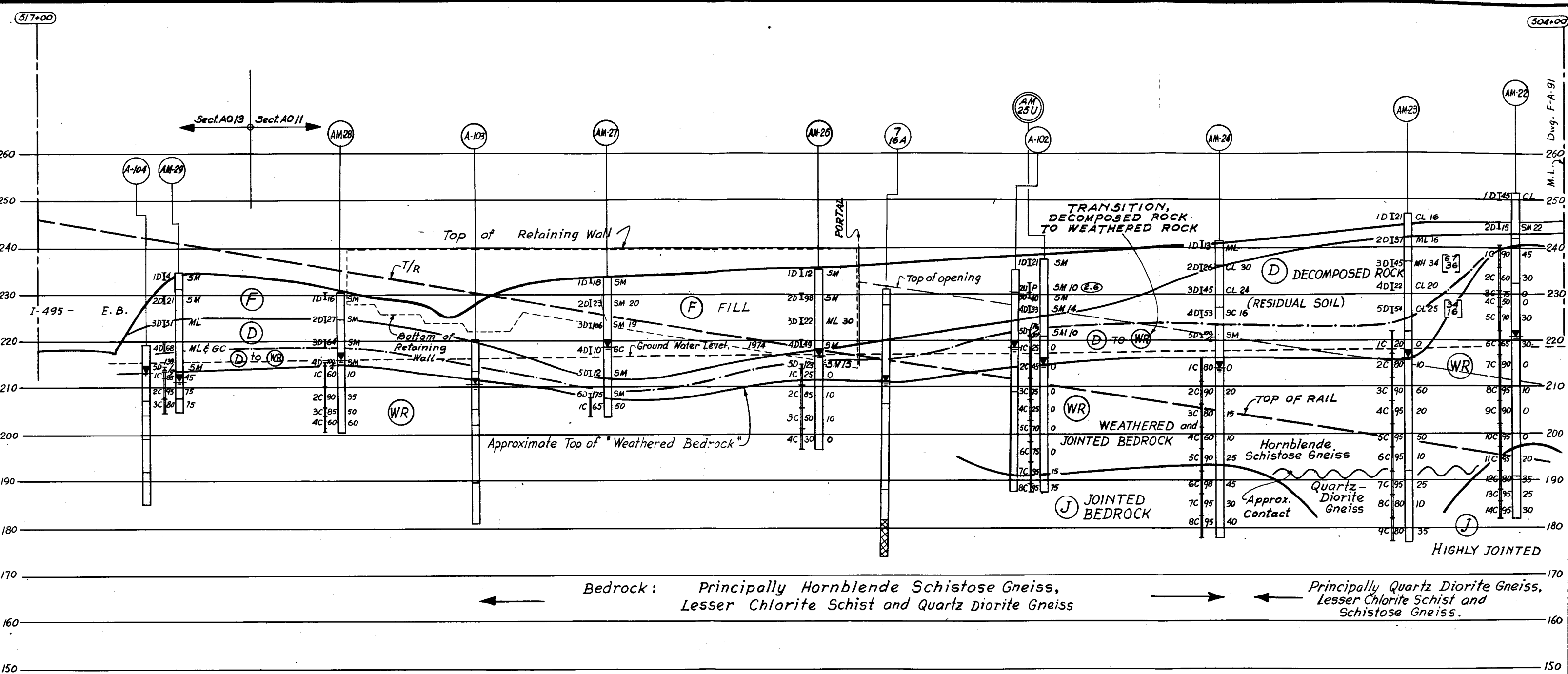


**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**  
**MUESER • RUTLEDGE • WENTWORTH & JOHNSTON**  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK 17, N.Y.

DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANT  
 HARRY WEESE & ASSOCIATES  
 GENERAL ARCHITECTURAL CONSULTANT

**ROCKVILLE ROUTE (A011)**  
**BORING LOCATION PLAN**  
 STA. 388+33 to 426+00  
 SCALE: 1" = 20'  
 DRAWING NO. **F-A-89**





Bedrock: Principally Hornblende Schistose Gneiss, Lesser Chlorite Schist and Quartz Diorite Gneiss

Principally Quartz Diorite Gneiss, Lesser Chlorite Schist and Schistose Gneiss.

Note: For detailed information on logs of borings of A-Series see MRWJ Report No.58, March 1972; of 7-Series see MRWJ Report No.33, February 1969.

REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY
AR & IHL 12-73	F-1 General Notes and Legend		
SLT & JPG 12-74	F-A-86 Boring Location Plan		



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER - RUTLEDGE - WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N. Y.

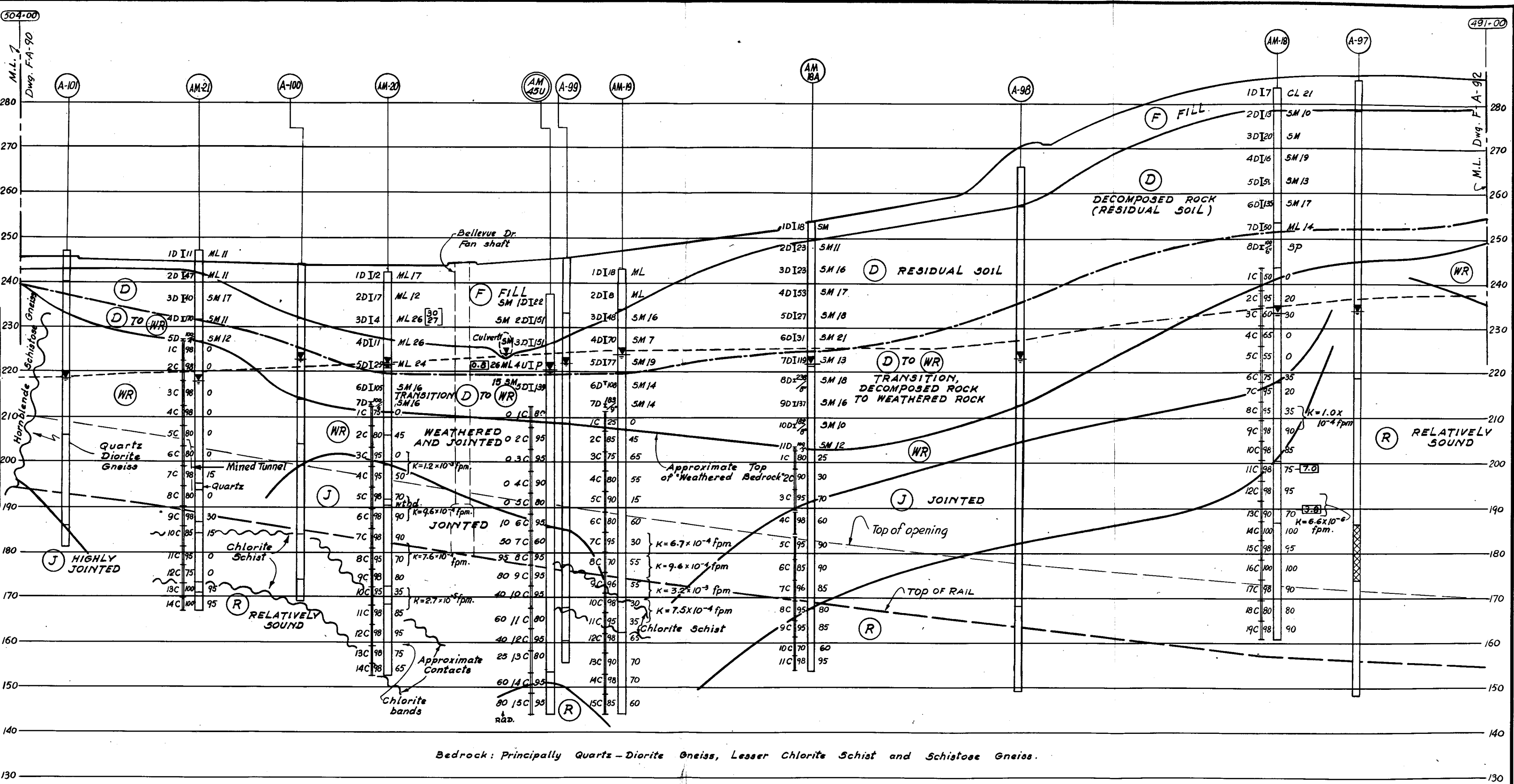
DE LEUW, CATHY & COMPANY  
GENERAL ENGINEERING CONSULTANT

HARRY WEISE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

**ROCKVILLE ROUTE (A011)**  
**GEOLOGICAL SECTION**  
**STATION 504+00 to 517+00**

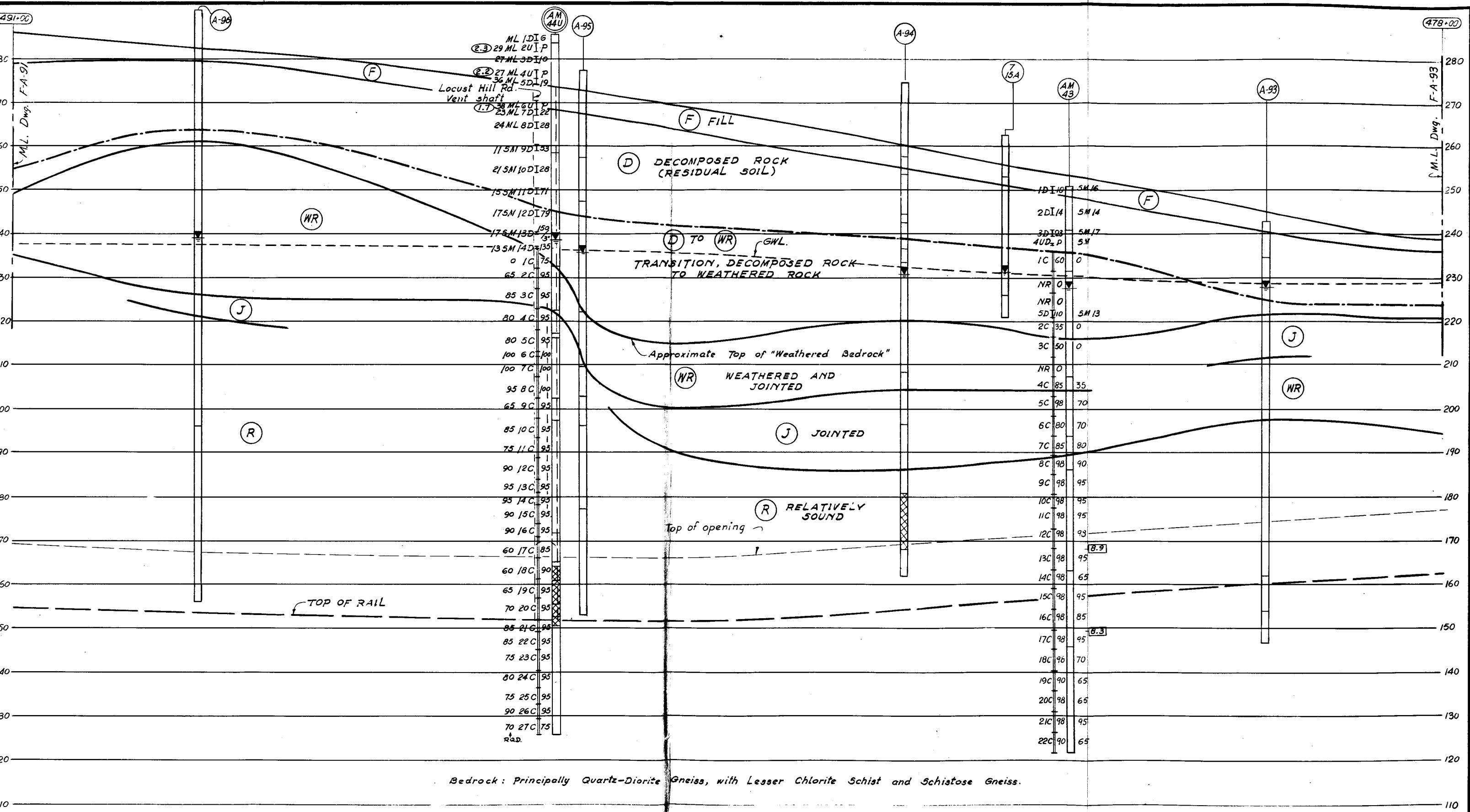
SCALE  
HORIZ. 1" = 20'  
VERT. 1" = 10'

DRAWING NO. **F-A-90**



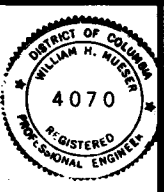
Bedrock: Principally Quartz-Diorite Gneiss, Lesser Chlorite Schist and Schistose Gneiss.

DESIGNED DRAWN CHECKED APPROVED	DATE 12-73 DATE 12-74 DATE	<b>REFERENCE DRAWINGS</b> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>NUMBER</th> <th>DESCRIPTION</th> </tr> <tr> <td>F-1</td> <td>General Notes and Legend</td> </tr> <tr> <td>F-A-86</td> <td>Boring Location Plan</td> </tr> </table>	NUMBER	DESCRIPTION	F-1	General Notes and Legend	F-A-86	Boring Location Plan	<b>REVISIONS</b> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>DATE</th> <th>BY</th> <th>DESCRIPTION</th> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> </table>	DATE	BY	DESCRIPTION					<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> <b>MUESER - RUTLEDGE - WENTWORTH &amp; JOHNSTON</b> CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y. SUBMITTED <i>William H. Hulse</i>	<b>DE LEUW, CATHAR &amp; COMPANY</b> GENERAL ENGINEERING CONSULTANT <input type="checkbox"/> <b>HARRY WEESE &amp; ASSOCIATES</b> GENERAL ARCHITECTURAL CONSULTANT <input type="checkbox"/>	<b>ROCKVILLE ROUTE (AO11)</b> <b>GEOLOGICAL SECTION</b> <b>STATION 491+00 to 504+00</b> SCALE HORIZ. 0 20' 40' 80' VERT. 0 5' 10' 20' DRAWING NO. <b>F-A-91</b>
NUMBER	DESCRIPTION																		
F-1	General Notes and Legend																		
F-A-86	Boring Location Plan																		
DATE	BY	DESCRIPTION																	



Bedrock: Principally Quartz-Diorite Gneiss, with Lesser Chlorite Schist and Schistose Gneiss.

REFERENCE DRAWINGS		REVISIONS	
DATE	NUMBER	DATE	DESCRIPTION
8-74	F-1		General Note: and Loc...
12-74	F-A-97		Boring Log - Loc Hill



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER - RUTLEDGE - WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N. Y.

SUBMITTED *William H. Mueser*

DE LEUW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANT

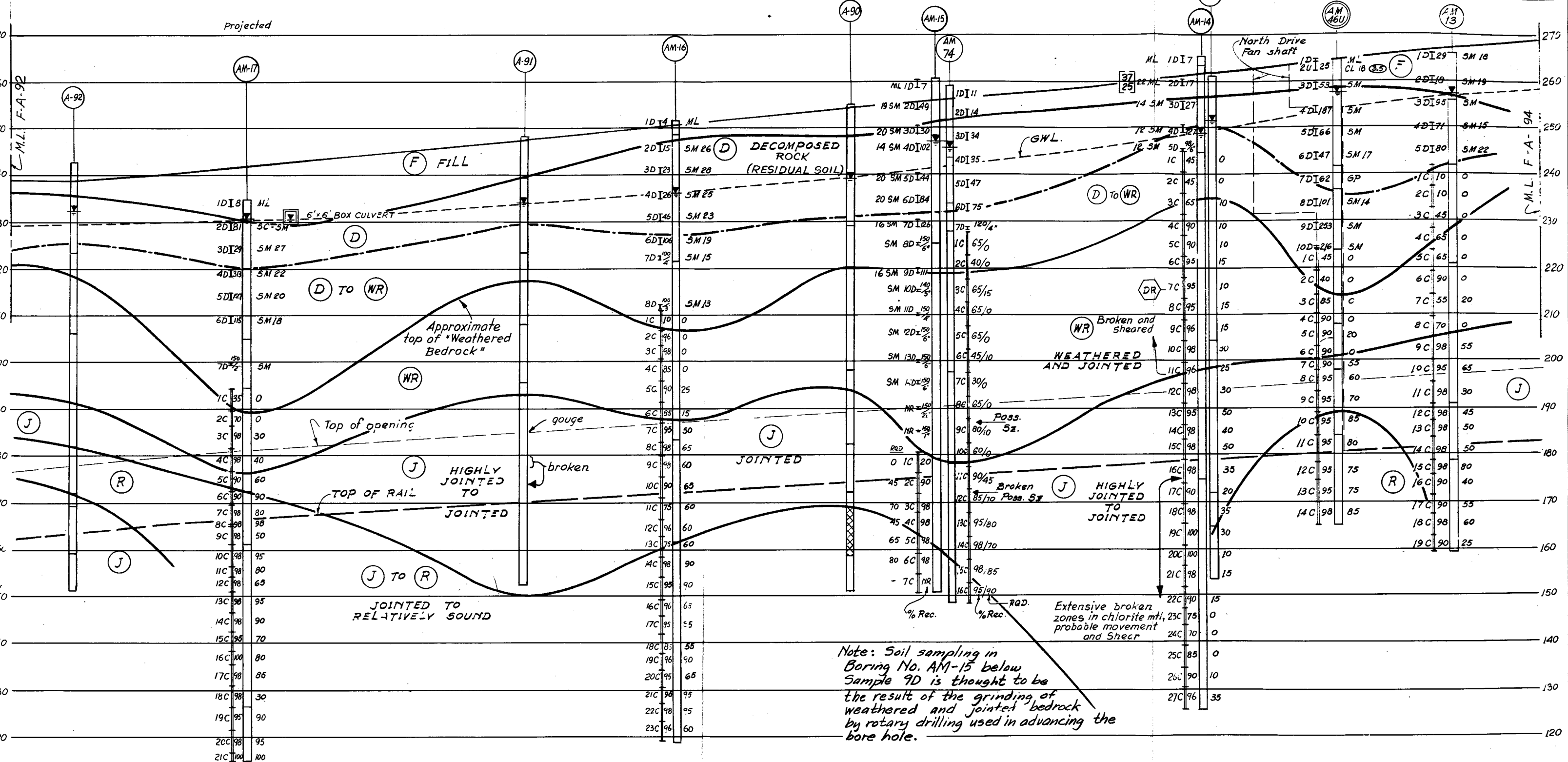
HARRY WESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

APPROVED

**ROCKVILLE ROUTE (A011)  
GEOLOGICAL SECTION  
STATION 478+00 to 491+00**

SCALE: HORIZ. 0" = 20' 40' 80'  
VERT. 0" = 3' 10' 20'

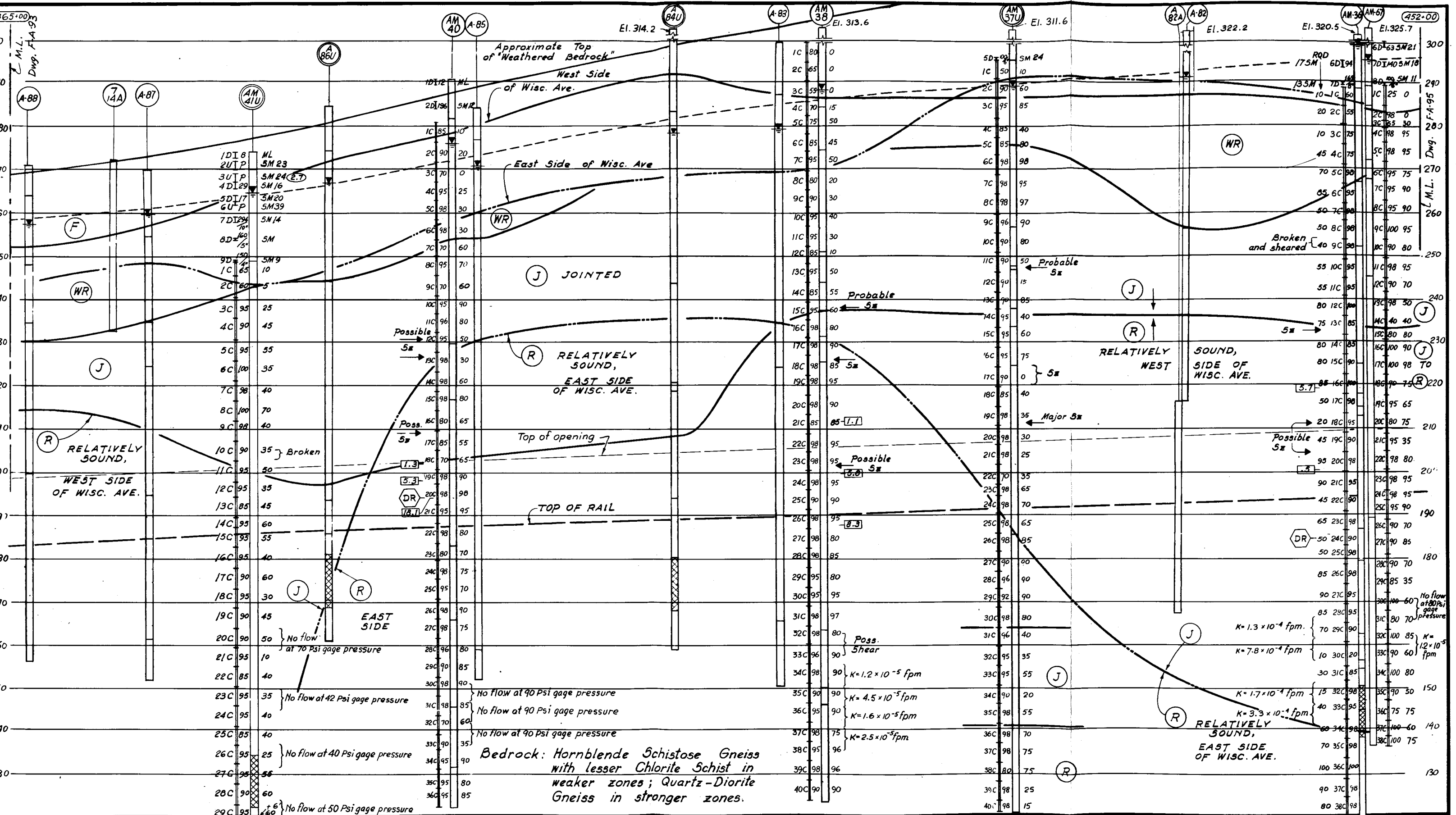
DRAWING NO. **F-A-92**



Note: Soil sampling in Boring No. AM-15 below Sample 7D is thought to be the result of the grinding of weathered and jointed bedrock by rotary drilling used in advancing the bore hole.

Bedrock: Principally Quartz-Hornblende Diorite and Diorite Gneiss with Lesser Chlorite Schist.

DESIGNED AR. & AG. 8-74 CHECKED SLT. & JPG. 12-74 APPROVED DATE	<b>REFERENCE DRAWINGS</b> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>NUMBER</th> <th>DESCRIPTION</th> </tr> <tr> <td>F-1</td> <td>General Notes and Legend</td> </tr> <tr> <td>F-A-87</td> <td>Boring Location Plan</td> </tr> </table>	NUMBER	DESCRIPTION	F-1	General Notes and Legend	F-A-87	Boring Location Plan	<b>REVISIONS</b> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>DATE</th> <th>BY</th> <th>DESCRIPTION</th> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> </table>	DATE	BY	DESCRIPTION					<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> <b>MUESER • RUTLEDGE • WENTWORTH &amp; JOHNSTON</b> CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y. SUBMITTED <i>William H. Mueser</i>	DE LEIJW, CATHIER & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT APPROVED	<b>ROCKVILLE ROUTE (A011)</b> <b>GEOLOGICAL SECTION</b> STATION 465+00 to 478+00 SCALE HORIZ. 1" = 40' VERT. 1" = 10' DRAWING NO. <b>F-A-93</b>
NUMBER	DESCRIPTION																	
F-1	General Notes and Legend																	
F-A-87	Boring Location Plan																	
DATE	BY	DESCRIPTION																



REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	DESCRIPTION
AR 6AG	8-74		
SLT. & JPB	12-74		

**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

MUESER - RUTLEDGE - WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N. Y.

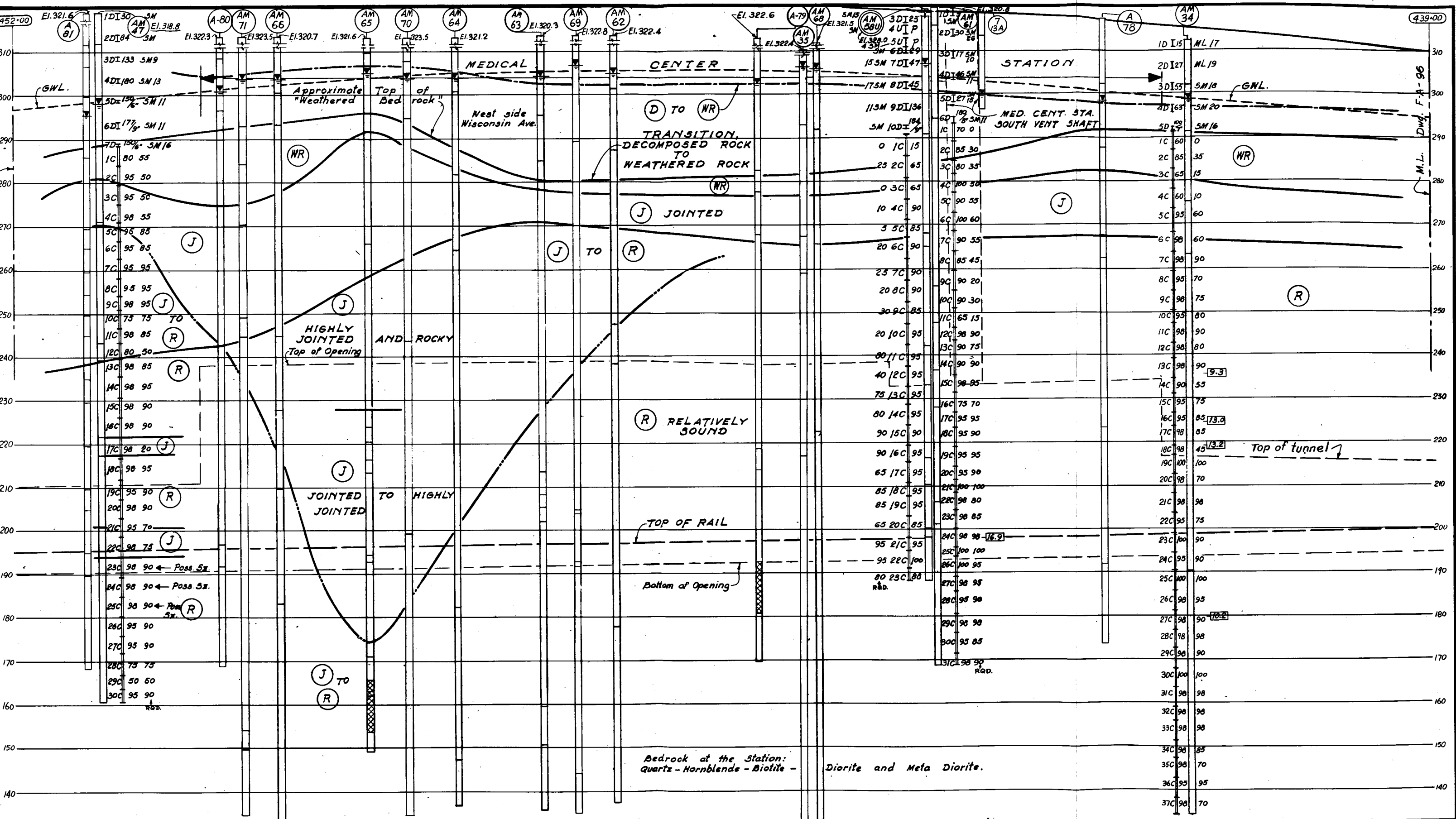
DE LEUW, CATHAR & COMPANY  
GENERAL ENGINEERING CONSULTANT

HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

**ROCKVILLE ROUTE (A011)**  
**GEOLOGICAL SECTION**  
**STATION 452+00 to 465+00**

SCALE: HORIZ. 1" = 20', VERT. 1" = 10'

DRAWING NO. **F-A-94**



REFERENCE DRAW		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY
F-1	General Notes and Legend		
F-A-88	Boring Location Plan		



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER • RUTLEDGE • WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N. Y.

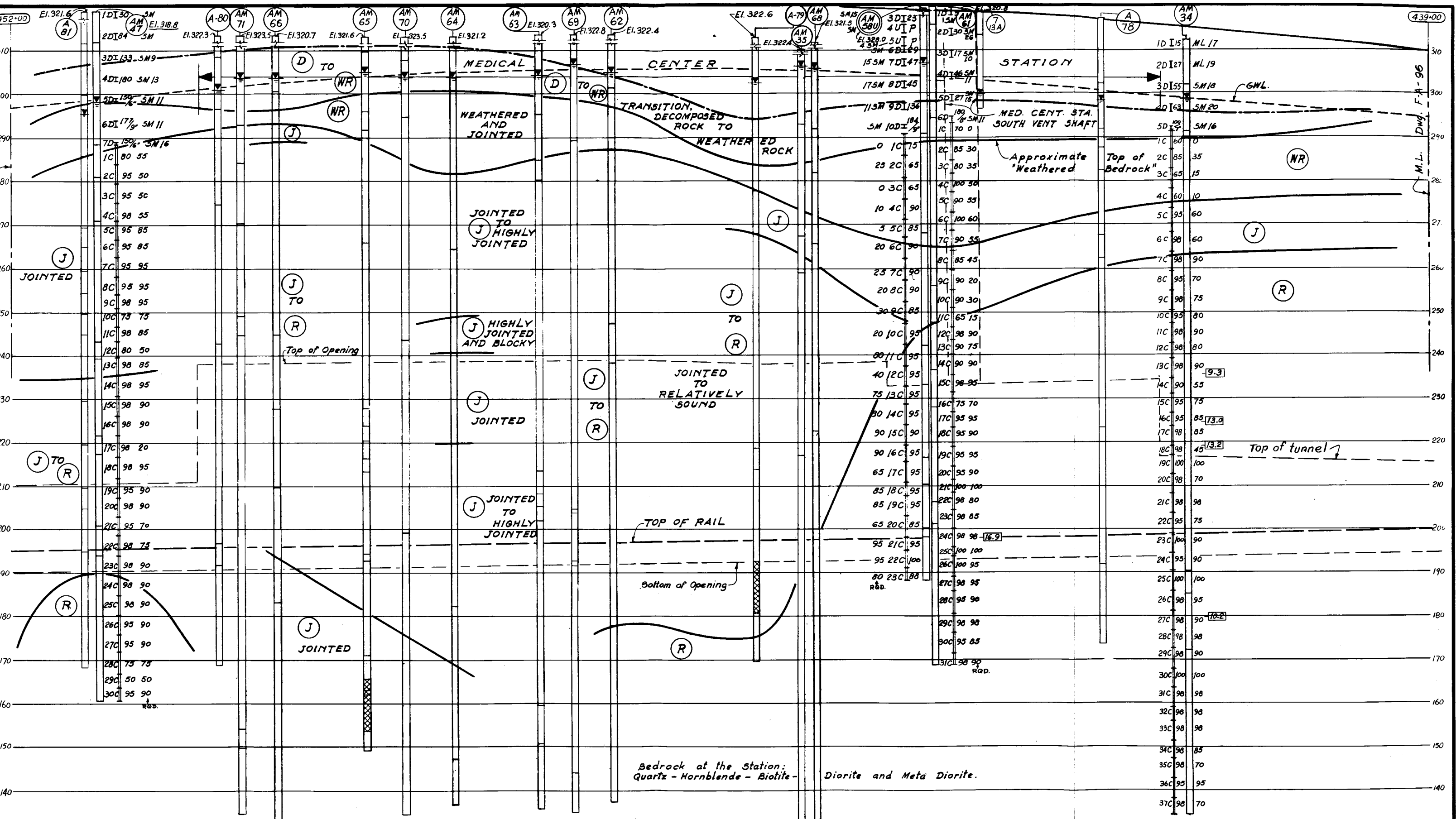
SUBMITTED *Killian & Co.* APPROVED \_\_\_\_\_

DE LEUW, CATHIER & COMPANY  
GENERAL ENGINEERING CONSULTANT  
HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

**ROCKVILLE ROUTE (A011)  
GEOLOGICAL SECTION  
STATION 439+00 to 452+00 (WEST SIDE)**

SCALE: HORIZ. 1" = 40' VERT. 1" = 20'

DRAWING NO. **F-A-95A**



REFERENCE DRAW		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY
F-1	General Notes and Leg. q		
F-A-88	Boring Location Plan		



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER • RUTLEDGE • WFNTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N. Y.

SUBMITTED *William H. Puzos*

DE LEUW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANT

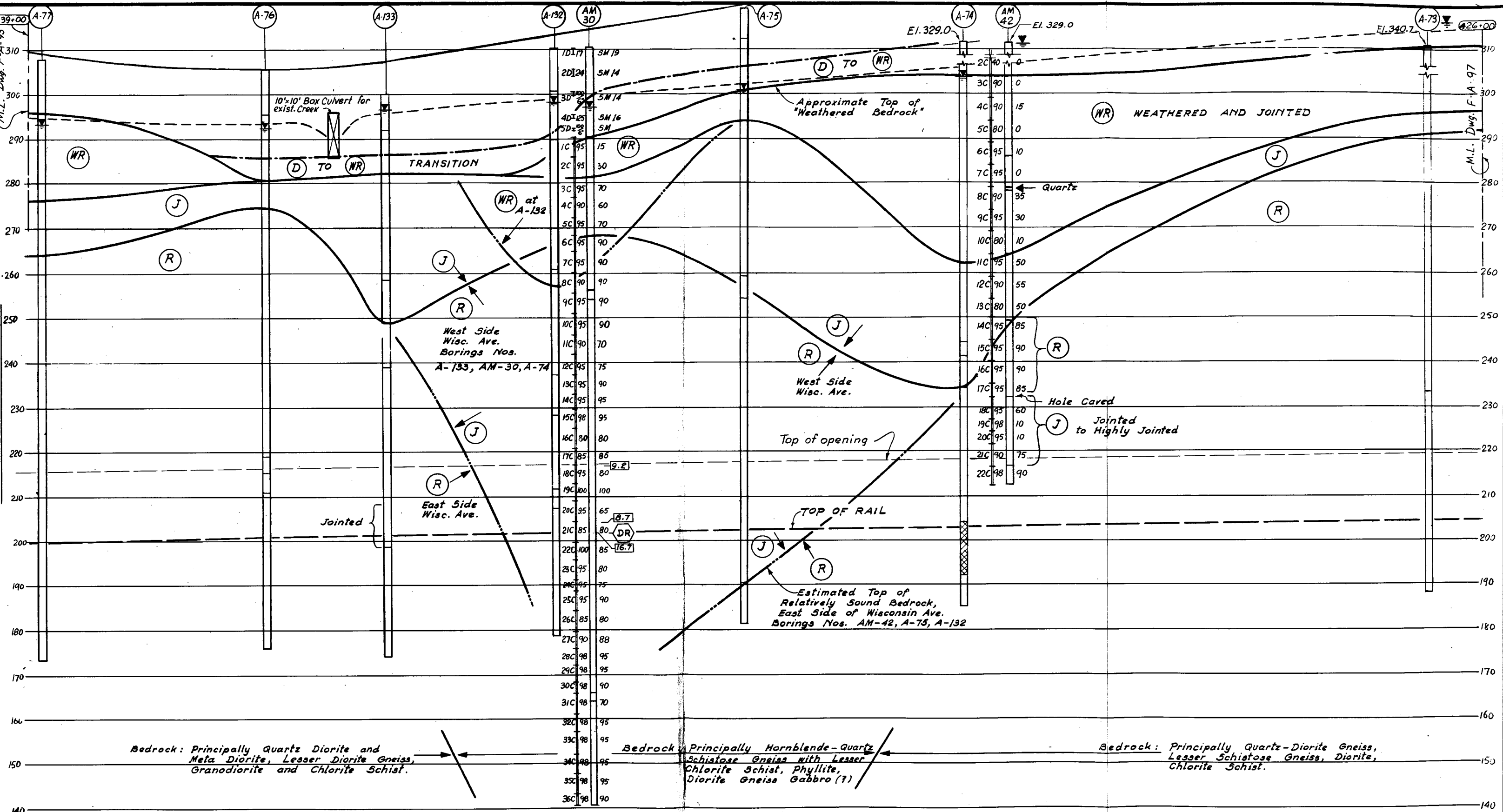
HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

APPROVED \_\_\_\_\_

**ROCKVILLE ROUTE (A011)  
GEOLOGICAL SECTION  
STATION 439+00 TO 452+00 (EAST SIDE)**

DRAWING NO. **F-A-95B**

SCALE  
HORIZ. 1" = 40'  
VERT. 1" = 20'

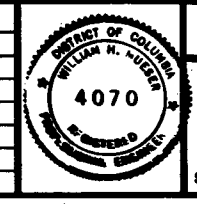


Bedrock: Principally Quartz Diorite and Meta Diorite, Lesser Diorite Gneiss, Granodiorite and Chlorite Schist.

Bedrock: Principally Hornblende-Quartz Schistose Gneiss with Lesser Chlorite Schist, Phyllite, Diorite Gneiss Gabbro (?)

Bedrock: Principally Quartz-Diorite Gneiss, Lesser Schistose Gneiss, Diorite, Chlorite Schist.

DESIGNED	DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION
RAWN	AR. & AG. 12-73	F-1	Gener. Notes and Legend			
CHECKED	SLT. & JPG. 12-74	F-A-88	Boring Location Plan			
APPROVED			For data on A-Series Borings, see Dwg. Nos. F-A-20 & 21, MRNJ Report 58.			



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MEESER • RUTLEDGE • WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N. Y.

SUBMITTED *William H. Wynn*

DE LEUW, CATHAR & COMPANY  
GENERAL ENGINEERING CONSULTANT

HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

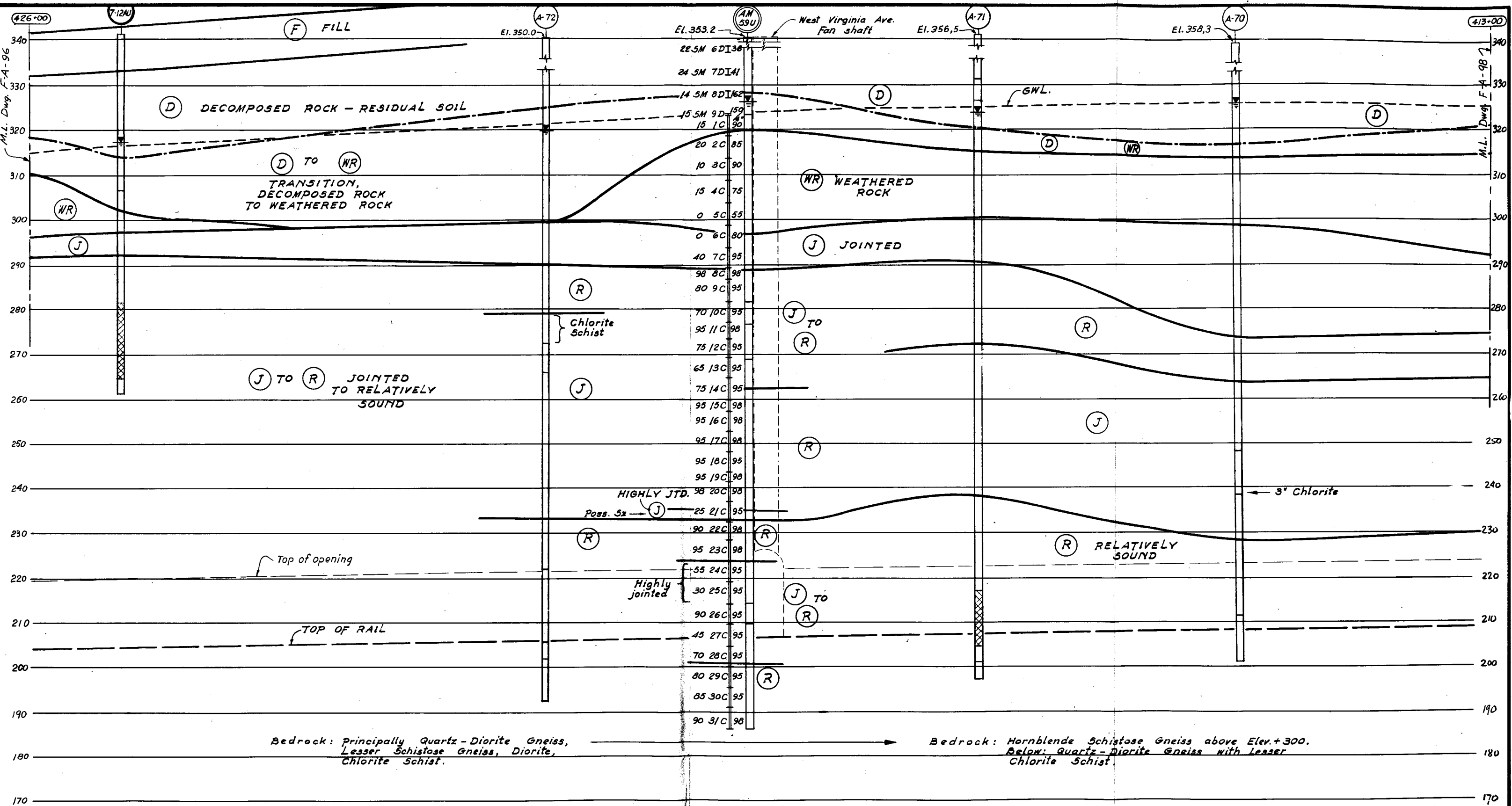
APPROVED \_\_\_\_\_

**ROCKVILLE ROUTE (AO11)  
GEOLOGICAL SECTION  
STATION 426+00 to 439+00**

DRAWING NO. **F-A-96**

SCALE  
HORIZ. 0 20' 40' 80'  
VERT. 0 5' 10' 20'



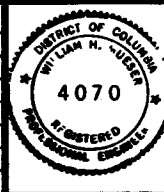


Bedrock: Principally Quartz-Diorite Gneiss, Lesser Schistose Gneiss, Diorite, Chlorite Schist.

Bedrock: Hornblende Schistose Gneiss above Elev.+300. Below: Quartz-Diorite Gneiss with Lesser Chlorite Schist.

DESIGNED BY: AL & AG  
 DATE: 8-74  
 DRAWN BY: SLT & JPG  
 DATE: 12-74  
 CHECKED BY: [Blank]  
 DATE: [Blank]  
 APPROVED BY: [Blank]  
 DATE: [Blank]

REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY
F-1	General Notes and Log. d.		
F-A-89	Boring Location Plan.		



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

NUMBER - RUTLEDGE - WENTWORTH & JOHNSTON  
 CONSULTING ENGINEERS  
 415 MADISON AVE., NEW YORK 17, N. Y.

DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANT

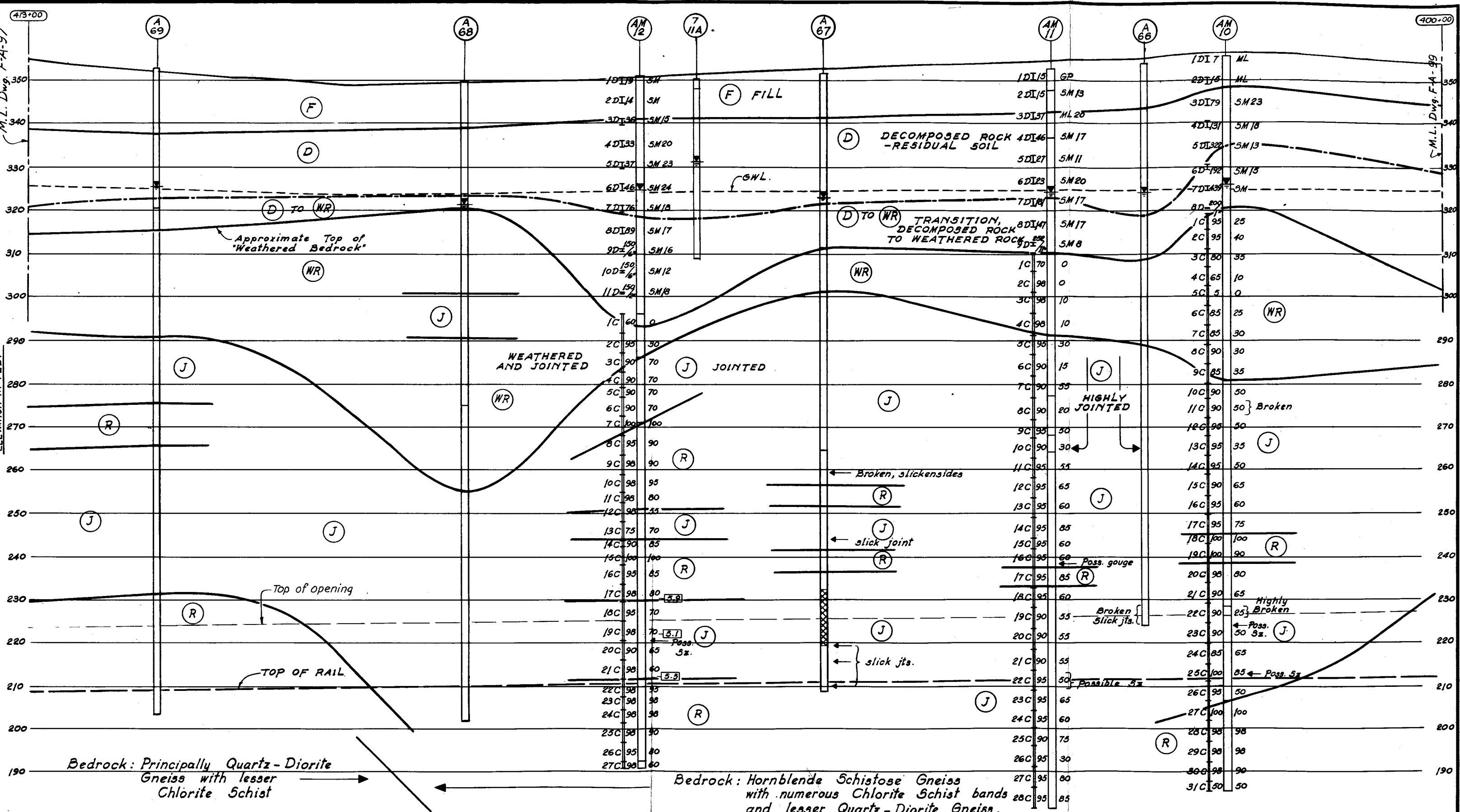
HARRY WEESE & ASSOCIATES  
 GENERAL ARCHITECTURAL CONSULTANT

SUBMITTED BY: *William H. Weaver*

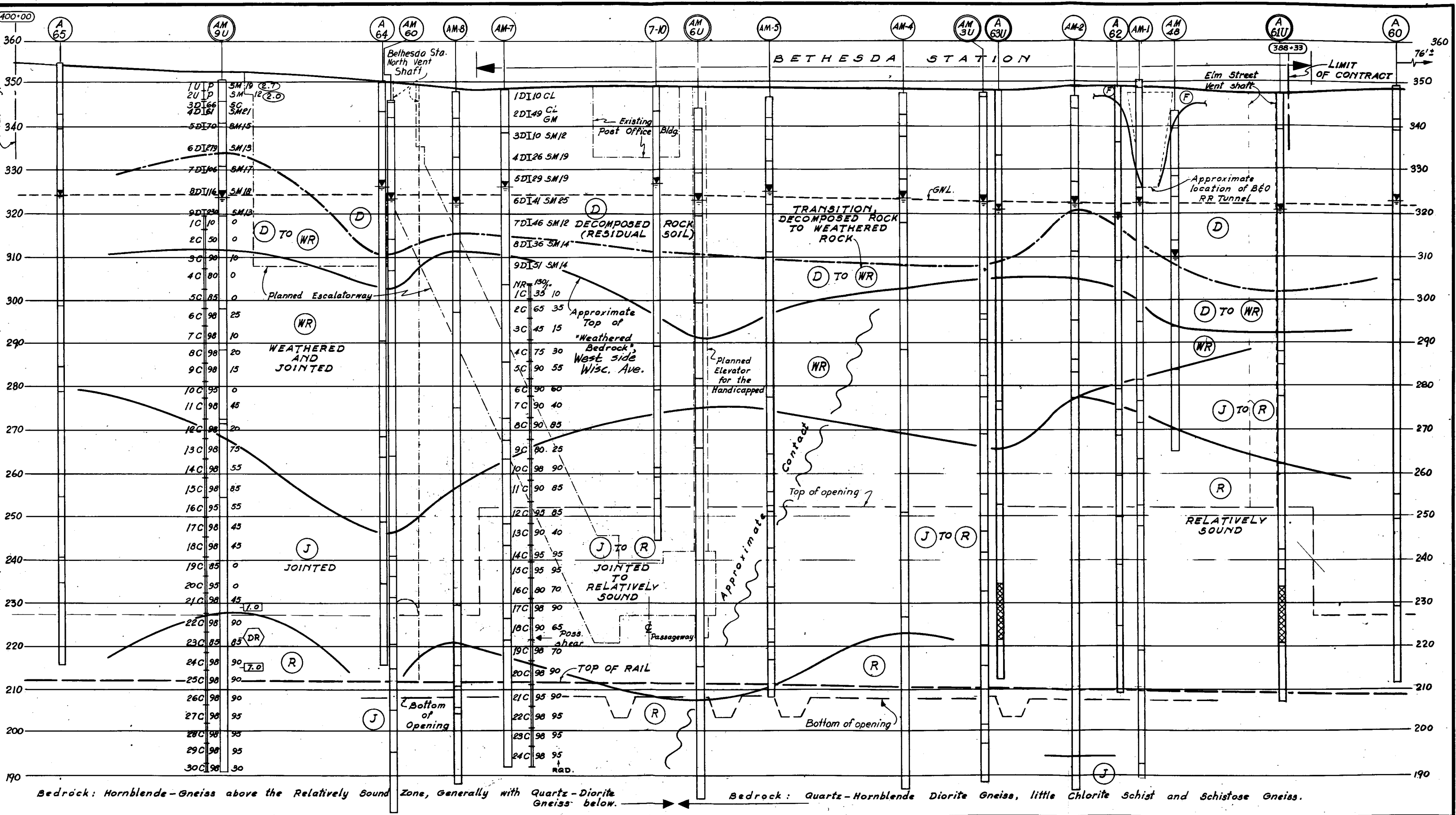
**ROCKVILLE ROUTE (A011)  
 GEOLOGICAL SECTION  
 STATION 413+00 to 426+00**

SCALE: VERT. 0 10' 20'  
 HORIZ. 0 40' 80'

DRAWING NO. **F-A-97**

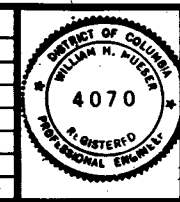


DESIGNED: AR. & AG. DATE: 8-74 DRAWN: SLT. & JPG. DATE: 12-74 CHECKED: DATE: 12-74 APPROVED: DATE:		REFERENCE DRAWINGS NUMBER DESCRIPTION F-1 General Notes and Legend F-A-89 Boring Location Plan		REVISIONS DATE BY DESCRIPTION			<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> WESER - RUTLEDGE - WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y. SUBMITTED: <i>William H. Funder</i>		DE LEIJW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT		ROCKVILLE ROUTE (A011) GEOLOGICAL SECTION STATION 400+00 to 413+00 SCALE: HORIZ. 1" = 20', VERT. 1" = 10' DRAWING NO. F-A-98	
---	--	---	--	----------------------------------	--	--	---	--	--	--	--	--



DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION
12-73	F-1	General Notes and Legend.			
12-74	F-A-89	Boring Location Plan.			
For detailed information on overburden conditions see Dwg. Nos. F-A-102 & -103					

DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER • RUTLEDGE • WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N. Y.

SUBMITTED *Kellian H. Weese*

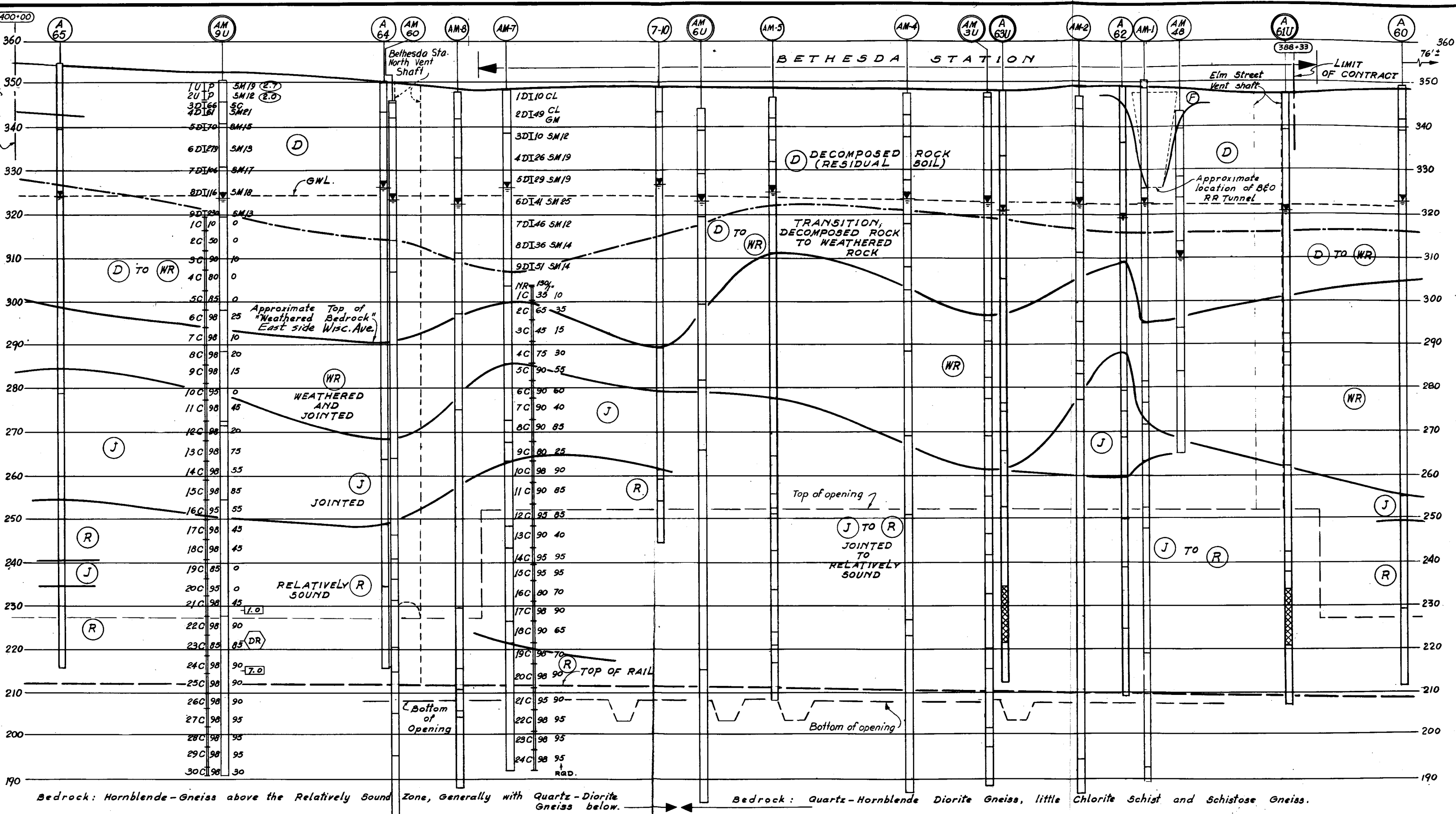
DE LEUW, CATHAR & COMPANY  
GENERAL ENGINEERING CONSULTANT

HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

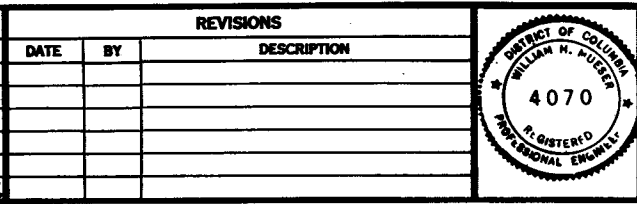
**ROCKVILLE ROUTE (A011)**  
**GEOLOGICAL SECTION**  
STATION 388+00 to 400+00 (WEST SIDE)

SCALE: HORIZ. 1" = 20', VERT. 1" = 10'

DRAWING NO. **F-A-99A**



REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY
F-1	General Notes and Legend.		
F-A-89	Boring Location Plan.		
For detailed information on overburden conditions see DWG. Nos. F-A-102 & -103.			



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER • RUTLEDGE • WENTWORTH & JOHNSTON**  
 CONSULTING ENGINEERS  
 415 MADISON AVE., NEW YORK 17, N. Y.

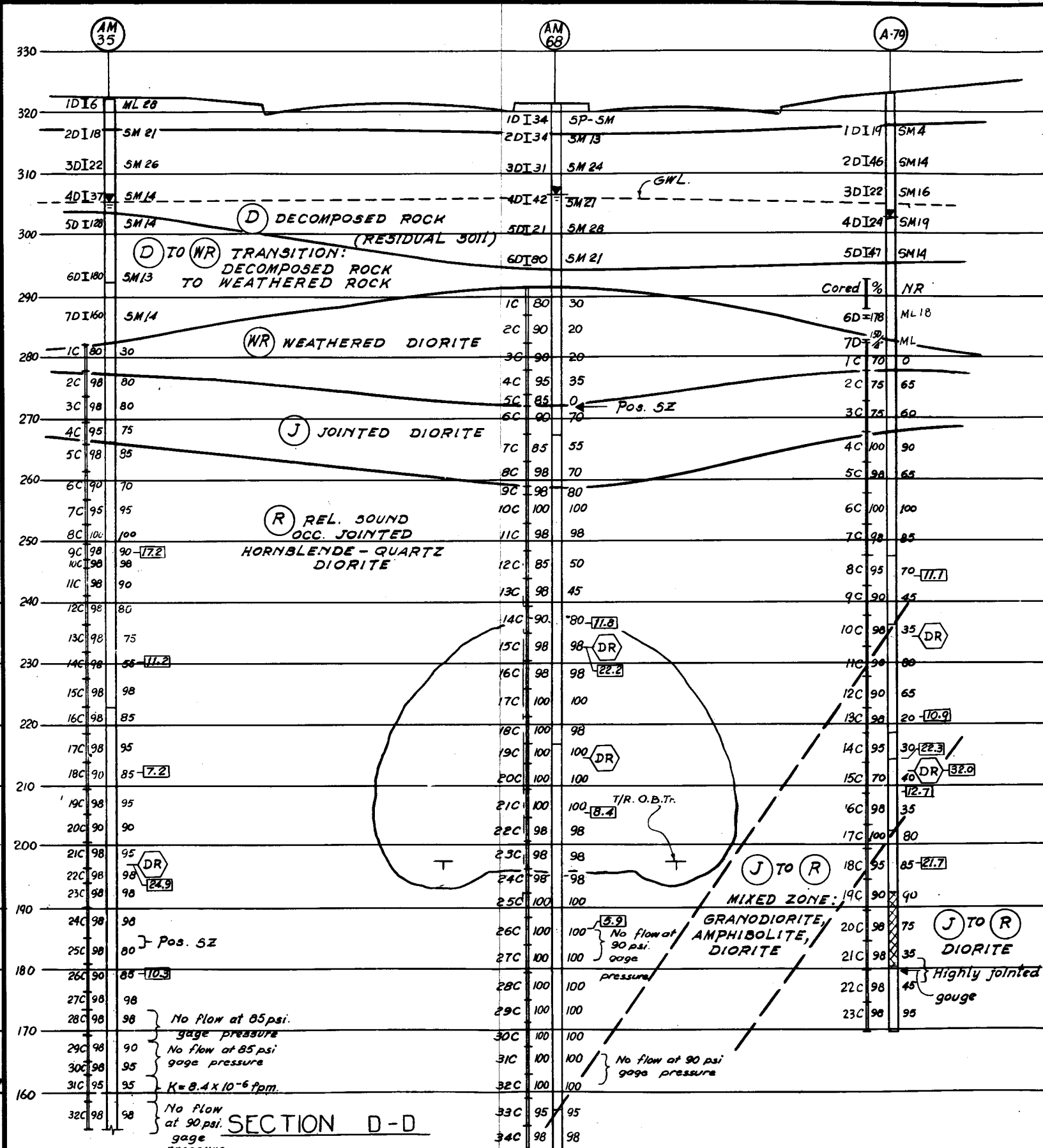
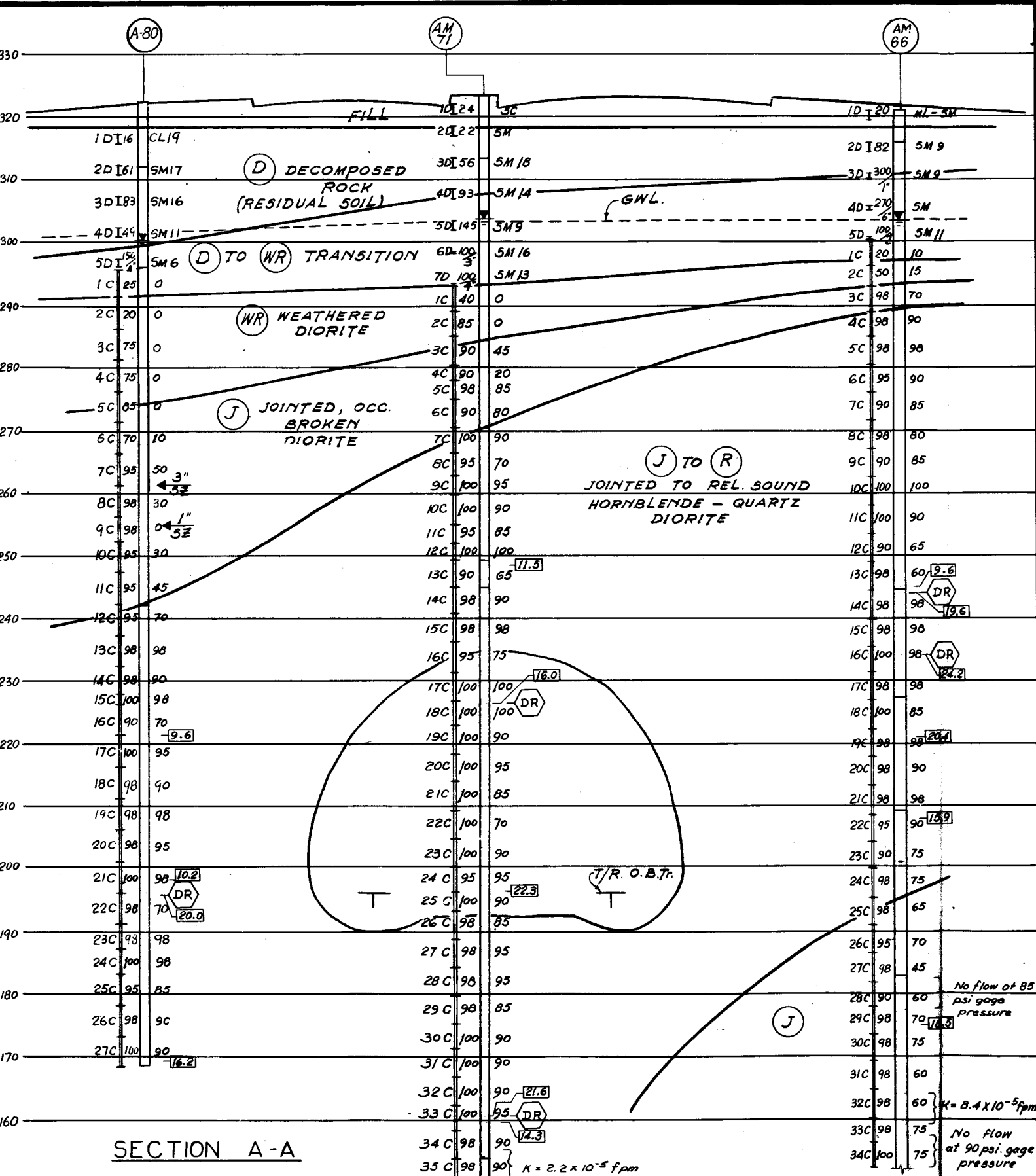
SUBMITTED *Killian H. Weese*

**ROCKVILLE ROUTE (A011)**  
**GEOLOGICAL SECTION**  
 STATION 388+00 to 400+00 (EAST SIDE)

DE LEUW, CATHER & COMPANY  
 GENERAL ENGINEERING CONSULTANT

HARRY WEESE & ASSOCIATES  
 GENERAL ARCHITECTURAL CONSULTANT

DRAWING NO. **F-A-99B**



REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY
AR. & AG.	1-74	F-1	General Notes and Legend.
SLT. & JPG.	12-74	F-A-88	Boring Location Plan.



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MEESER - RUTLEDGE - WENTWORTH & JOHNSTON**  
 CONSULTING ENGINEERS  
 415 MADISON AVE., NEW YORK 17, N. Y.

DE LEUW, CATHIER & COMPANY  
 GENERAL ENGINEERING CONSULTANT

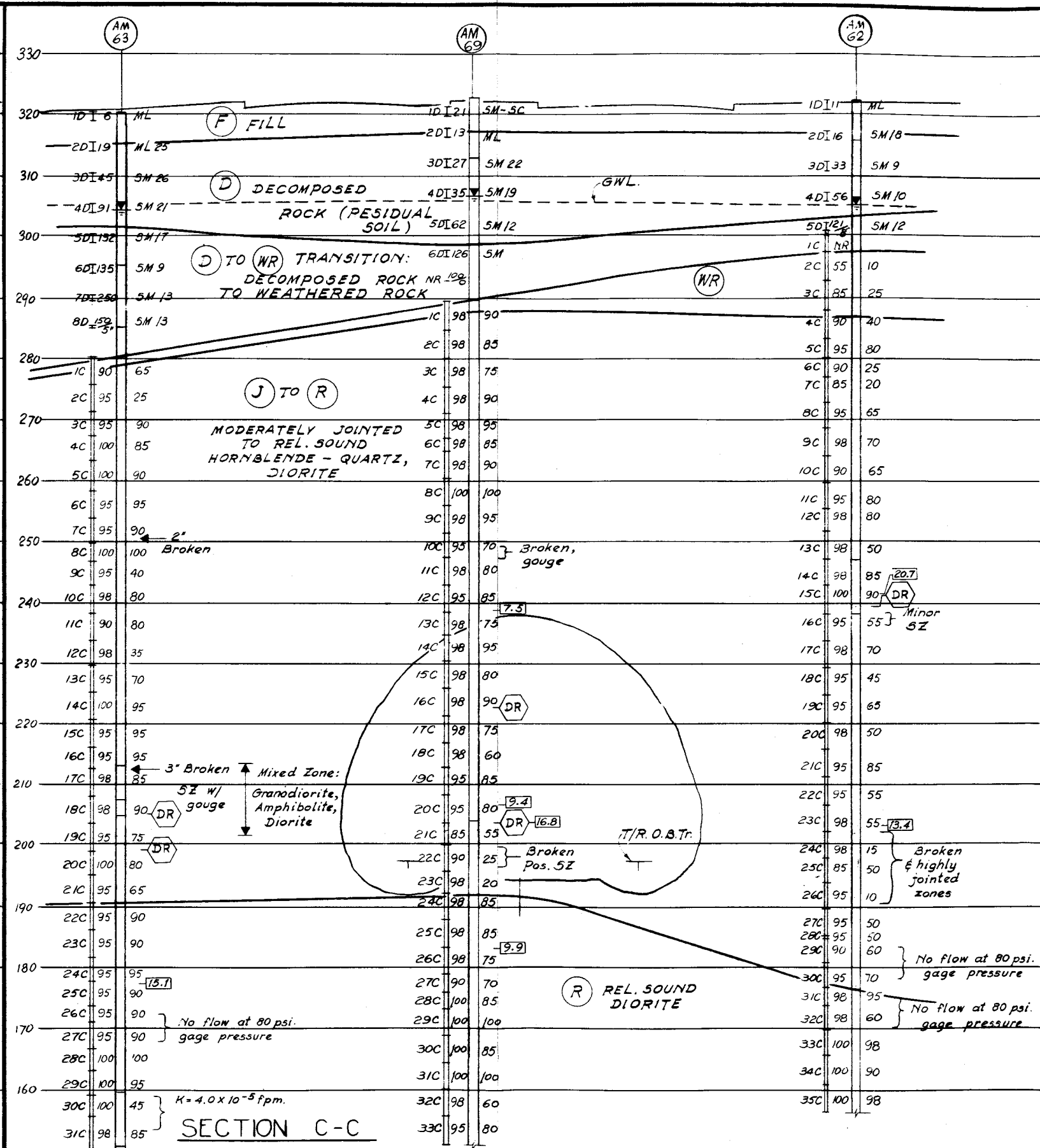
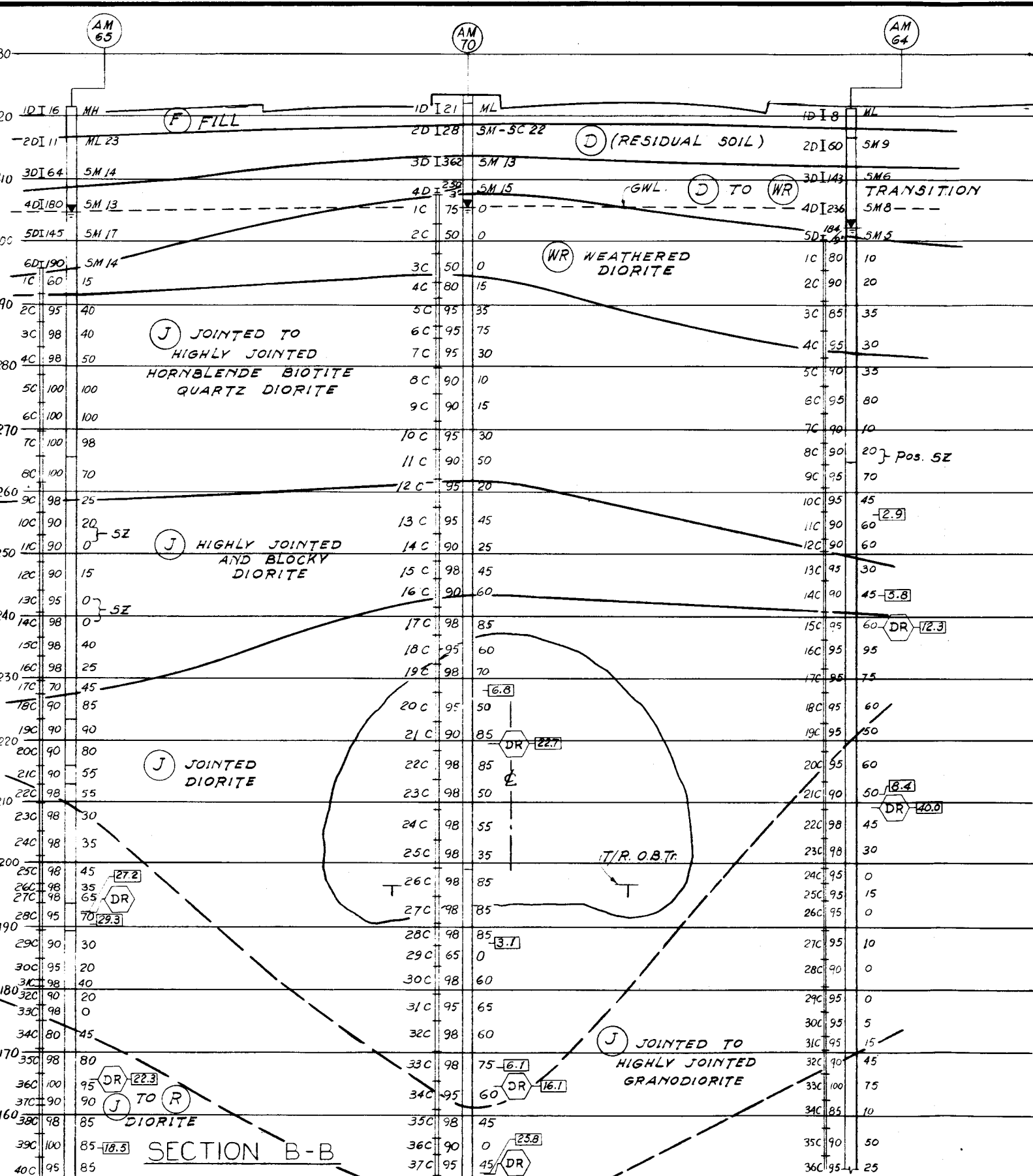
HARRY WEESE & ASSOCIATES  
 GENERAL ARCHITECTURAL CONSULTANT

APPROVED \_\_\_\_\_

**ROCKVILLE ROUTE (A011)**  
**MEDICAL CENTER STATION**  
 GEOLOGICAL SECTIONS A-A and D-D

SCALE: 0 10' 20'  
 HORIZ. & VERT.

DRAWING NO. **F-A-100**



DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION
AR & AG 1-74	F-1	General Notes and Legend.			
SLT & JPG 12-74	F-A-88	Boring Location Plan.			

WILLIAM H. MUESER  
4070  
REGISTERED PROFESSIONAL ENGINEER

**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

MUESER - RUTLEDGE - WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N. Y.

SUBMITTED *William H. Mueser*

DE LEUW, CATHIER & COMPANY  
GENERAL ENGINEERING CONSULTANT

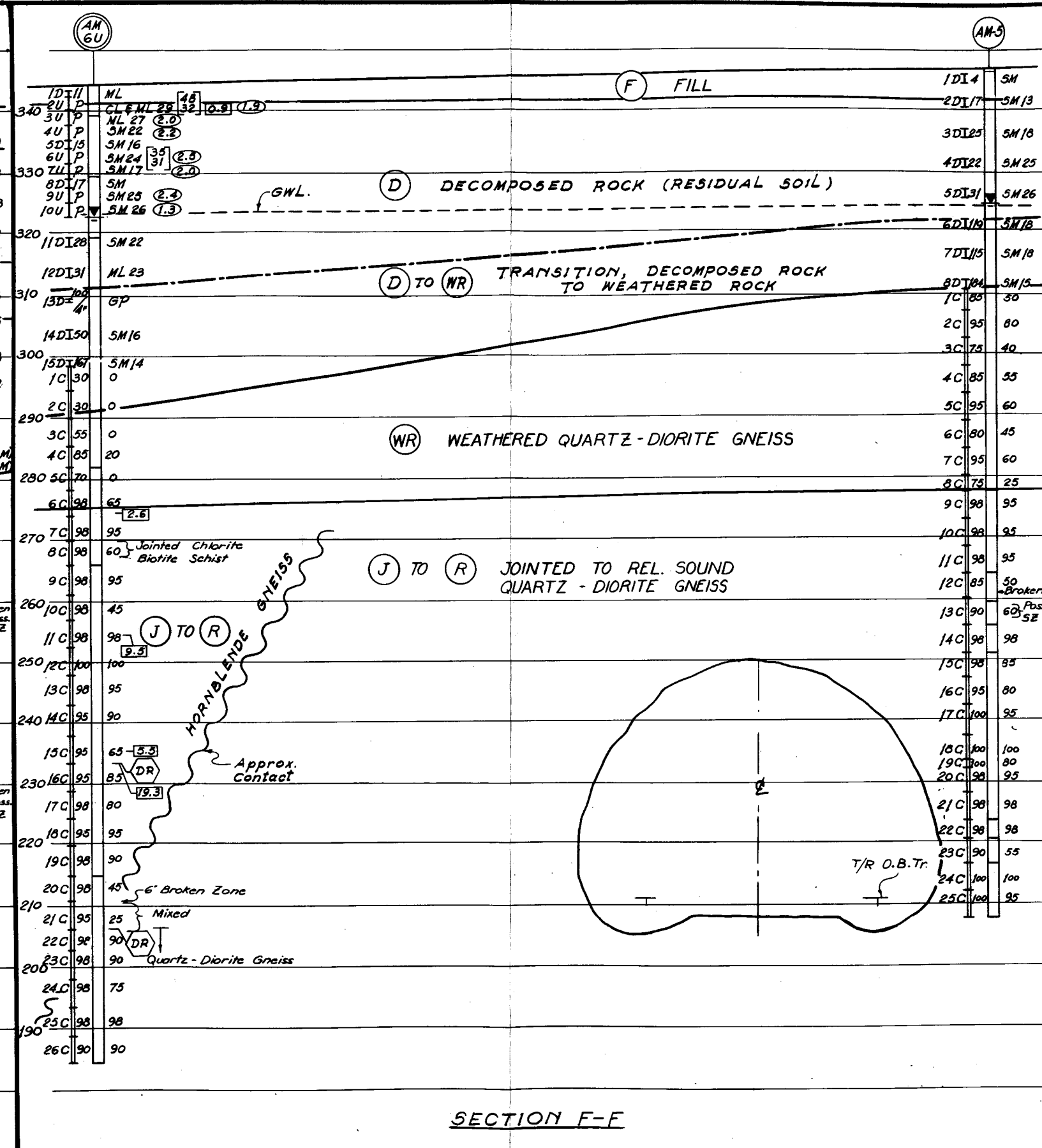
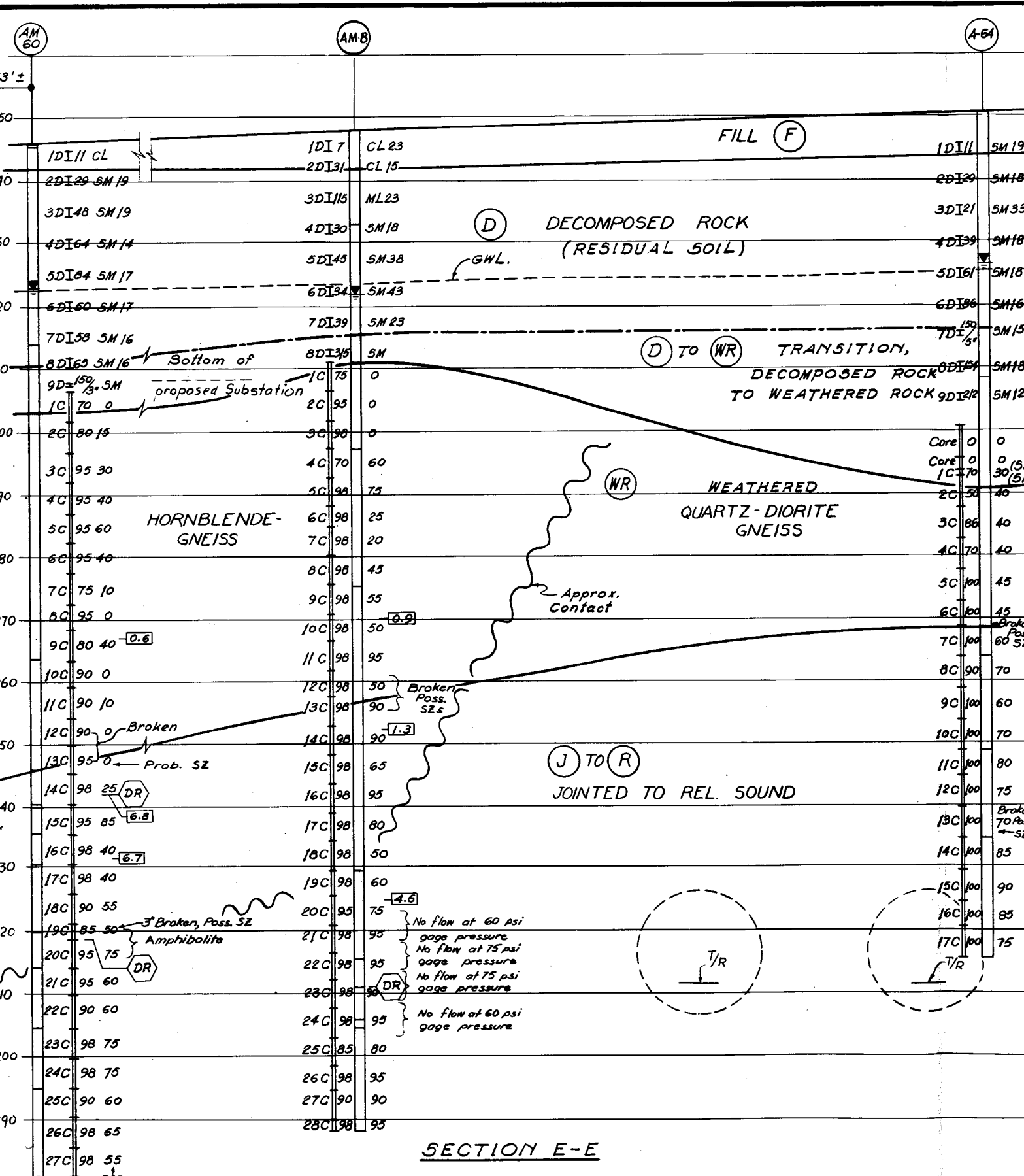
HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

APPROVED

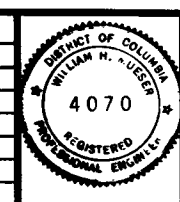
**ROCKVILLE ROUTE (A011)  
MEDICAL CENTER STATION  
GEOLOGICAL SECTIONS B-B and C-C**

SCALE: 0 10' 20'  
HORIZ. & VERT.

DRAWING NO. **F-A-101**



REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY
F-1	General Notes and Legend.		
F-A-89	Boring Location Plan.		



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**WEESER • RUTLEDGE • WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N. Y.

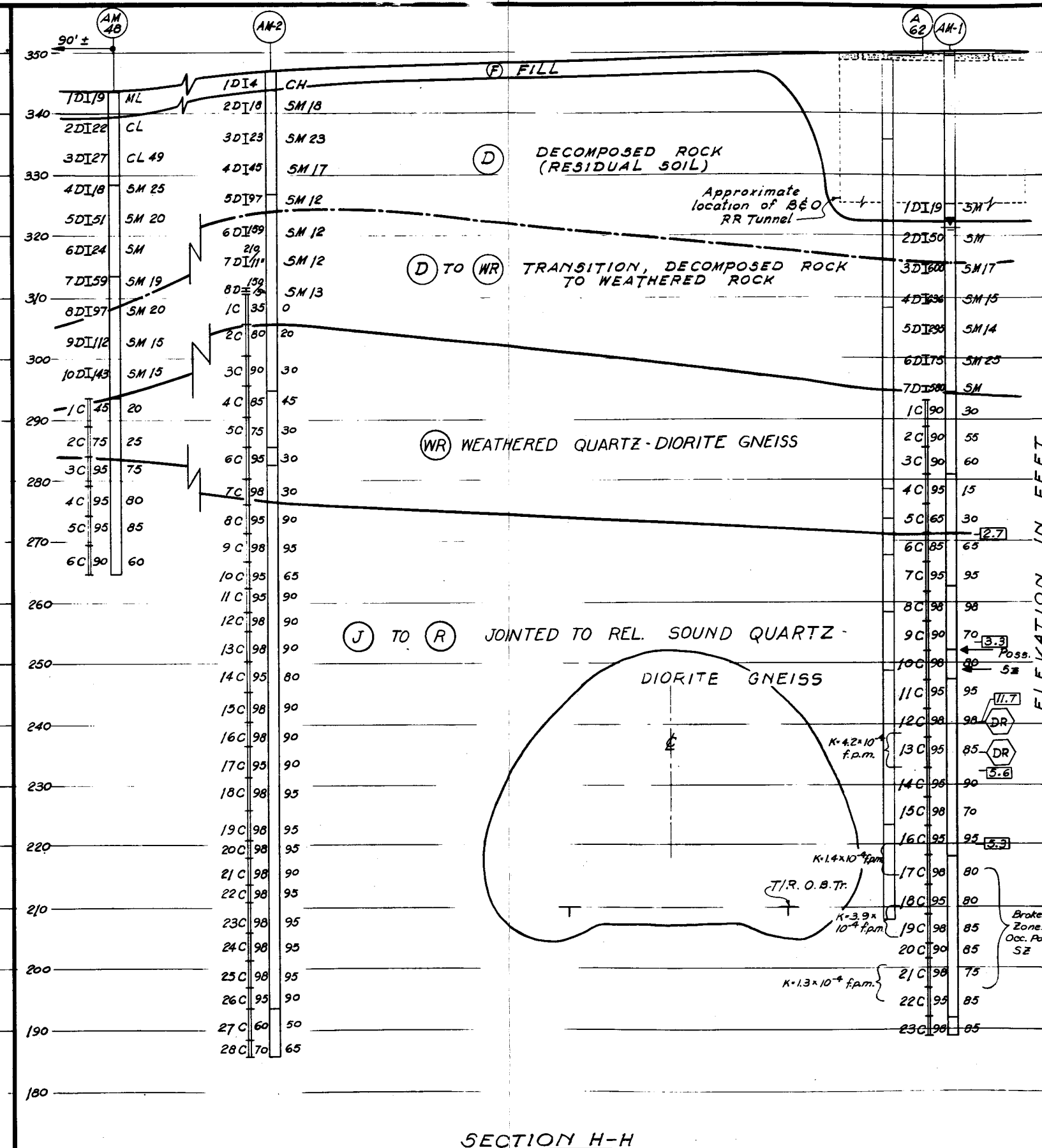
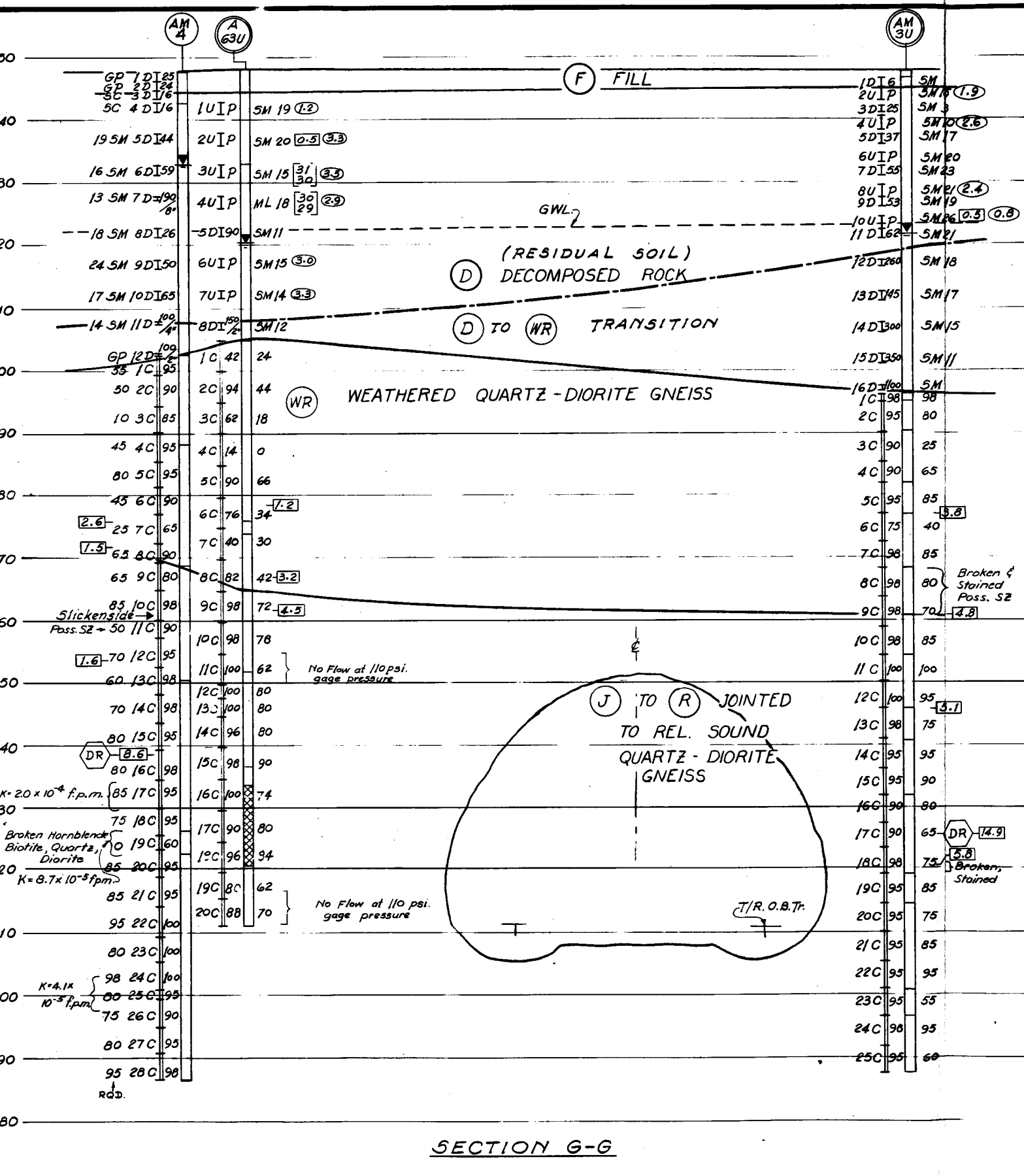
DE LEIJW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANT

HARRY WEESSE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

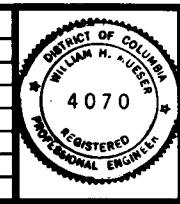
**ROCKVILLE ROUTE (A011)  
BETHESDA STATION  
GEOLOGICAL SECTIONS E-E and F-F**

SCALE: HORIZ. 0 5' 10' 20'  
VERT. 1" = 10'

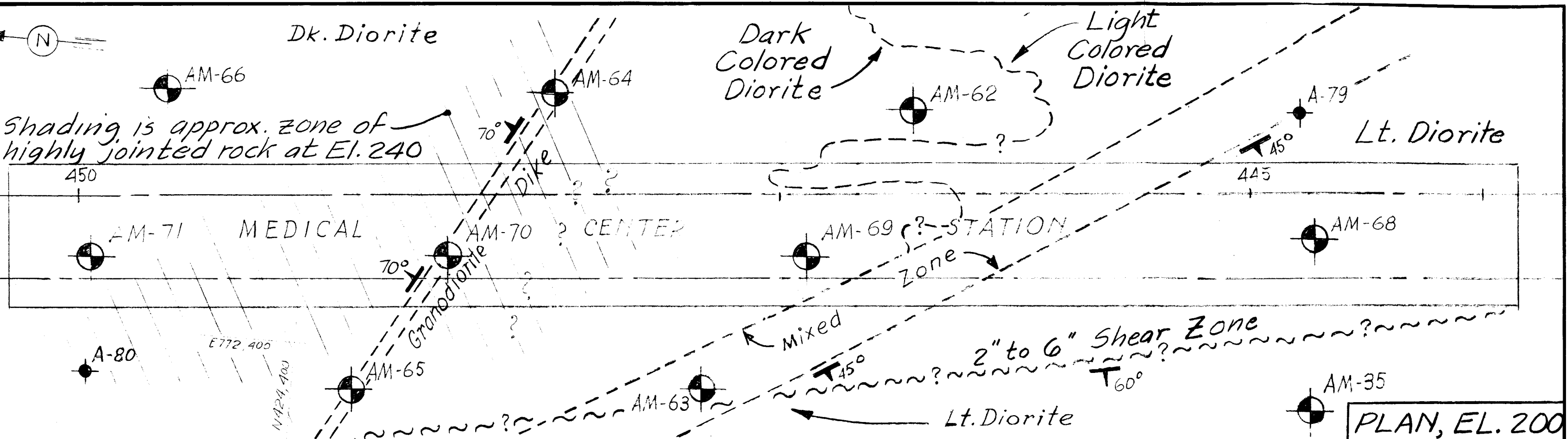
DRAWING NO. **F-A-102**



DESIGNED		DATE		REFERENCE DRAWINGS		REVISIONS		WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY		ROCKVILLE ROUTE (AOII)	
BY	DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION	DESCRIPTION	CONSULTING ENGINEERS		BETHESDA STATION	
AG. & I.H.L.	8-74	F-1	General Notes and Legend.					DE LEUW, CATHER & COMPANY		GEOLOGICAL SECTIONS G-G and H-H	
SLT. & J.P.G.	12-74	F-A-89	Boring Location Plan.					HARRY WEESE & ASSOCIATES		SCALE	
								GENERAL ARCHITECTURAL CONSULTANT		HORIZ. & VERT. 0 5' 10' 20'	
								SUBMITTED <i>William H. Weese</i>		DRAWING NO. F-A-103	

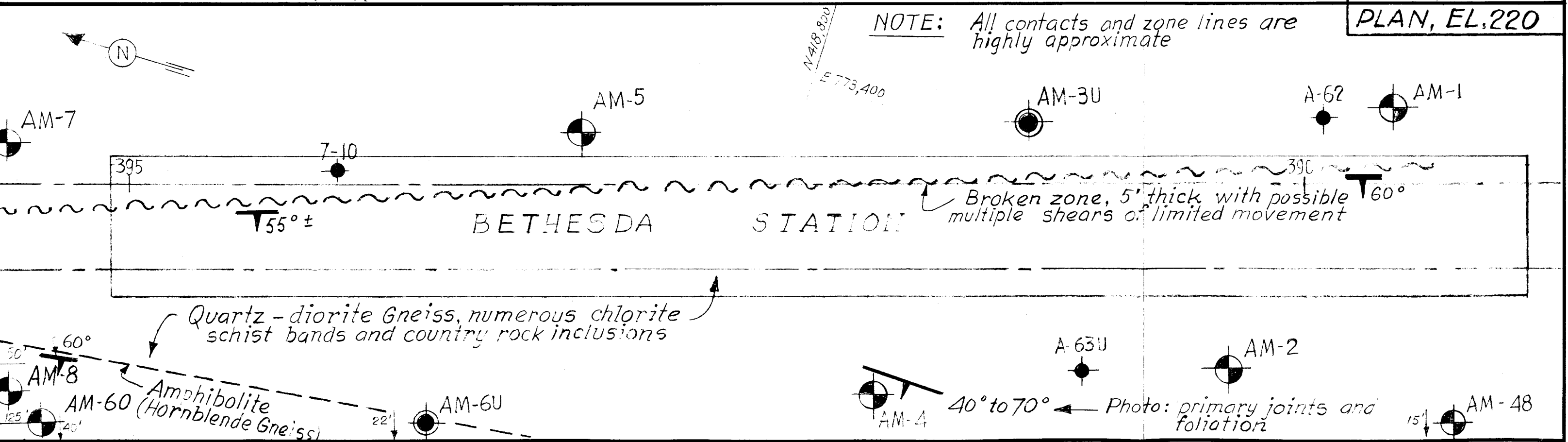




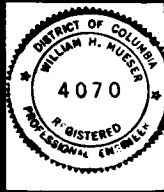


PLAN, EL. 200  
 PLAN, EL. 220

NOTE: All contacts and zone lines are highly approximate



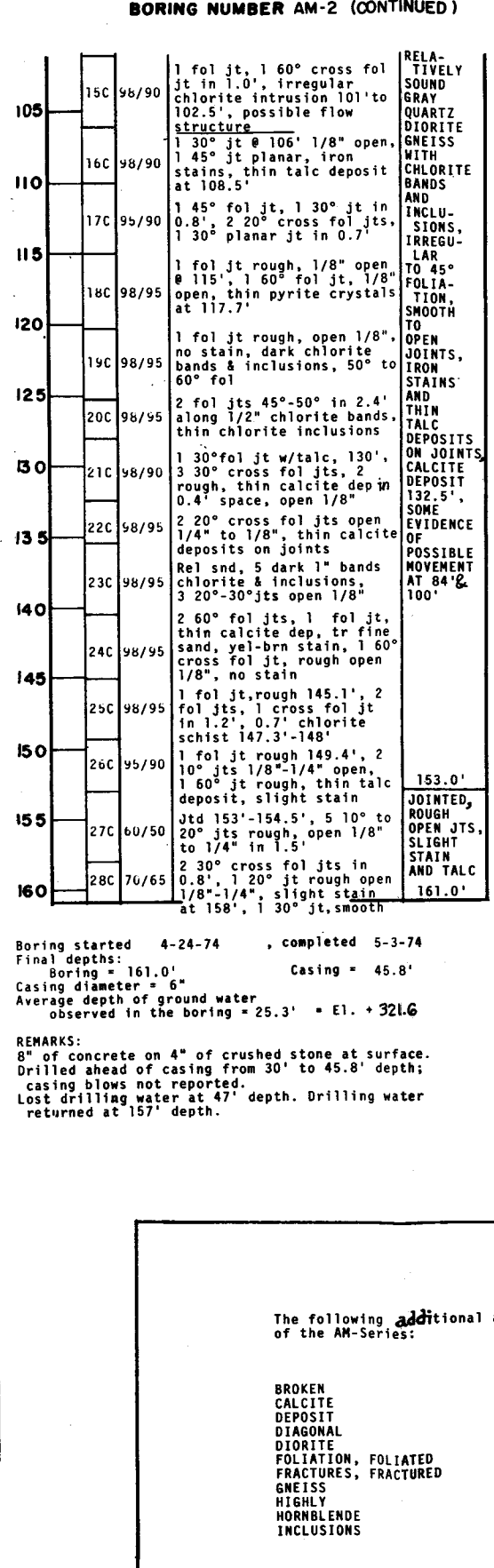
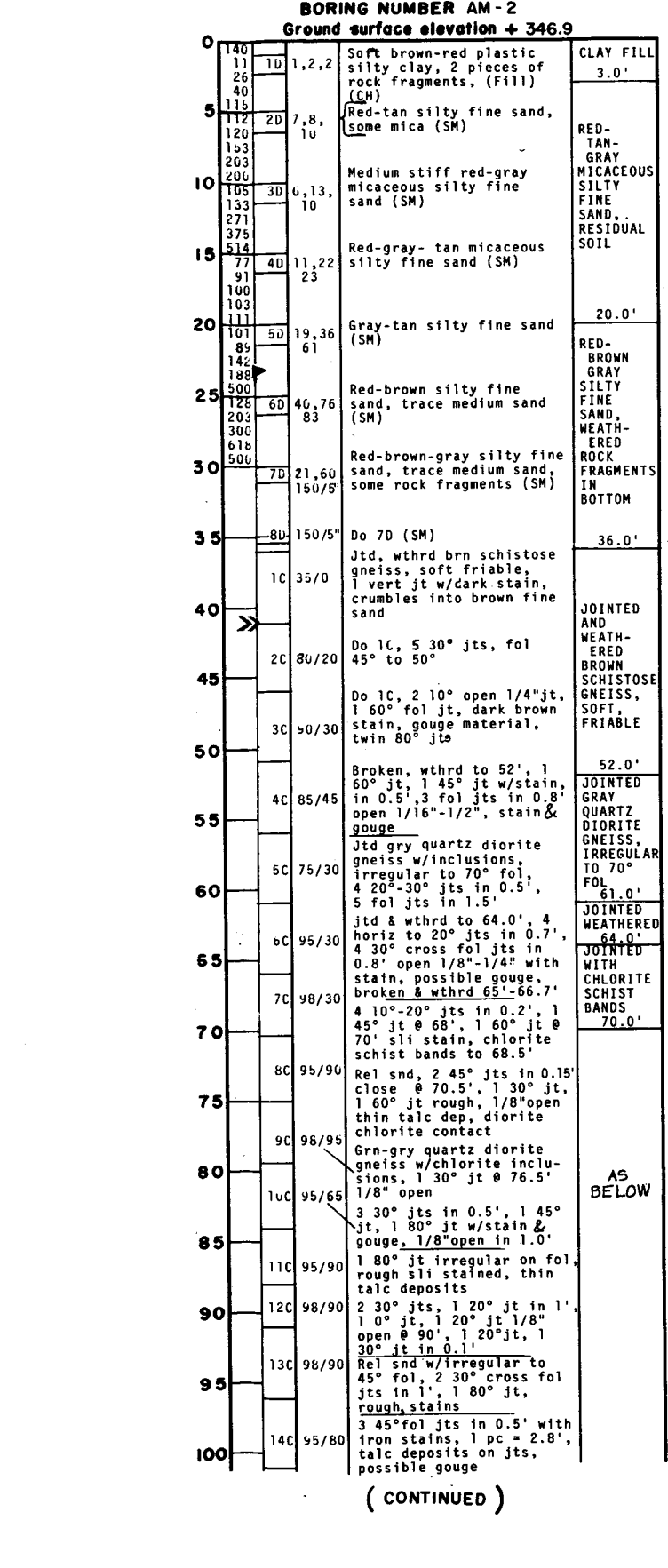
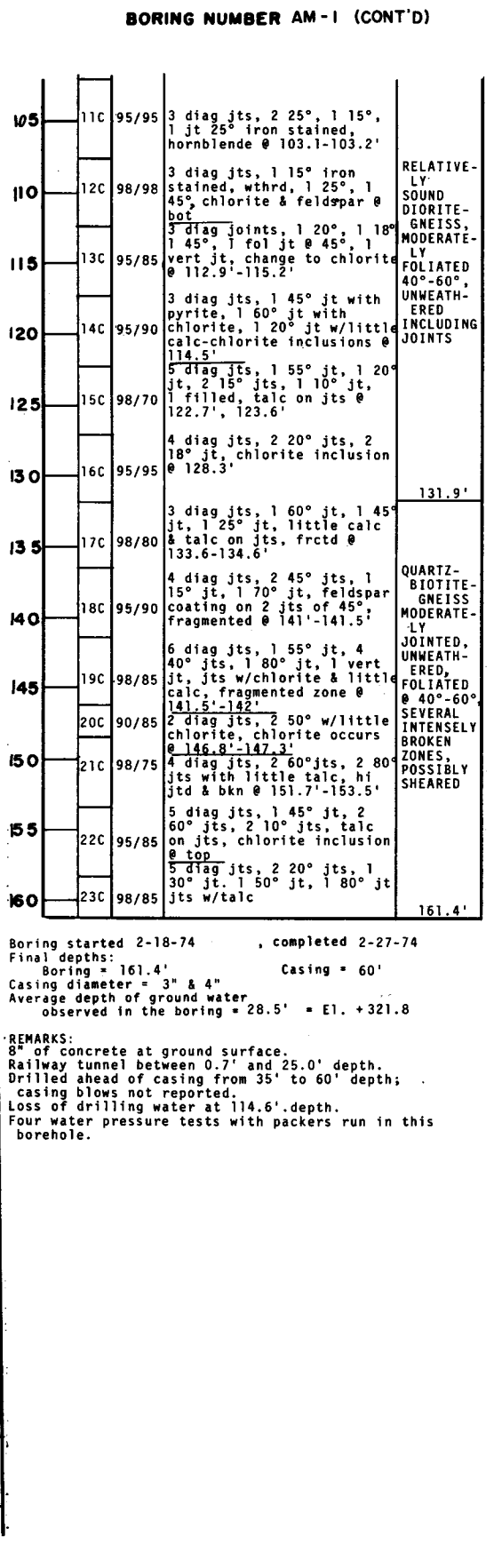
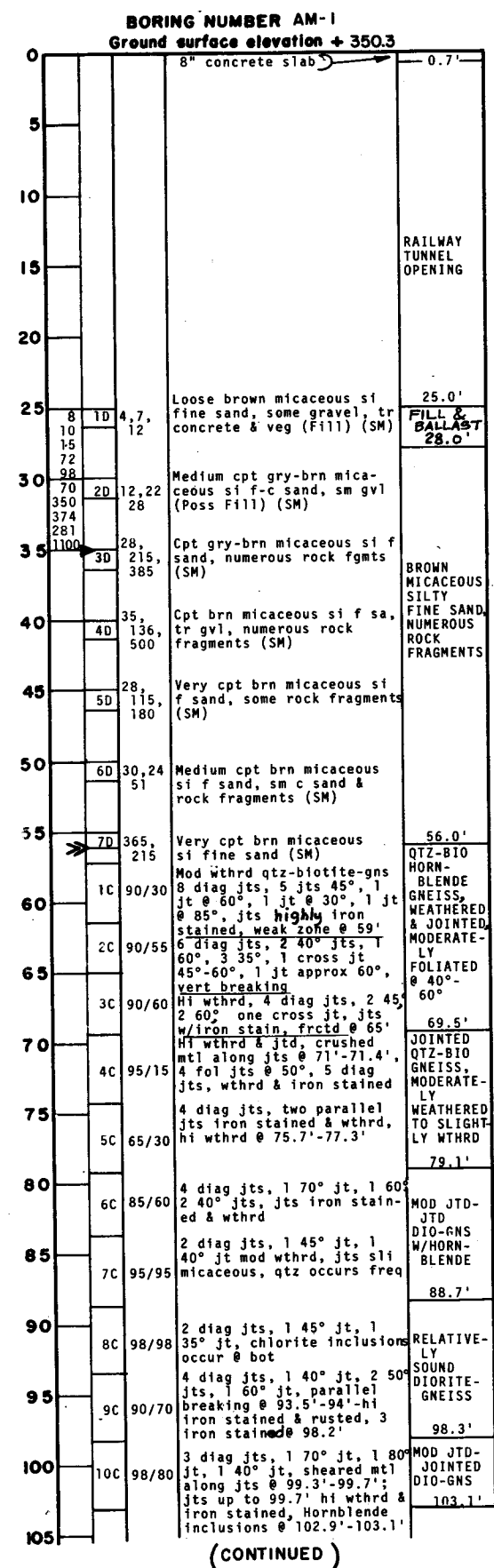
REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY
F-1	General Notes and Legend		



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**  
 MUESER • RUTLEDGE • WENTWORTH & JOHNSTON  
 CONSULTING ENGINEERS  
 415 MADISON AVE., NEW YORK 17, N. Y.  
 SUBMITTED *William H. Mueser*

DE LEUW, CATHIER & COMPANY  
 GENERAL ENGINEERING CONSULTANT  
 HARRY WEESE & ASSOCIATES  
 GENERAL ARCHITECTURAL CONSULTANT

ROCKVILLE ROUTE (A011)  
 MEDICAL CENTER AND BETHESDA STATIONS  
 GEOLOGY  
 SCALE 0 20' 40'  
 DRAWING NO. F-A-104



**NOTES FOR AM-SERIES BORING LOGS**

- A total of 59 borings of the AM-series between Nos. AM-1 and AM-71 were made by Empire Soils Investigations, Inc. from September 1973 to August 1974 under the continuous inspection of Mueser, Rutledge, Wentworth & Johnston engineers. A group of eight supplementary borings, Nos. AM-72 through AM-79, were made by Empire Soils Investigations, Inc. in December 1974 and January 1975 under the continuous supervision of Mueser, Rutledge, Wentworth & Johnston engineers.
- General notes concerning symbols and the method of presentation of data on boring log drawings and geological section drawings are given on Information Drawing No. F-1. For survey data on coordinate locations and ground surface elevations at the borings see Table No.1 of Mueser, Rutledge, Wentworth & Johnston Report No.117 of December 30, 1974.
- All rock cores from Borings Nos. AM-1 through AM-71 were recovered in NX-size, triple tube, split core barrels (Christensen No. NWD-3) recovering cores 2-1/8-inch in diameter; except that Borings Nos. AM-7, -23 and -42 were cored with a 4-1/2-inch by 5-inch diamond bit yielding a core approximately 4-inches in diameter.
- The "Rock Quality Designation" (RQD) is a parameter which reflects the intensity of jointing in the recovered core. For any particular core the RQD equals the sum of the lengths of pieces of core run, expressed as a percentage. General descriptions of rock quality are based primarily on RQD values. For example, "relatively sound rock" is that core wherein the mineral matrix is essentially unweathered and RQD exceeds 75 per cent. This description does not mean that the rock will remain intact and free of joints, fractures or breaks in the process of excavation. For a detailed description of adjectives used to describe rock quality see Mueser, Rutledge, Wentworth & Johnston Report No. 117.
- The natural materials overlying bedrock are derived from weathering and decomposition of the parent rock in-situ. In this investigation natural overburden materials above bedrock are divided into two categories:  
Stratum (D): decomposed rock (residual soil)  
Stratum (D) to (WR): transition, decomposed rock to weathered rock  
The upper "residual soil" stratum is composed almost entirely of soil-like material. The lower "transition" stratum is expected to contain both soil and rock-like materials in roughly equal proportions. The division between these two strata is generally taken in the borings at a standard sampler penetration resistance value of approximately 100 blows per foot and in accordance with certain other criteria given in Mueser, Rutledge, Wentworth & Johnston Report No.117. The "approximate top of weathered bedrock" constitutes the boundary between the transition stratum and bedrock which exhibits essentially rock-like characteristics. This boundary generally is taken where rock core recoveries exceed approximately 50 per cent and/or RQD values exceed approximately 10 per cent.

**ABBREVIATIONS**

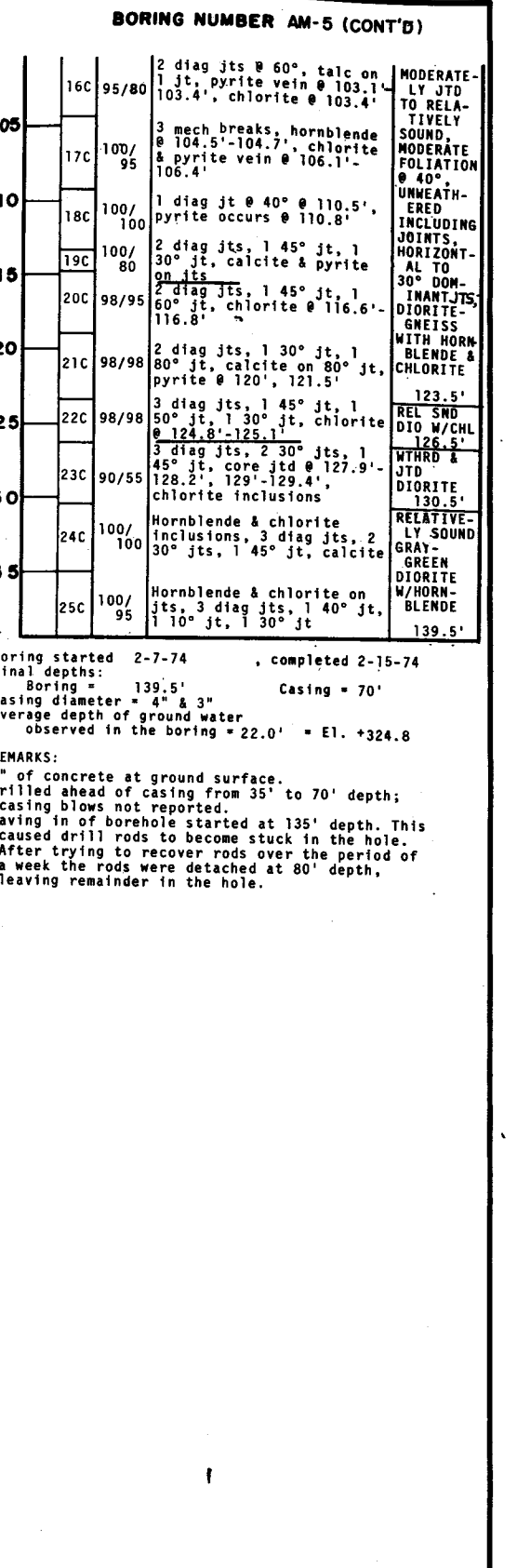
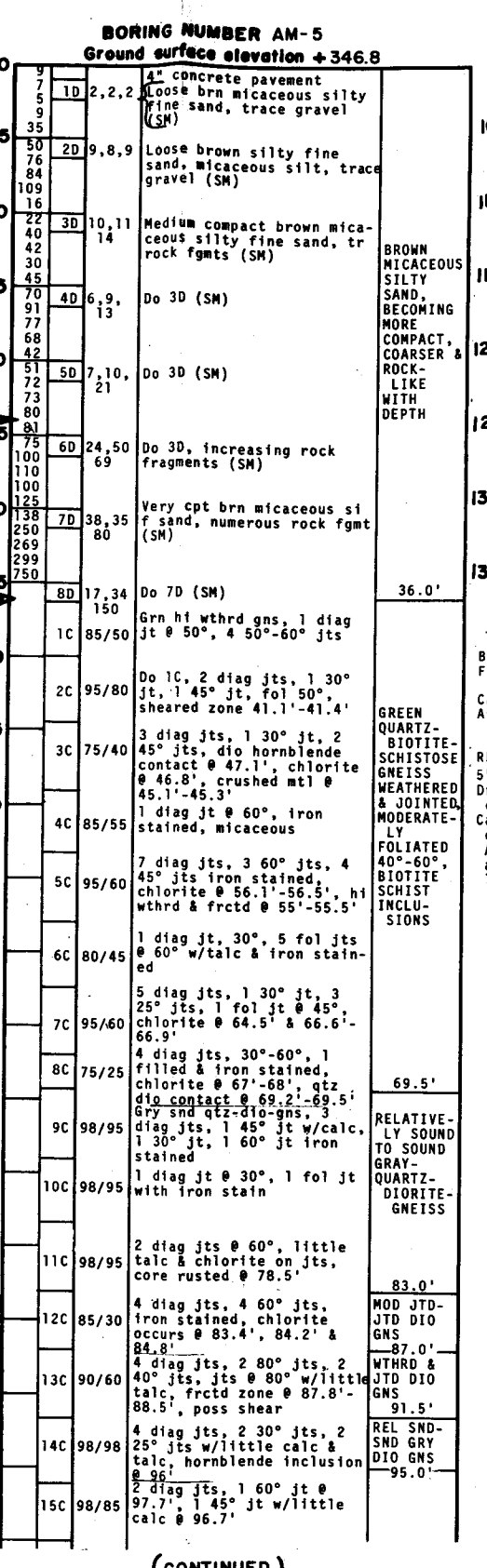
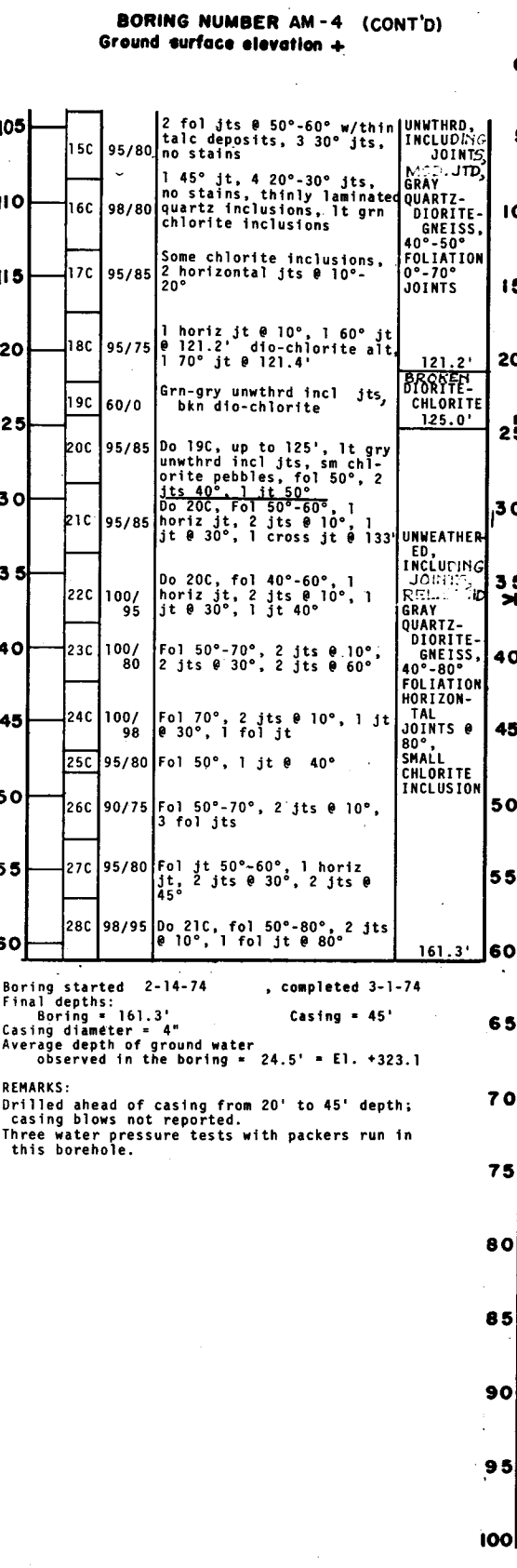
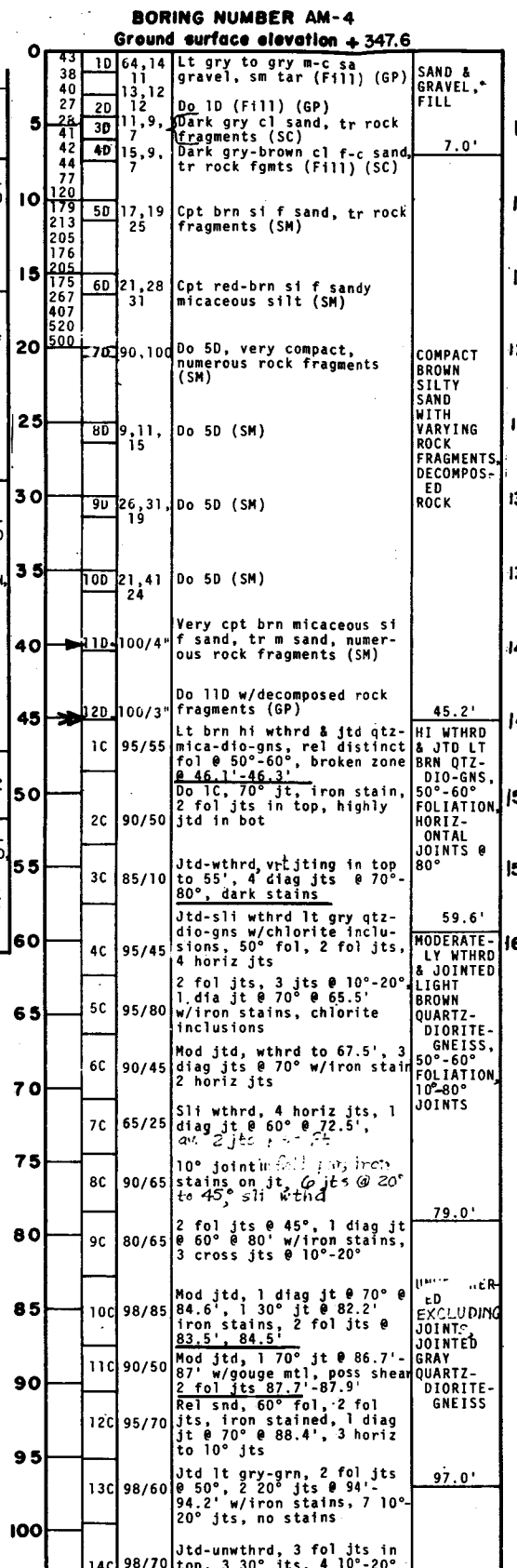
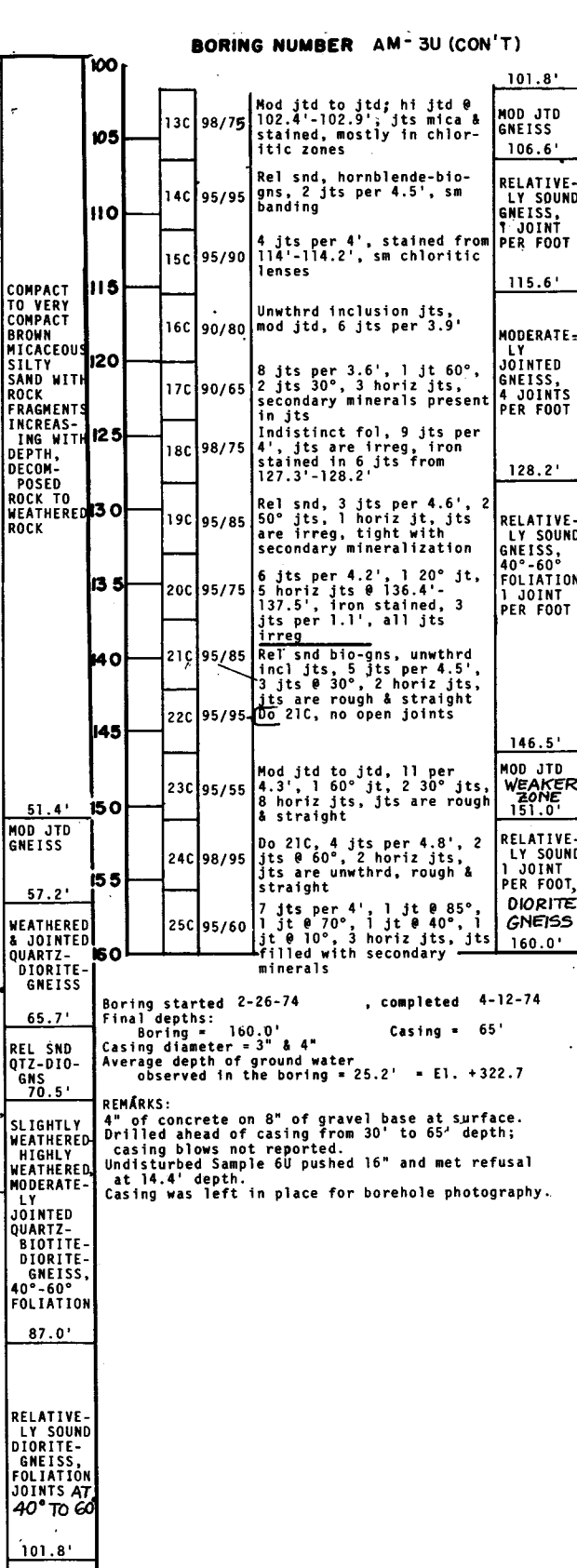
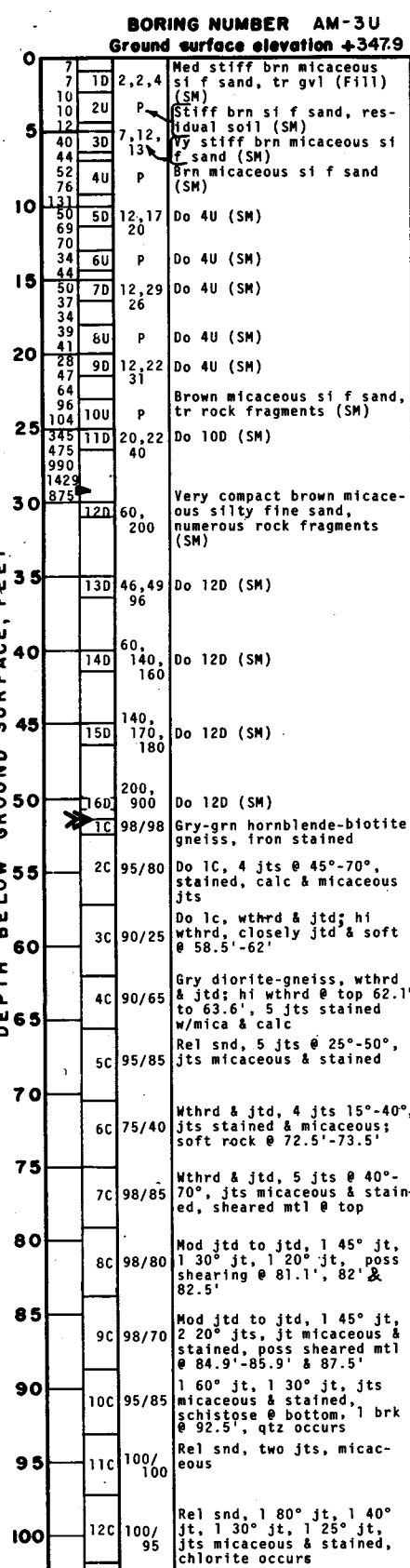
The following additional abbreviations have been utilized in the logs of Borings of the AM-Series:

BROKEN CALCITE DEPOSIT	brkn, bkn	JOINT, JOINTED, JOINTS, JOINTING	jt, jtd, jts
DIAGONAL DIORITE FOLIATION, FOLIATED FRACTURES, FRACTURED GNEISS	diag	MODERATELY QUARTZ	mod
HIGHLY HORNBLLENDE INCLUSIONS	dio	RELATIVELY SOUND	qtz
	fol	SCHIST	rel snd
	fract, frctd	SLIGHTLY UNWEATHERED	sch
	gns	VARIABLE	sli
	h1	VERTICAL	unwthrd
	horn	WEATHERED	vert
	inclu		wthrd

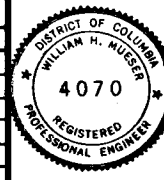
DESIGNED	DATE	REFERENCE DRAWINGS	REVISIONS		<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> <b>MUESER - RUTLEDGE - WENTWORTH &amp; JOHNSTON</b> CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK 17, N. Y. SUBMITTED <i>William H. Mueser</i>	DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT	<b>ROCKVILLE ROUTE (AOII)</b> <b>LOGS OF AM-SERIES BORINGS Nos. AM-1 to AM-2</b>
DRAWN	DATE	DESCRIPTION	DATE				
CHECKED	DATE	DESCRIPTION	DATE				
APPROVED	DATE	DESCRIPTION	DATE				

SCALE: VERT. 0' 2' 4' 8' 12'

DRAWING NO. F-A-105



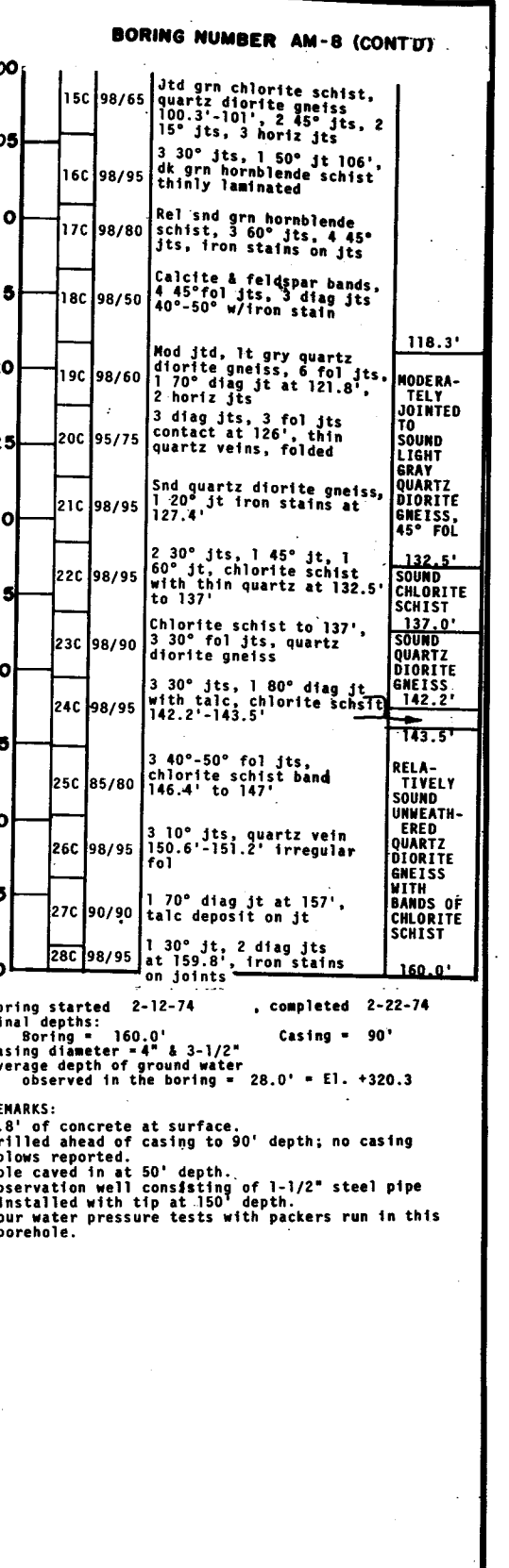
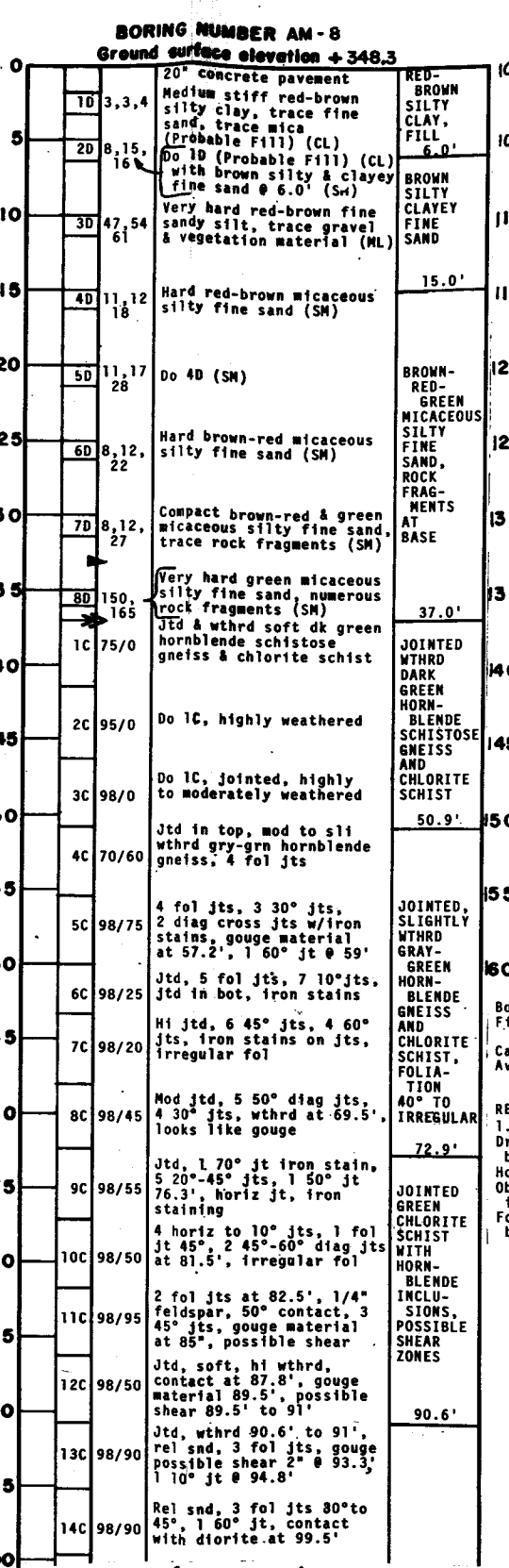
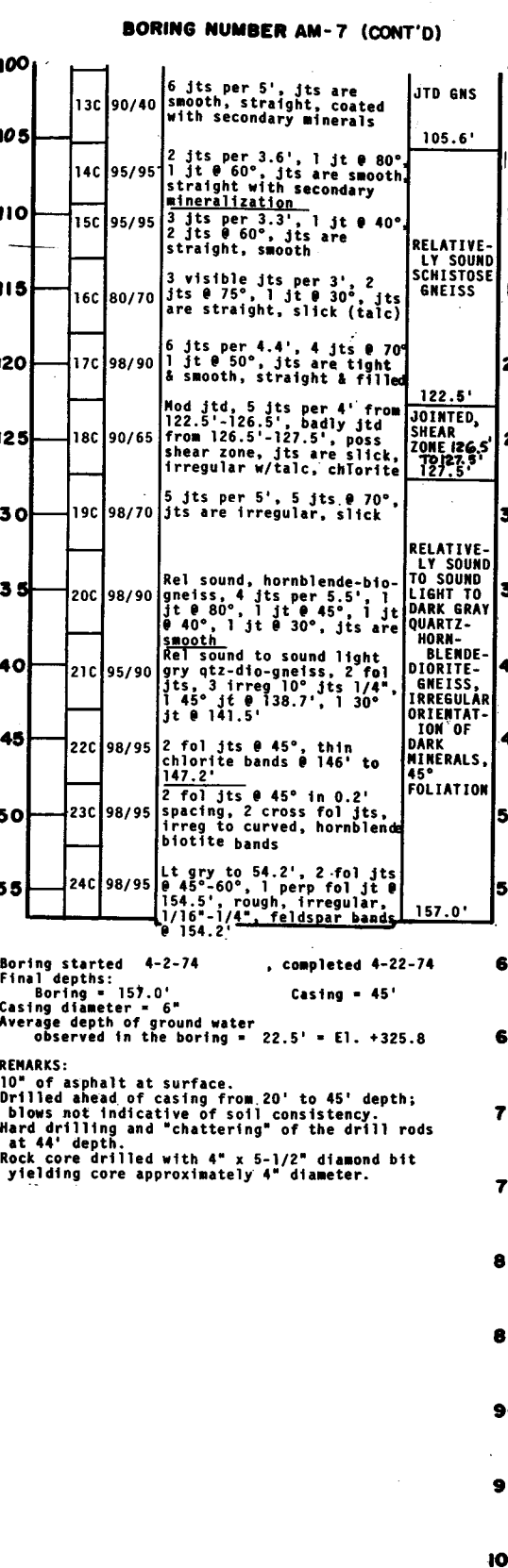
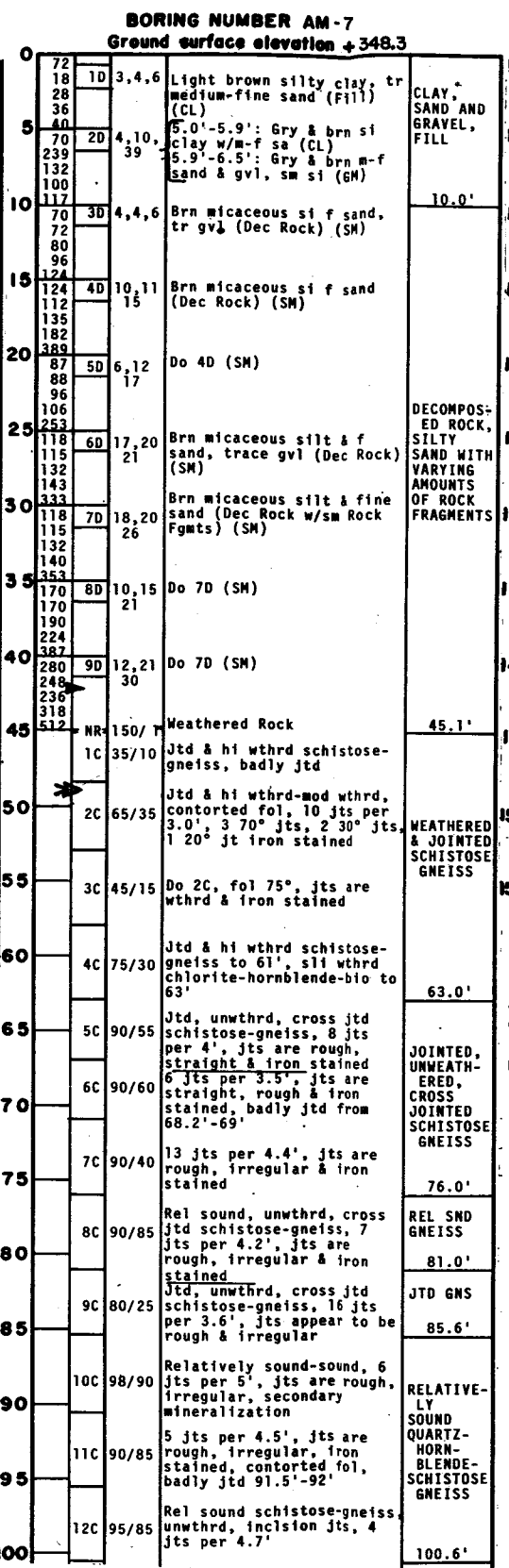
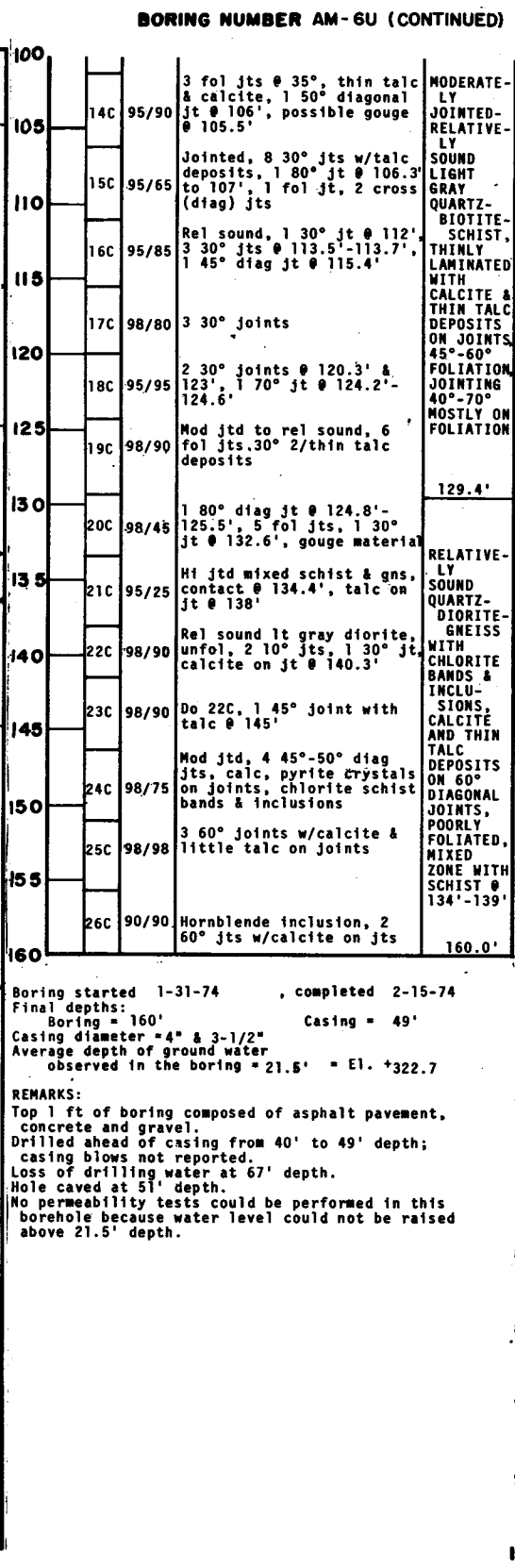
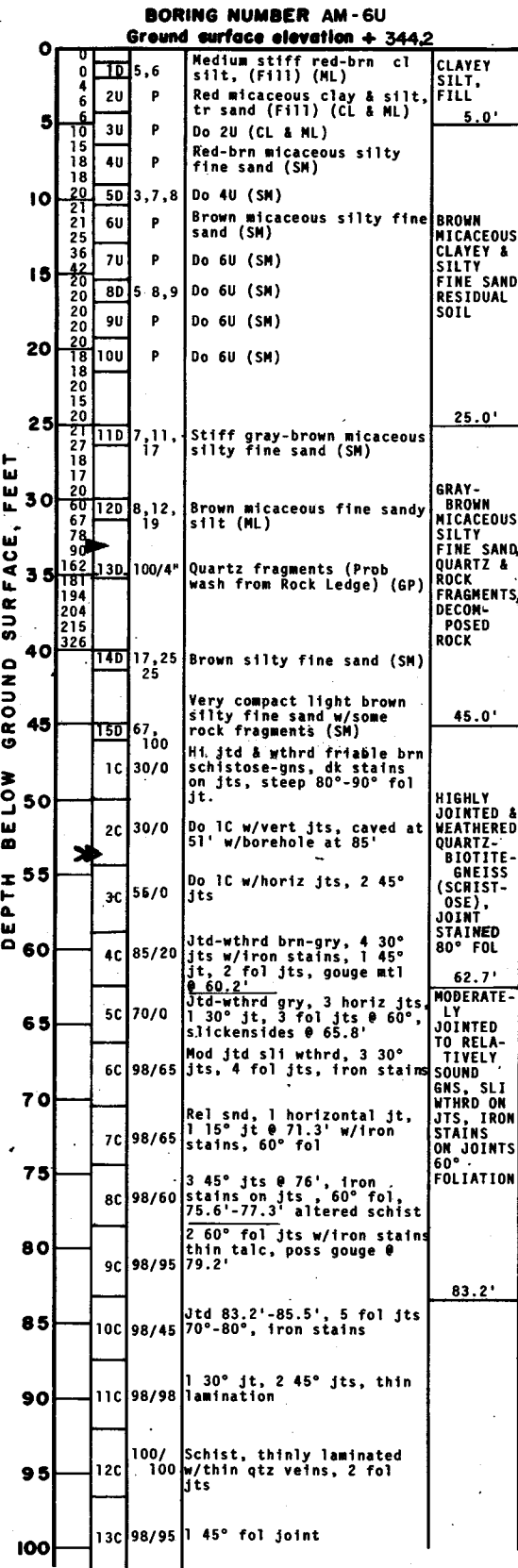
DESIGNED		DATE		REFERENCE DRAWINGS		REVISIONS		WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY		ROCKVILLE ROUTE (A011)	
DRAWN	IHL & AR	DATE	12/74	NUMBER	F-1	DESCRIPTION	General Notes & Legend	MUESER • RUTLEDGE • WENTWORTH & JOHNSTON		LOGS OF AM-SERIES BORINGS Nos. AM-3U to AM-5	
CHECKED	SLT & JPG	DATE	12/74	NUMBER	F-A-96 to F-A-104	DESCRIPTION	Plans & Geological Sections	CONSULTING ENGINEERS		SCALE	
APPROVED		DATE						415 MADISON AVE., NEW YORK 17, N.Y.		DRAWING NO. F-A-106	



WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
MUESER • RUTLEDGE • WENTWORTH & JOHNSTON  
CONSULTING ENGINEERS  
415 MADISON AVE., NEW YORK 17, N.Y.

DE LEUW, CATHAR & COMPANY  
GENERAL ENGINEERING CONSULTANT  
HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

ROCKVILLE ROUTE (A011)  
LOGS OF AM-SERIES BORINGS Nos. AM-3U to AM-5  
SCALE: VERT. 0' 3' 6' 12'  
DRAWING NO. F-A-106



(CONTINUED)

(CONTINUED)

(CONTINUED)

DESIGNED	DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION
DRAWN IHL & AR	12/74	F-1	General Notes & Legend			
CHECKED SLT & JPS	12/74	F-A-107	Plans & Geological Sections			
APPROVED						

REGISTERED PROFESSIONAL ENGINEER

### WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

**MUESER - RUTLEDGE - WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N. Y.

DE LEUW, CATHAR & COMPANY  
GENERAL ENGINEERING CONSULTANT

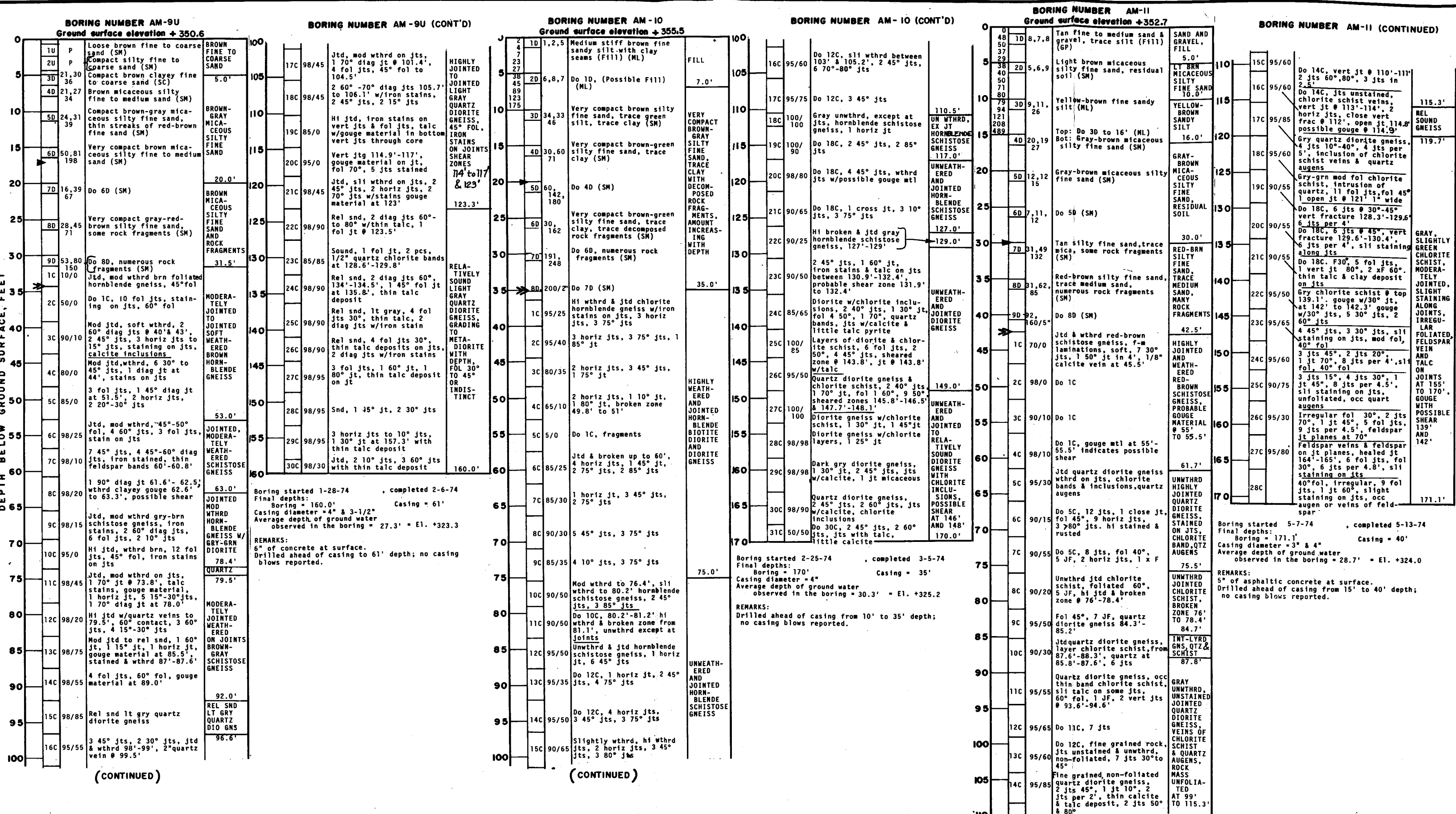
HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

### ROCKVILLE ROUTE (A011)

#### LOGS OF AM-SERIES BORINGS Nos. AM-6U to AM-8

SCALE: VERT. 1" = 10'

DRAWING NO. F-A-107



DESIGNED	DATE	REFERENCE DRAWINGS		REVISIONS		SUBMITTED	APPROVED	SCALE	DRAWING NO.
		NUMBER	DESCRIPTION	DATE	BY				
DRAWN	12/74	F-1	General Notes & Legend			4070	WILLIAM H. WEESE	1" = 10'	F-A-108
CHECKED	12/74	F-A-96 to F-A-104	Plans & Geological Sections						
APPROVED									

**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**WEESE - RUTLEDGE - WENTWORTH & JOHNSTON**  
 CONSULTING ENGINEERS  
 415 MADISON AVE. NEW YORK 17, N. Y.  
*William H. Weese*

**ROCKVILLE ROUTE (A011)**

**LOGS OF AM-SERIES BORINGS Nos. AM-9U to AM-11**  
 HARRY WEESE & ASSOCIATES  
 GENERAL ARCHITECTURAL CONSULTANT

BORING NUMBER AM-12 Ground surface elevation + 351.1		BORING NUMBER AM-12 (CONT'D)		BORING NUMBER AM-13 Ground surface elevation + 266.1		BORING NUMBER AM-14 Ground surface elevation + 265.2		BORING NUMBER AM-14 (CONTINUED)							
0	1D 5.8, 11	Red-brown micaceous silty fine sand, trace clay, trace asphalt (Possible Fill) (SM)	YELLOW-RED-BROWN MICACEOUS SAND, TR ASPHALT & ROCK FRAGMENTS FILL 10.0'	0	23 10 9.13, 16	Brown silty fine sand (Fill) (SM)	BROWN-TAN SILTY FINE SAND, TRACE MEDIUM SAND	0	13 1D 2.3, 4	Brown medium stiff fine sandy silt & silty fine sand, trace mica (Fill) (SM)	FILL 2.0'	100	19C 100/30	Do 17C, hi jtd wtl, sm pink feldspar 0.5' chlorite bands @ 100.5' to 103', shear evidence 103'-103.5'	GREENISH-WHITE HIGHLY FRACTURED & SHEARED DIORITIC GNEISS AND ALTERED CHLORITE SCHIST W/SEVERAL FELDSPAR & QUARTZ VEINS, FOLIATION INDISTINCT EVIDENCE OF INTRUSION/SHEARING/DECOMPOSITION, POSSIBLE HIGH-ANGLE MULTIPLE FAULT OR SHEAR ZONES, ATTITUDE CHIEFLY THAT OF FOLIATION
5	2D 4, 6, 8	Medium stiff yellow-brown micaceous silty fine sand, trace rock fragment (Possible Fill) (SM)	105	13C 75/70	1 diag jt 40° slickensided calcite on joint, chlorite prominent	Tan silty fine sand, trace medium sand (Prob Residual Soil) (SM)	5	41 2D 7.7, 11	Brown-red clayey silt, trace fine to medium sand, trace of org. & mica, residual soil (ML)	20.0'	105	20C 100/10	Do 17C, hi jtd & fragmtd wtl, chlorite & feldspar band 105'-107.5', fol indistinct, sheared & crushed wtl 108'-108.5', Grn chloritic schist, partly wthrd, hi jtd, evidence of shear, crushing & decomposition 110.5', 111.5' & 112'		
10	3D 10, 12, 24	Very stiff green-gray fine sand, some micaceous silt, residual soil (SM)	110	14C 98/85	1 pc w/ occasional chlorite patches, garnets occur	Brown silty fine sand, some rock fragments (SM)	10	81 3D 32.48, 47	Medium compact gray-red-brown silty fine sand, some silt layers, trace medium sand (SM)	10.0'	110	21C 98/15	Do 21C, wtl wthrd, crushed & sheared 114.0'-115.5', and dioritic gneiss 116.5'-118' crushed & sheared chlorite		
15	4D 8.14, 19	Hard brown-gray fine sand, some micaceous silt, trace gravel (SM)	115	15C 100/100	2 diag jts, 2 30° jts, sli micaceous, chlorite inclusion	Brown, trace white silty fine-medium sand, few rock fragments (SM)	15	47 4D 15.34, 37	Very compact gray-brown fine to medium sand, little silt (SM)	20.5'	115	22C 90/15	Do 21C, wtl wthrd, crushed & sheared 114.0'-115.5', and dioritic gneiss 116.5'-118' crushed & sheared chlorite		
20	5D 6.13, 24	Compact green-brown micaceous silty fine sand (SM)	120	16C 95/85	5 diag jts, 2 40°, 2 20°, 1 10° jts w/ chlorite, calcite & pyrite, chlorite inclusions prominent	Do 4D (SM)	20	55 5D 22.23, 57	Highly jointed & weathered brown quartz-diorite-gneiss	24.0'	120	23C 75/0	Alt band diorite gneiss & chlorite schist, crushed wtl 119'-119.5' & 121'-121.5', rock quality very poor, hi jtd		
25	6D 9.16, 30	Do 5D (SM)	125	17C 98/80	7 diag jts, jts w/calcite & chlorite, pyrite also occurs, possible shear at 130'	Highly jointed & weathered gray-brown quartz-diorite-gneiss, 45° foliation & jointing, vertical joints	25	57 4D 37	Do 1C	HIGHLY JOINTED & WEATHERED QUARTZ-DIORITE-GNEISS WITH IRON STAINS ON JOINTS, 65° FOLIATION	125	24C 70/0	Do 24C, hi jtd, crushed & sheared w/sm wthrd 126'-126.5', 127'-128' & 129'-130', sm schistose diorite 130'-130.5'		
30	7D 18.33, 43	Very compact green-gray micaceous silty fine sand (SM)	130	18C 95/70	5 diag jts, calcite, mica & little talc on jts, 2 jts filled between 131.3' to 132.3'	Do 3C, highly jointed & weathered	30	59 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	45.0'	130	25C 85/0	Do 24C, hi jtd, crushed & sheared w/sm wthrd 126'-126.5', 127'-128' & 129'-130', sm schistose diorite 130'-130.5'		
35	8D 35.41, 48	Very compact green-gray micaceous silty fine sand (SM)	135	19C 98/70	Gry-grn, 3 diag jts, 3 parallel jts, calcite on jts, little talc, pyrite occurrence	Highly jointed & weathered gray-brown quartz-diorite-gneiss	35	61 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	60.0'	135	26C 90/10	Do 24C, hi jtd, crushed & sheared w/sm wthrd 126'-126.5', 127'-128' & 129'-130', sm schistose diorite 130'-130.5'		
40	9D 150/6	Very compact green-gray micaceous silty fine sand, numerous rock fragments (SM)	140	20C 90/65	Gray-grn quartz hornblende gneiss, 1 diag jt, 1 filled jt starting @ 140.6', calcite & pyrite on jts	Highly jointed & weathered gray-brown quartz-diorite-gneiss	40	63 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	60.0'	140	27C 96/60	Do 24C, hi jtd, crushed & sheared w/sm wthrd 126'-126.5', 127'-128' & 129'-130', sm schistose diorite 130'-130.5'		
45	10D 150/6	Hard dark gray micaceous silty fine sand & rock fragments (SM)	145	21C 96/60	6 gray diorite gneiss, 1 diag jt, 1 parallel jt, calcite on jt, chlorite also occurs	Highly jointed & weathered gray-brown quartz-diorite-gneiss	45	65 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	60.0'	145	28C 98/98	Do 24C, hi jtd, crushed & sheared w/sm wthrd 126'-126.5', 127'-128' & 129'-130', sm schistose diorite 130'-130.5'		
50	11D 150/2	Very compact dark gray micaceous silty fine sand, trace gravel and rock fragments (SM)	150	22C 98/95	2 diag jts, jts w/calcite & pyrite, chlorite occurs	Highly jointed & weathered gray-brown quartz-diorite-gneiss	50	67 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	60.0'	150	29C 98/90	Do 24C, hi jtd, crushed & sheared w/sm wthrd 126'-126.5', 127'-128' & 129'-130', sm schistose diorite 130'-130.5'		
55	12C 60/0	Gry-grn biotite gneiss, hi wthrd, frag, iron stained, 2 diag jts, gouge wtl 56'-57.2'	155	23C 98/98	4 diag jts, calcite on jts, rock jtd at top, possible slickenside at 150.4', quartz occurs	Highly jointed & weathered gray-brown quartz-diorite-gneiss	55	69 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	60.0'	155	26C 95/80	Do 24C, hi jtd, crushed & sheared w/sm wthrd 126'-126.5', 127'-128' & 129'-130', sm schistose diorite 130'-130.5'		
60	13C 95/30	Do 1C, 2 diag jts, chlorite inclusion, rock fragmented at 59.5'-60', highly weathered	160	24C 98/98	1 diag jt w/little calcite pyrite on broken faces	Highly jointed & weathered gray-brown quartz-diorite-gneiss	60	71 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	60.0'	160	27C 98/60	Do 24C, hi jtd, crushed & sheared w/sm wthrd 126'-126.5', 127'-128' & 129'-130', sm schistose diorite 130'-130.5'		
65	14C 90/70	Do 1C w/chlorite inclusions, 5 diag jts, iron stained	Boring started 3-5-74, completed 3-22-74 Final depths: Boring = 160', Casing = 61' Casing diameter = 3" & 4" Average depth of ground water observed in the boring = 26.5' = El. +324.6		160	25C 98/90	2 diag jts, jts w/calcite & pyrite, chlorite occurs	65	73 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	65	30C 90/70	Change 57' to hornblende schistose gneiss severely jtd & wthrd, 60.5'-61' coarse grained, well fol, occ chlorite veins, 70' fol		
70	15C 90/70	Gry-grn diorite gneiss w/chlorite, 2 diag jts	REMARKS: 6" of asphalt at surface. Drilled ahead of casing from 30' to 61' depth; no casing blows reported. Loss of drilling water at 99' depth. Drilling water returned at 112' depth.		165	26C 95/80	4 diag jts, calcite on jts, rock jtd at top, possible slickenside at 150.4', quartz occurs	70	75 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	70	31C 96/25	Change 61'-62.5' to granitic gneiss, change 62.5' to 65' horn sch gneiss & chlorite sch, poss. fault		
75	16C 90/70	Do 4C, w/chlorite inclusion, 1 diag jt 70°, calcite on joint	REMARKS: Boring started 11-19-73, completed 11-28-73 Final depths: Boring = 106.7', Casing = 25' Casing diameter = 4" Average depth of ground water observed in the boring = 9.4' = El. +256.7		170	27C 98/60	1 diag jt w/little calcite pyrite on broken faces	75	77 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	75	32C 95/50	Do 11C, rel snd, mod jtd, 3 fol jts, 8 diag jts, mainly chlorite schist, occ feldspar veins		
80	17C 100/100	1 jt filled @ 81.8', pyrite & garnet apparent	REMARKS: Boring bit was broken off at 106.7' depth, so hole was abandoned.		175	28C 98/98	2 diag jts, jts w/calcite & pyrite, chlorite occurs	80	79 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	80	33C 98/40	Do 13C, rel snd, mod jtd quartz diorite		
85	18C 95/90	2 diag jts, 1 45° jt, sli micaceous & iron stained, 1 30° jt at bot, chlorite prominent			180	29C 98/90	5 diag jts, 1 filled jt at top, calcite & pyrite on jts	85	81 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	85	34C 98/50	Do 13C, rel snd, mod jtd quartz diorite		
90	19C 98/90	2 diag jts, 1 50°, 1 25°, jts sli micaceous & talcy, pyrite & garnet occurrence, chlorite @ 90.5'-91.2'			185	30C 98/90	4 diag jts, calcite on jts, rock jtd at top, possible slickenside at 150.4', quartz occurs	90	83 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	90	35C 98/35	Do 13C, rel snd, mod jtd quartz diorite		
95	100C 98/95	2 diag jts, 1 40°, 1 30°, calcite on jts of 40°, chlorite inclusions			190	31C 90/55	6 gray diorite gneiss, 1 diag jt, 1 parallel jt, calcite on jt, chlorite also occurs	95	85 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	95	36C 98/20	Do 13C, rel snd, mod jtd quartz diorite		
100	110C 98/80	Chlorite inclusions prominent, 3 diag jts with secondary mineral			195	32C 98/60	Gray-grn quartz hornblende gneiss, 1 diag jt, 1 filled jt starting @ 140.6', calcite & pyrite on jts	100	87 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	100	37C 98/35	Do 13C, rel snd, mod jtd quartz diorite		
100	120C 98/55	2 diag jts at bot, 1 40°, 1 30°, chlorite inclusion			200	33C 90/25	6 gray diorite gneiss, 1 diag jt, 1 parallel jt, calcite on jt, chlorite also occurs	105	89 5D 22.23, 57	Highly jointed & weathered gray-brown quartz-diorite-gneiss	105	38C 90/25	Do 13C, rel snd, mod jtd quartz diorite		

DESIGNED	DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION	<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> <b>MUESER - RUTLEDGE - WENTWORTH &amp; JOHNSTON</b> CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK 17, N. Y. SUBMITTED <i>William H. Mueser</i>	DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT	<b>ROCKVILLE ROUTE (AO11)</b> <b>LOGS OF AM-SERIES BORINGS Nos. AM-12 to AM-14</b> SCALE VERT. 0" = 10' DRAWING NO. <b>F-A-109</b>
DRAWN	DATE	DESCRIPTION	DATE	BY	DESCRIPTION				
CHECKED	DATE	DESCRIPTION	DATE	BY	DESCRIPTION				
APPROVED	DATE	DESCRIPTION	DATE	BY	DESCRIPTION				

BORING NUMBER AM-15		Ground surface elevation +260.6	
10	1D 3, 3.4	Medium compact brown fine sandy silt (Fill) (ML)	
15	2D 13, 20	Compact brown silty fine sand (Residual Soil) (SM)	
20	3D 6, 12	Do 2D (SM)	
25	4D 20, 42	Do 2D, very compact (SM)	
30	5D 20, 17	Do 2D (SM)	
35	6D 20, 34	Do 2D (SM)	
40	7D 18, 35	Do 2D, transition to weathered rock (SM)	
45	8D 140/6	Brown silty coarse-fine sand, numerous rock fragments (SM)	
50	9D 150/6	Very compact green-brown silty fine sand with some rock fragments (SM)	
55	10D 150/6	Do 11D (SM)	
60	11D 150/6	Do 11D (SM)	
65	12D 150/6	Do 11D (SM)	
70	13D 150/6	Do 11D (SM)	
75	14D 150/6	Do 11D (SM)	
80	15D 150/6	Probably Do 11D (SM)	
85	16D 150/6	Probably Do 11D (SM)	
90	17D 150/6	Probably Do 11D (SM)	
95	18D 150/6	Probably Do 11D (SM)	
100	19D 150/6	Probably Do 11D (SM)	
105	20D 150/6	Probably Do 11D (SM)	
110	21D 150/6	Probably Do 11D (SM)	

BORING NUMBER AM-16 (CONTINUED)		Ground surface elevation +251.5	
105	16C 96/65	Unweathered, moderately jointed, 5 diagonal joints, 2 foliation joints, 1 vertical joint 104'-104.5'	& WITH FEW JOINTS, MOST FRESH JOINTS & FRACTURES SUGGEST BREAKING OF CORES DURING DRILLING, HENCE SOUND ROCK INDICATED, SOME PYRITE EVIDENT
110	17C 95/85	Relatively sound, unweathered, moderately jtd, 5 diagonal joints, 2 vertical joints, quartz veins, jts relatively fresh	
115	18C 85/55	Mod jtd, core in long pcs, broken during drilling operation, rel fresh jts	
120	19C 96/90	Sound, unweathered, slightly jointed, upper joints to 114.5'	
125	20C 95/65	Unweathered, jointed, 1 joint only @ 120', 1 vert frct @ 119.8'-120.5'	
130	21C 95/95	Sound, unweathered, 1 fresh joint due to coring	
135	22C 98/95	Sound unweathered, 3 relatively fresh joints	
140	23C 96/60	Change to jointed, 10 joints slightly weathering & crushing 129'-130', calcite & talc	

BORING NUMBER AM-17		Ground surface elevation +234.0'	
10	1D 3, 3.5	Medium stiff red-brown fine sandy silt, little red clay, trace mica (Fill) (ML)	
15	2D 10, 17	Very compact gray silty fine sand, little clay, trace 1 pc medium gravel, trace organics (Possible Fill) (SC-SM)	
20	3D 14, 14	Medium compact light gray-brown silty fine sand, residual soil (SM)	
25	4D 66, 66	Very compact light gray-brown silty fine sand, little fine mica, trace hornblende (SM)	
30	5D 139, 174	Soft rock	
35	6D 38, 61	Do 4D (SM)	
40	7D 67, 61	Do 4D (SM)	
45	8D 150/3	Fine sand from rock	
50	9D NR	Soft rock	
55	10D NR	Soft rock	
60	11D 150/2	Very compact brown fine sand, some silt (SM)	
65	12D NR	Soft rock	
70	13D NR	Soft rock	
75	14D NR	Soft rock	
80	15D NR	Soft rock	
85	16D NR	Soft rock	
90	17D NR	Soft rock	
95	18D NR	Soft rock	
100	19D NR	Soft rock	
105	20D NR	Soft rock	

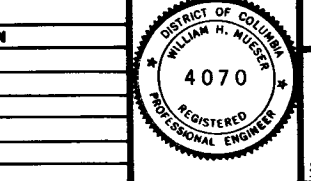
BORING NUMBER AM-18		Ground surface elevation +283.6	
10	1D 2, 3.4	Medium stiff red-brown silty clay, trace fine-coarse sand (Fill) (CL)	
15	2D 6, 6.7	Red-brown silty fine sand (Residual Soil) (SM)	
20	3D 6, 10	Brown silty fine sand (SM)	
25	4D 4, 7.9	Do 3D (SM)	
30	5D 13, 23	Do 3D (SM)	
35	6D 35, 100	Very compact tan & gray silty fine sand (SM)	
40	7D 23, 17	Tan & white fine sandy silt (ML)	
45	8D 100/6	Rock fragments (SP)	
50	9D NR	Attempted core, transition to weathered bedrock	
55	10D NR	Attempted core, transition to weathered bedrock	
60	11D NR	Attempted core, transition to weathered bedrock	
65	12D NR	Attempted core, transition to weathered bedrock	
70	13D NR	Attempted core, transition to weathered bedrock	
75	14D NR	Attempted core, transition to weathered bedrock	
80	15D NR	Attempted core, transition to weathered bedrock	
85	16D NR	Attempted core, transition to weathered bedrock	
90	17D NR	Attempted core, transition to weathered bedrock	
95	18D NR	Attempted core, transition to weathered bedrock	
100	19D NR	Attempted core, transition to weathered bedrock	
105	20D NR	Attempted core, transition to weathered bedrock	

BORING NUMBER AM-17 (CONTINUED)		Ground surface elevation +234.0'	
105	18C 98/30	Highly jointed, vertical jointing 104.4' to 108.0', 1 60° joint, talc joints	
110	19C 95/90	Rel sound, 1 60° jt 112.8', green chlorite bands at 109'-110', calcite veins	
115	20C 100/100	Relatively sound, 1 60° joint at 118.0', thin talc calcite veins	
120	21C 100/100	Sound gray quartz diorite gneiss	

BORING NUMBER AM-18 (CONTINUED)		Ground surface elevation +283.6	
105	16C 100/100	Sound green-light gray 1 30° joint at 108.0', 2 pieces	
110	17C 98/90	2 45° joints at 111.6' & 112.5', 1 30° joint at 113.0'	
115	18C 80/80	1 45° joint at 114.7'; 1 30° joint at 117.5'; 1 20° joint with thin calcite deposits	
120	19C 98/90	2 60° joints at 120.4' & 122.0'; 3 15° joints; 3 30° joints; green-gray chlorite inclusions	

DESIGNED	DATE	NUMBER	DESCRIPTION
IHL	12/74	F-1	General Notes & Legend
SLT & JPG	12/74	F-A-86 to F-A-106	Plans & Geological Sections

DATE	BY	DESCRIPTION



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**MUESER • RUTLEDGE • WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N. Y.

DE LEUW, CATHAR & COMPANY  
GENERAL ENGINEERING CONSULTANT

HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

APPROVED

**ROCKVILLE ROUTE (AOI)**

**LOGS OF AM-SERIES BORINGS Nos. AM-15 to AM-18**

SCALE: VERT. 0' 5' 10'

DRAWING NO. **F-A-110**

**BORING NUMBER AM-18A**  
Ground surface elevation +253.4

**BORING NUMBER AM-19**  
Ground surface elevation +243.0

**BORING NUMBER AM-20**  
Ground surface elevation +242.3

**BORING NUMBER AM-21**  
Ground surface elevation +247.2

**BORING NUMBER AM-22**  
Ground surface elevation +251.6

**BORING NUMBER AM-23**  
Ground surface elevation +247.4

Table for Boring AM-18A with columns for depth (0-100), soil type (e.g., Brown silty fine-medium sand), and fill material (FILL).

Table for Boring AM-19 with columns for depth (0-95), soil type (e.g., Reddish brown silty clay), and geological notes.

Table for Boring AM-20 with columns for depth (0-90), soil type (e.g., Medium stiff brown-red clay), and geological notes.

Table for Boring AM-21 with columns for depth (0-80), soil type (e.g., Medium stiff brown fine sandy silt), and geological notes.

Table for Boring AM-22 with columns for depth (0-70), soil type (e.g., Stiff brown-red silty clay), and geological notes.

Table for Boring AM-23 with columns for depth (0-70), soil type (e.g., Stiff red silty clay), and geological notes.

Boring started 9-24-73, completed 9-28-73. Final depths: Boring = 100.0', Casing = 55'. Casing diameter = 3-1/2" & 4". Average depth of ground water observed in the boring = 32.0' = E1. +221.4

Boring started 10-1-73, completed 10-4-73. Final depths: Boring = 99.0', Casing = 50'. Casing diameter = 3-1/2" & 4". Average depth of ground water observed in the boring = 19.2' = E1. +223.8

Boring started 11-7-73, completed 11-12-73. Final depths: Boring = 90.0', Casing = 30'. Casing diameter = 3-1/2" & 4". Average depth of ground water observed in the boring = 20.0' = E1. +222.3

Boring started 11-13-73, completed 11-16-73. Final depths: Boring = 80.0', Casing = 19'. Casing diameter = 4". Average depth of ground water observed in the boring = 28.0' = E1. +219.2

Boring started 9-13-73, completed 9-19-73. Final depths: Boring = 70.0', Casing = 9'. Casing diameter = 6". Average depth of ground water observed in the boring = 31.0' = E1. +220.6

Boring started 10-20-73, completed 10-28-73. Final depths: Boring = 70.0', Casing = 25'. Casing diameter = 6". Average depth of ground water observed in the boring = 31.0' = E1. +216.4

REMARKS: Drilled ahead of casing from 15' to 55' depth; casing blows not reported. Caving in of boring occurred at approximately 65' depth.

REMARKS: Drilled ahead of casing from 5' to 50' depth; casing blows not reported. Four water pressure tests with packers run in this borehole.

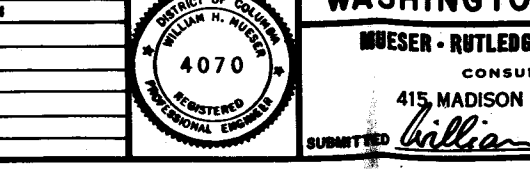
REMARKS: Drilled ahead of casing from 10' to 30' depth; casing blows from 10' to 25' depth not indicative of soil consistency. Four water pressure tests with packers run in this borehole.

REMARKS: Drilled ahead of casing from 5' to 9' depth; casing blows not reported. Drilling water at 23.5' to 28.5' became brown-yellow in color. Intrusive ridge of granite diorite gneiss indicated near surface.

REMARKS: Drilled ahead of casing from 20' to 25' depth; casing blows not reported. Thin clay deposits observed on joint of rock core No. 1C. Rock core drilling by 4" x 5-1/2" diamond drill bit, providing core approximately 4" in diameter.

REFERENCE DRAWINGS table with columns: NUMBER, DESCRIPTION, DATE, BY

REVISIONS table with columns: DATE, BY, DESCRIPTION



WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
MUESER - RUTLEDGE - WENTWORTH & JOHNSON  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17. N. Y.  
SUBMITTED: *William H. Mueser*

DE LEUW, CATHER & COMPANY  
GENERAL ENGINEERING CONSULTANT  
HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

ROCKVILLE ROUTE (AO11)  
LOGS OF AM-SERIES BORINGS Nos. AM-18A to AM-23  
SCALE: VERT. 1" = 5'  
DRAWING NO. F-A-III





BORING NUMBER AM-30		Ground surface elevation +310.4	
10	1D	3,6, 11	Medium compact brown silty fine sand, little fine mica (SM)
12	2D	12,12, 12	Do 1D, trace hornblende fragments (SM)
15	3D	100/6	Very compact brown silty fine to medium sand (SM)
20	4D	25,100	Do 3D, with some silt layers, traces of hornblende & feldspar fragments (SM)
25	1C	95/15	Hi jtd & wthrd brown & lt gry diorite, horz jts, thin "poker-chip" pcs 20'-21.5', 22.2'-22.8'
30	2C	95/30	Wthrd in top on horz jts at 27', no fol, 5 angular jts, iron stain in top, jtd pcs in bot'
35	3C	95/70	Jtd lt gry diorite gneiss 3 horz jts, 2 angular jts w/iron stain
40	4C	90/60	6 diag jts, 3 horz jts, iron stain on jts
45	5C	95/70	3 diag jts, 2 horz jts, iron stain on jts, quartz inclusion
50	6C	95/90	Rel sound quartz diorite gneiss, 2 jts
55	7C	95/90	Rel sound lt gray diorite gneiss, 4 horz jts, chlorite inclusions
60	8C	90/90	2 horz jts (3 pcs) chlorite band at 54.3', altered
65	9C	95/90	2 horz jts, 3 diag jts, iron stain on diag at 59.2'
70	10C	95/90	3 horz jts, 2 diag jts
75	11C	90/70	4 horz jts, 3 diag jts
80	12C	95/75	3 horz jts, 5 diag jts, irregular foliation, chlorite 72'-73'
85	13C	95/90	2 horz jts, 6 diag jts, talc on jt at 73.3'
90	14C	95/95	Rel sound rock, looks crystalline, not foliated very hard rock, 1 ft/hr drilling rate
95	15C	98/95	Rel snd lt gry diorite gneiss, chlorite band 81' to 81.6', 4 horz jts, tr garnet, chlorite inclusions 1 diag jt @ 85.2', thin talc deposit, chlorite band 87.4'-87.7'
100	16C	80/80	Rel sound lt gry-grn quartz diorite gneiss, 2 horz jts
105	17C	85/85	Rel sound lt gry, dark minerals, biotite hornblende, in core, tr garnet 4 horz jts
110	18C	95/80	Sound light gray, 1 jt almost horizontal
115	19C	100/100	

BORING NUMBER AM-30 (CONTINUED)		Ground surface elevation +310.4	
105	20C	95/65	Mod jtd, 3 horz jts, 1 irregular jt 101.9' to 103' w/thin talc deposit 2 diag jts @ 104', 105.5' thin talc deposit
110	21C	85/80	Rel snd, 2 horz jts, 3 diag jts w/thin talc deposit on diag, chlorite inclusion
115	22C	100/85	1 diag jt at 110.5', 2 diag jts at 112', 112.4'
120	23C	95/80	5' diag jts, 1 horz jt
125	24C	95/75	Mod jtd, 4 diag jts, 1 horz jt w/thin talc deposit
130	25C	95/90	Sound, 2 diag jts, 2 horz jts, chlorite inclusion, dip 45°
135	26C	85/80	Rel snd, 2 diag jts, 2 horz jts, thin chlorite veins (1/16" @ 60°, some pyrite, 60° fol
140	27C	90/88	2 horz jts, 1 diag jt angular inclusion, 2' chlorite band at 132'
145	28C	98/95	Rel snd, 2 horz jts, 1 diag jt
150	29C	98/95	2 horz jts, 2 diag jts, rectangular inclusions
155	30C	98/90	1 horz jt, 2 20° diag jts 2 50° jts @ 144' & 144.5' with thin talc
160	31C	98/70	Mod jtd, hi jtd 144.5'-146.5', 5 diag jts with 1/4"-1/2" calcite deposit thin talc
165	32C	98/95	Sound massive gray diorite gneiss
170	33C	98/95	2 horz jts, 2 diag jts, lenticular inclusions, thin jt plane 154', calcite filled, slight fol 45°-60°
175	34C	98/95	Sound, 3 diag jts, 1 at 162.5' w/thin calcite deposit, no definite fol, very few inclusions, thin pyrite dep on jt faces
180	35C	98/95	Rel snd, 3 diag jts, 1 piece = 3.3'
185	36C	98/90	2 horz jts, 2 diag jts, thin horz calcite filled joints
190	Boring started 10-4-73, completed 10-16-73 Final depths: Boring = 170.0' Casing = 22' Casing diameter = 3-1/2" & 4" Average depth of ground water observed in the boring = 13.4' = El. +297.0		
195	REMARKS: Drilled ahead of casing from 10' to 22' depth; casing blows not reported. Very hard rock coring at approximately 77' depth, drilling rate equals 1 foot per hour.		

BORING NUMBER AM-34		Ground surface elevation +312.3	
10	1D	7,7,8	Medium stiff red-brown silt, trace fine sand, trace clay (ML)
5	2D	5,13, 14	Stiff gray-brown-red micaceous fine sandy silt, little clay (ML)
10	3D	15,23 32	Compact gray-green silty fine sand with streaks of brown silty fine sand, along weathered joint planes (SM)
15	4D	15,25 38	Do 3D, gray (SM)
20	5D	100/7	Do 3D, gray, numerous rock fragments (SM)
25	1C	60/0	Jtd & wthrd gry-grn diorite w/chlorite 5 30° jts, silty fine sand on jts
30	2C	85/35	Do 1C, 9 horz jts, 3 diag jts
35	3C	65/15	Do 1C, 7 horz jts, 4 diag jts, staining at jts
40	4C	60/10	Wthrd to 36', friable gry-grn diorite w/chlorite 4 horz jts, 3 diag jts, staining at jts
45	5C	95/60	Jtd sli wthrd gry-grn diorite w/chlorite, 5 horz jts, 7 diag jts
50	6C	98/60	5 horz jts, 4 diag jts, staining & weathering at 45'
55	7C	98/90	Unwthrd gry-grn quartz diorite w/4 horz & 2 diag jts, staining at jts
60	8C	95/70	Darker minerals, granitic intrusion at 57', 3 horz jts, 2 diag jts, talc at jts
65	9C	98/75	9 horz jts, 1 diag jt, staining & intrusion of quartzite at jts
70	10C	95/80	3 horz jts, 2 diag jts, pyrite & hornblende at jts clay on core @ 64'
75	11C	98/90	2 horz jts, 3 diag jts, talc at joints
80	12C	98/80	3 horz jts, 4 diag jts
85	13C	98/90	Darker minerals as biotite, 4 horz jts, 1 diag jt
90	14C	90/55	4 diag jts, 4 vert jts, calcite & talc at jts, fracturing zone 80' to 81'
95	15C	95/75	6 horz jts, 2 diag jts, diorite 80' to 82'
100	16C	95/85	Quartz diorite core white with black spots, presence of biotite, 3 horz jts, 1 diag jt
105	17C	98/85	Blocky, 7 horz jts, granitic intrusion from 89' to 91.5'
110	18C	98/45	6 horz jts, 1 diag jt, granitic intrusion from 93' to 95'
115	19C	100/100	2 horz jts, 2 diag jts
120	20C	98/70	3 horz jts, 3 diag jts, thin calcite veins at jts, broken @ 101.5' to 102', hornblende increases below 101'

BORING NUMBER AM-34 (CONT'D)		Ground surface elevation +312.3	
105	21C	98/98	Jtd, 3 diag jts
110	22C	95/75	1 horz jt, 6 diag jts, calcite veins at jts, fractured 110' to 111'
115	23C	100/90	2 horz jts, 3 diag jts
120	24C	95/90	5 diag jts
125	25C	100/100	Rel snd, 1 horz jt, 1 diag jt
130	26C	98/95	Rel snd, 2 diag jts
135	27C	98/90	More biotite, 1/2" quartz vein @ 132.8', 4 horz jts, 1 diag jt
140	28C	98/98	Increased biotite, 2 horz jts, 2 diag jts
145	29C	98/90	Increased biotite, 2 diag jts, 2 horz jts, talc & pyrite at jts, fol 45°, intrusion of quartz
150	30C	100/100	Rel sound rock
155	31C	98/98	2 diag jts, 1 horz jt, calcite, pyrite & chlorite at jts
160	32C	98/98	2 diag jts, 2 horz jts, traces of pyrite at jts
165	33C	98/98	3 diag jts, trace of pyrite
170	34C	98/88	3 diag jts, trace of pyrite & talc at jts
175	35C	98/70	Aplite intrusion 165'-166' 5 diag jts, 3 horz jts, pyrite in jts, 7 jts between 167'-168'
180	36C	95/95	3 diag jts, 3 horz jts, talc & pyrite at jts
185	37C	98/70	6 diag jts, 1 horz jt, pyrite & talc at jts, presence of quartzite
190	Boring started 11-26-73, completed 12-10-73 Final depths: Boring = 178.5' Casing = 25' Casing diameter = 4" Average depth of ground water observed in the boring = 13.6' = El. +298.7		
195	REMARKS: Drilled ahead of casing from 10' to 25' depth; casing blows not indicative of soil consistency.		

BORING NUMBER AM-35		Ground surface elevation +322.4	
0	1D	1,3,3	Stiff red-brown fine sandy silt, trace clay, trace mica (ML)
5	2D	7,8, 10	Brown silty fine to medium sand, little mica (SM)
10	3D	8,11, 11	Do 2D (SM)
15	4D	11,16, 21	Compact brown micaceous silty fine to medium sand (SM)
20	5D	28, 100	Do 4D (SM)
25	Core	NR	Probably soft rock (SM)
30	6D	80, 100	Light gray-brown micaceous silty fine sand with rock fragments (SM)
35	7D	60, 100	Do 6D (SM)
40	NR	50/0	Refusal, top of bedrock
45	1C	80/30	Jtd unwthrd gray quartz diorite, 9 horz jts, 2 diag jts (60° @ 43.7' & 44', little iron stain on jts)
50	2C	98/80	Do 1C, jointed, 1 diag jt @ 46.3' (60°), 2 diag jts @ 47.6' (20°), 1 diag jt @ 48.2' (70°)
55	3C	98/80	Do 1C, jointed, 2 diag jts 2 horz jts
60	4C	95/75	Do 1C, mod jtd, 3 horz jts, 1 diag 60° jt @ 55' sli wthrd on jts
65	5C	98/85	Rel snd diorite, 2 horz jts, 1 vert jt, 2 diag jts, 1 @ 60.5'
70	6C	90/70	Do 5C, 1 steep diag jt 60.8'-61.9', 2 diag jts @ 63.3', 1 diag jt 64.8', chlorite inclusions w/talc irregular foliation
75	7C	95/95	Rel snd, 3 diag jts 10° to 20°
80	8C	100/100	2 pcs, 2 10° diag jts, 1 horz jt
85	9C	98/90	5 pcs, 2 chlorite lenses
90	10C	98/98	4 pcs, 4 horz jts
95	11C	98/90	Rel snd, 1 diag jt @ 76.5', 1 horz jt @ 78.3', no foliation, 2 pcs 1.7'-2.9'
100	12C	98/80	4 horz jtsm 2 diag jts, 1 jt @ 83.5'
105	13C	98/75	Mod jtd, 9 pcs, 1 diag @ 87' (0.4'), 8 horz joints chlorite & feldspar bands 86.3'-86.5' chlorite from 89.5'-90'
110	14C	98/55	6 horz jts, 4 diag jts w/talc, 1 45° jt @ 92', 70° diag jt @ 93'
115	15C	98/98	Rel snd, sli altered, some angular inclusions, chlorite band 97.2'-98.3' irregular foliation
120	16C	98/85	3 horz jts, 2 dfag jts, jointing 101.7'-102.5' w/inclusions, chlorite band 100.2'-100.6'
125	Boring started 10-17-73, completed 11-1-73 Final depths: Boring = 190.0' Casing = 40" Casing diameter = 3-1/2" & 4" Average depth of ground water observed in the boring = 16.8' = El. +305.6		
130	REMARKS: Drilled ahead of casing from 10' to 40' depth; casing blows not reported. Four water pressure tests with packers run in this borehole.		

BORING NUMBER AM-35 (CONT'D)		Ground surface elevation +322.4	
105	17C	98/95	Rel snd, 2 horz jts, 4 10° diag jts, 1 45° jt @ 106.0'
110	18C	90/85	1 horz jt, horz fol, 4 10° diag jts, 1 60° diag jt @ 112'
115	19C	98/95	1 50° diag jt @ 112.5', 1 15° diag jt @ 113.5', 4 horz jts, few inclusions
120	20C	90/90	1 30° jt @ 117', 1 45° jt @ 118.5' w/talc, irregular to horizontal foliation
125	21C	98/95	Rel snd, 3 horz jts, 1 45° jt @ 123.5'
130	22C	98/98	1 30° jt @ 127', 3 horz jts
135	23C	98/98	1 horz jt, 1 60° jt @ 129.7', 1 30° jt @ 131.1'
140	24C	98/98	No joints in core
145	25C	98/80	1 hor jt rel fractured, 137.6'-138.4' rock fragmented, possible shear zone
150	26C	90/85	4 horz jts, rel fresh hard rock
155	27C	98/98	Rel fresh jt @ 146.2'
160	28C	98/98	Sound diorite, no jts, no foliation, occasional quartz bands
165	29C	98/90	Sound & mod jtd, 4 diag jts, 3 horz jts, slight talc, chlorite band 155' to 155.3'
170	30C	98/95	Jtd, mostly fresh jts caused by drilling breaks
175	31C	95/95	Sound, little talc at bot, filled old jts
180	32C	98/98	2 jts, 1 horz, 1 diag, indistinct foliation, minor inclusions
185	33C	95/95	Sound, unjtd, 1 long vert jt 169.2'-171.5', rel fresh jt
190	34C	98/98	Snd, slightly jtd, 4 horz jts, rel fresh, core broken by repeated re-coring attempts
195	35C	98/95	Broken as in 34C
200	36C	98/85	6 fol jts, inclusions, chlorite quartz layers, little talc on joints
205	37C	95/95	Sound, 3 horz jts, quartz veins
210	38C	95/90	Sound, 1/2" vert quartz vein 186'-186.8' & 188.8' to 189.2', vert jt 188.8' to 189', 1 horz jt.
215	Boring started 10-17-73, completed 11-1-73 Final depths: Boring = 190.0' Casing = 40" Casing diameter = 3-1/2" & 4" Average depth of ground water observed in the boring = 16.8' = El. +305.6		
220	REMARKS: Drilled ahead of casing from 10' to 40' depth; casing blows not reported. Four water pressure tests with packers run in this borehole.		

DESIGNED <b>IHL</b>	DATE <b>12/74</b>	REFERENCE DRAWINGS <b>F-1 General Notes &amp; Legend</b>	REVISIONS		<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b>		<b>ROCKVILLE ROUTE (A011)</b>	
DRAWN <b>SLT &amp; JPG</b>	DATE <b>12/74</b>	DESCRIPTION <b>FA-86 to FA-104</b>	DATE		<b>DE LEW, CATHER &amp; COMPANY</b> GENERAL ENGINEERING CONSULTANT			<b>LOGS OF AM-SERIES BORINGS Nos. AM-30 to AM-35</b>
CHECKED <b>SLT &amp; JPG</b>	DATE	DATE	DATE		<b>HARRY WEESE &amp; ASSOCIATES</b> GENERAL ARCHITECTURAL CONSULTANT			
APPROVED	DATE	DATE	DATE	DATE	DATE	DATE	DATE	
							SCALE VERT. 0' 2' 4' 8' 12' DRAWING NO. <b>F-A-113</b>	

BORING NUMBER AM-36		Ground surface elevation +320.5	
0	26	10	5.8.7
5	55	20	10.15
10	29	30	16.18
15	30	40	28.32
20	27	50	17.39
25	183	60	17.30
30	288	70	100.2
35	1C	60/10	
40	2C	55/20	
45	3C	75/10	
50	4C	75/45	
55	5C	98/70	
60	6C	95/85	
65	7C	98/50	
70	8C	98/50	
75	9C	98/40	
80	10C	95/55	
85	11C	95/55	
90	12C	100/80	
95	13C	85/75	
100	14C	85/80	
105	15C	90/80	
110	16C	100/85	
115	17C	98/50	

BORING NUMBER AM-36 (CONTINUED)		Ground surface elevation +311.6	
110	18C	95/20	
115	19C	90/45	
120	20C	98/95	
125	21C	95/90	
130	22C	90/45	
135	23C	98/65	
140	24C	90/50	
145	25C	98/50	
150	26C	98/85	
155	27C	95/90	
160	28C	95/85	
165	29C	90/70	
170	30C	20/10	
175	31C	85/30	
180	32C	98/15	
185	33C	95/40	
190	34C	98/60	
195	35C	98/70	
200	36C	100/100	
	37C	98/90	
	38C	98/80	

BORING NUMBER AM-37U		Ground surface elevation +311.6	
105	13	1D	5.6.7
	43	2U	
	49	3P	
	51	3D	
	52	9	10
	202	30	9.15
	250	40	47.40
	247	40	70
	267		
	271		
	301		
	497		
	1200		
15	5D	100/6'	
20	1C	50/10	
25	2C	90/60	
30	3C	95/85	
35	4C	85/40	
40	5C	85/80	
45	6C	98/98	
50	7C	98/95	
55	8C	98/97	
60	9C	96/90	
65	10C	90/80	
70	11C	90/50	
75	12C	90/15	
80	13C	90/85	
85	14C	95/40	
90	15C	95/60	
95	16C	95/75	
100	17C	90/0	
105	18C	85/40	
	19C	98/35	
	20C	98/30	

BORING NUMBER AM-37U (CONTINUED)		Ground surface elevation +313.6	
110	21C	98/25	
115	22C	70/35	
120	23C	98/65	
125	24C	98/70	
130	25C	98/65	
135	26C	98/85	
140	27C	90/90	
145	28C	96/90	
150	29C	92/90	
155	30C	98/80	
160	31C	96/40	
165	32C	95/35	
170	33C	95/55	
175	34C	90/20	
180	35C	98/55	
185	36C	98/70	
190	37C	98/75	
	38C	80/75	
	39C	98/25	
	40C	98/15	

BORING NUMBER AM-38		Ground surface elevation +313.6	
0	1D	4.7	
5	2D	35.42	
10	3D	27.40	
15	1C	80/0	
20	2C	65/0	
25	3C	55/0	
30	4C	70/15	
35	5C	75/50	
40	6C	85/45	
45	7C	95/50	
50	8C	80/20	
55	9C	90/30	
60	10C	95/40	
65	11C	95/30	
70	12C	85/10	
75	13C	95/50	
80	14C	85/55	
85	15C	95/60	
90	16C	98/80	
95	17C	98/90	
100	18C	98/85	
	19C	98/95	
	20C	98/90	
	21C	85/85	

BORING NUMBER AM-38 (CONTINUED)		Ground surface elevation +313.6	
105	22C	98/95	
110	23C	98/95	
115	24C	98/95	
120	25C	90/90	
125	26C	98/95	
130	27C	98/80	
135	28C	98/85	
140	29C	95/80	
145	30C	95/95	
150	31C	98/97	
155	32C	98/80	
160	33C	96/90	
165	34C	98/90	
170	35C	90/90	
175	36C	95/90	
180	37C	98/75	
185	38C	98/96	
190	39C	98/96	
	40C	90/90	

Boring started 10-22-73, completed 10-31-73  
 Final depths: Boring = 200.0' Casing = 30'  
 Casing diameter = 4"  
 Average depth of ground water observed in the boring = 20.2' = El. +300.3

REMARKS:  
 Drilled ahead of casing from 10' to 30' depth; casing blows not indicative of soil consistency. Observation well consisting of 1 1/2" steel pipe installed with tip at 180' depth. Four water pressure tests with packers run in this borehole.

Boring started 10-30-73, completed 11-13-73  
 Final depths: Boring = 190.0' Casing = 19'  
 Casing diameter = 3-1/2" & 4"  
 Average depth of ground water observed in the boring = 22.5' = El. +289.1

REMARKS:  
 Drilled ahead of casing from 14' to 19' depth; casing blows not reported. Mechanical breakdown of drilling rig caused breakage of the rock sample in core No. 3C.

Boring started 10-16-73, completed 10-24-73  
 Final depths: Boring = 189.5' Casing = 25'  
 Casing diameter = 3-1/2" & 4"  
 Average depth of ground water observed in the boring = 25.0' = El. +288.6

REMARKS:  
 Drilled ahead of casing to 25' depth; casing blows not reported. Four water pressure tests with packers run in this borehole.

DESIGNED IHL DATE 12/74	REFERENCE DRAWINGS F-1 General Notes & Legend F-A-86 to Plans & Geological Sections F-A-104	REVISIONS	WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY		ROCKVILLE ROUTE (A011)	
CHECKED SLT & JPC DATE 12/74			WEESE - RUTLEDGE - WENTWORTH & JOHNSTON		LOGS OF AM-SERIES BORINGS Nos. AM-36 to AM-38	
APPROVED			CONSULTING ENGINEERS 415 MADISON AVE. NEW YORK 17, N.Y.		SCALE VERT. 0" = 5' = 10'	
			DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANT		DRAWING NO. F-A-114	
			HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT			

BORING NUMBER AM-40		Ground surface elevation +290.4	
1D	3,5,7	Stiff brown sandy & clayey silt with some organic material, topsoil, roots (Fill) (ML)	DECOMPOSED GNEISS, SILTY SAND AND TOPSOIL 9.0'
2D	38,57,79	Very compact brown silty fine sand (Decomposed Schistose Gneiss) (SM)	
1C	85/10	Brown & green-brown, wthrd & jts coarse grained schistose gneiss, iron stained, 45° fol, 8 fol jts, 10 diag jts	GRY-GREEN WEATHERED AND JOINTED SCHISTOSE GNEISS AND HORN-BLENDE SCHISTOSE GNEISS WITH IRON STAINING AT JOINTS SOME CHLORITE VEINS AND TALC EVIDENT, MULTIPLE JOINTING 37.0'
2C	90/20	Do 1C, wthrd & jtd, 10 diag jts, 5 fol jts, iron staining & talc on joints	
3C	70/0	Do 1C, wthrd & crushed gneiss 19'-20.5', vert jointing 20.5'-21', iron staining, slightly decomposed 23'-23.5', pos shear	GRY-GREEN UNWTHRD REL SND MOD JOINTED HORN-BLENDE MICHA SCHISTOSE GNEISS W/OCC BANDS OF CHLORITE AND QUARTZ, PROBABLE SHEAR ZONE 59' TO 60'
4C	95/25	Do 1C slightly wthrd, mod jtd, sheared 23.5'-24', multiple jts 24'-26.5', rel snd unwthrd mtl 26.5' to 28', 60° fol	
5C	98/30	Unwthrd jtd hornblende schistose gneiss, 8 fol jts, 11 diag jts, some chlorite veins 29'-30'	
6C	98/30	Do 5C, mod jtd, mod wthrd, 10 diag jts, 3 fol jts, slight weathering 36'-36.5'	
7C	70/60	Do 5C, rel snd, mod jtd unwthrd, 5 diag jts, 2 fol jts, fol change 39.5' to 40.5', w/evident chlorite & quartz intrusion	GRY-GREEN UNWTHRD REL SND MOD JOINTED HORN-BLENDE MICHA SCHISTOSE GNEISS W/OCC BANDS OF CHLORITE AND QUARTZ, PROBABLE SHEAR ZONE 59' TO 60'
8C	95/70	Unwthrd, rel snd, mod jtd 5 diag jts, 1 fol jt & 1 vert jt 45'-45.5'	
9C	70/60	Unwthrd, rel snd, mod jtd 2 fol jts, 2 diag jts, slight staining & talc on joints	
10C	95/90	Rel snd, unwthrd, slightly jointed, 3 fol jts, 2 diag jts, possible shear 53.0'	
11C	96/80	Vein chlorite schist 54' to 54.7', 4 fol jts, 4 transverse jts, rel snd, mod jtd	CHLORITE SCHIST & PHYLITE W/OCC BANDS SCHISTOSE GNEISS, SHEARED 63.5' TO 65.5'
12C	95/50	Gneiss changes @ 61' to chlorite mica schist, shear & quartz vein 59' to 59.5', change in fol, friable & flaky, thin quartz veins in schist	
13C	98/30	Chlorite mica schist, jtd, fract & sheared, wthrdng & decomp 65'-65.5', band of gry sch gns 65.5'-67' changes back to chl sch	REL SND SCHISTOSE GNEISS 74.0'
14C	98/60	Finely laminated phyllite unwthrd, rel snd, mod jtd occ bands schistose gneiss, 71.5'-72' 5 diag jts	
15C	98/80	Phyllite changes to chlorite schist 72.5-74', Gray rel snd, mod jtd sch gns 74'down w/occ chlorite	REL SND SCHISTOSE GNEISS 81.0'
16C	80/65	Schistose gneiss w/band chlorite & mica schist, steep fol, 8 fol jts, 2 diag jts, slickensides	GREEN CHLORITE SCHIST, PHYLITE & MICHA SCHIST 88.0'
17C	85/55	Grn chlorite schist w/sm blk mica schist band & phyllite, unwthrd, mod jtd possible shear 81'-82'	
18C	70/65	Do 17C, sm shear 86', phyllite, rel snd, unwthrd mod jtd, changes to schistose gneiss @ 88'	GRAY REL SND DIORITIC AND SCHISTOSE GNEISS W/OCC BANDS & LENSES CHLORITE SCHIST
19C	98/90	Rel snd, unwthrd, slt jtd dioritic & schistose gns	
20C	98/98	Do 19C, band chlorite schist 95'-95.5', phyllite 96'-97'	
21C	95/95	Rel snd, mod jtd, unwthrd schistose gneiss, band of chl schist 99.7'-100' & 102.2'-102.5', 2 fol jts	

(CONTINUED)

BORING NUMBER AM-40 (CONTINUED)		Ground surface elevation +274.0	
22C	98/80	Rel snd, unwthrd, slt jtd schistose gneiss, chlorite schist band 102.2'-103.4' & 106.6'-106.8', 1 fol jt, 2 diag jts, sm slip & cleavage in chlorite schist	REL SND GRY-GREEN HORN-BLENDE SCHISTOSE GNEISS, MOD JTD, THIN QUARTZ VEINS 108.0'
23C	80/70	Rel snd, unwthrd, slt jtd, change @ 108' to hornblende gneiss, fol 60', 2 diag jts, 1 vert jt, 1 fol jt	
24C	98/75	Var fol, unwthrd, rel snd, mod jtd, 6 diag jts, 3 fol jts, thin quartz veins	
25C	95/70	Gray diorite gneiss 116' to 119', gry-grn horn schis gns w/qtz veins 119'down, rel snd, unwthrd, mod jtd, 9 diag jts, 2 fol jts	
26C	98/90	Gry-grn horn sch gneiss w/sm qtz veins, rel snd, unwthrd sl jtd, 7 diag jts var fol jts	
27C	98/75	Rel snd, mod jtd, var fol, chlorite schist 125'-127' & 128'-129', 3 fol jts, 7 diag jts	
28C	96/80	Grn chlorite schist, rel snd, unwthrd, mod jtd, 5 fol jts, 3 diag jts fol 50°	RELATIVELY SOUND, MODERATELY JOINTED CHLORITE SCHIST 125.0'
29C	90/85	Rel snd, unwthrd, mod jtd, 5 fol jts, 4 diag jts, quartz bands 137'-138'	
30C	98/90	Unwthrd, rel snd, mod jtd 4 fol jts, 3 diag jts fol 50°	
31C	98/85	Rel snd, mod jtd, unwthrd change 143.2', triple set jts 144'-144.5', 4 fol jts 3 diag jts, coarse grained schistose gneiss 145'-147' fol 50°	REL SND MOD JTD GRY DIORITIC GNEISS W/SOME QUARTZ VEINS AND CHLORITE BANDS 145.0'
32C	70/60	Rel snd, mod jtd, unwthrd, 2 fol jts, 1 diag jt, band chlorite schist 150.5' to 151.3'	
33C	90/35	Rel snd, h1 jtd, unwthrd, 14 diag jts, 6 fol jts, chlorite & mica schist inclusions 153.5'-154', multiple jt pattern	
34C	95/90	Rel snd, mod jtd, 2 fol jts, 4 diag jts, fine grained chlorite schist 159', 2 vert jts 158' to 158.5' & 158.5'-159'	REL SND MOD JTD GREEN CHLORITE SCHIST 159.0'
35C	95/80	Grn rel snd chlorite schist, mod jtd, some fracturing 159.8'-160.2' & 160.7'-161', 3 fol jts, 8 diag jts	
36C	95/85	Do 35C, rel snd, mod jtd, 3 fol jts, 5 diag jts	

Boring started 10-12-73, completed 10-19-73  
 Final depths: Boring = 168.5' Casing = 20'  
 Casing diameter = 3-1/2" & 4"  
 Average depth of ground water observed in the boring = 15.2' = El. +275.2

REMARKS:  
 Drilled ahead of casing from 9' to 20' depth; casing blows not reported.  
 Four water pressure tests with packers run in this borehole.

BORING NUMBER AM-41U		Ground surface elevation +274.0	
1D	2,3,5	2" Topsoil, loose brown clayey silt w/roots (Fill) (ML)	FILL 4.0'
2U		Silty sand (Possible Fill) (SM)	
3U		Brown silty f-m sand, tr gravel (SM)	
4D	6,12	Loose brown fine to coarse micaceous sand, some silt, some gravel (SM)	MICACEOUS SILTY SAND, TRACE GRAVEL, DECOMPOSED ROCK 25.3'
5D	12,8	Very compact brown-gray micaceous silty medium to fine sand, some rock fragments (Decomposed Rock) (SM)	
7D	88, 144, 150.2	Do 8D (SM)	
8D	160.5	Brn h1 wthrd & jtd, jts 20'-75', hornblende gneiss with iron stains	HIGHLY WTHRD & JOINTED HORN-BLENDE GNEISS 31.4'
9D	150.4	Top: Do 1C	
1C	65/10	Bot: Gry-grn unwthrd & mod jtd hornblende gneiss, broken & decomposed horz zone @ 33.3' to 33.5'	
2C	60/5	Gry-grn unwthrd, jtd & mod broken hornblende gneiss	
3C	95/25	Do 3C, unwthrd & mod jtd, broken horz zone with some slickensides at 41.0' to 41.6'	
4C	90/45	Do 4C, with 2 small broken horz zones of 1" to 2" at 44.1' and 48'	
5C	95/55	Do 4C, with small broken horz zone of 1" at 50.2'	
6C	100/35	Do 4C	UNWEATHERED AND JOINTED TO MODERATELY JOINTED HORN-BLENDE GNEISS, TALC ON JOINTS THROUGHOUT WITH NUMEROUS CHLORITE PATCHES AND LENSES
7C	98/40	Unwthrd, mod jtd hornblende gneiss	
8C	100/70	Do 8C with 45° to 65° jts	
9C	98/40	Do 8C, with horz to 25° jt; broken zone between 69' & 71.5'	
10C	90/35	Do 8C, with a broken zone, small pieces, at 71.8'-72'	
11C	95/50	Do 8C, with horizontal joints	
12C	95/35	Do 8C, with a broken & weathered zone between 81.6' to 82'	
13C	85/45	Do 8C, unweathered & jointed	
14C	95/60	Do 8C, unweathered & jointed	
15C	95/55	Well jtd hornblende gneiss secondary minerals in jts, 2 horz jts, 13 jts @ 40°-60°	
16C	95/40	Mod jointed to jointed 6 joints at 40°-60°	
17C	90/60		

(CONTINUED)

BORING NUMBER AM-41U (CONTINUED)		Ground surface elevation +292.0	
18C	95/30	Stiff brown fine sandy silt w/gravel fine sand in bot, tr gravel (Fill) (SM)	GRAY FINE SAND FILL 4.5'
19C	90/45	Compact light gray-brown silty fine sand, some mica trace hornblende fragments residual soil (SM)	LIGHT GRY-BRN SILTY FINE SAND AND ROCK FRAGMENTS 15.0'
20C	90/50	Mod jtd to jtd, 3 horz jts, 2 80° jts, 2 45° jts, soft green chlorite patches 114'-115'	
21C	95/10	Well jointed, badly fractured, soft patches at 115'-116' & 118'-119'	
22C	85/40	Do 18C	
23C	95/35	Do 18C, 12 jts at 40°-60°, 6° patch of well fractured chlorite & hornblende at 124', badly fractured 3" patch at 127'	
24C	95/40	Do 18C, 6 jts at 40°-60°, 5 jts at 20°-40°	
25C	85/40	Do 18C, 7 jts at 40°-60°, 4 horz jts, 2° patch of chloritic mineral @ 137.6'	
26C	95/25	Chloritic hornblende gneiss, 10 jts at 40°-60°, 5 jts at 20°-40°	
27C	95/55	Do 26C, 2 jts at 70°-80°, 6 jts at 40°-60°, mod jtd to jtd	
28C	90/60	Do 26C, 2 jts at 50°-60°, 6 jts at 40°-50°, mod jtd to jtd	
29C	95/30	Hornblende gneiss, change at 154.9' to pink granite gneiss, 15 jts, badly fractured chloritic zone at 154.5'-154.9'	154.9'
30C	95/80	Pinkish feldspar granite gneiss 1 horz jt, 4 jts at 40°-60°, soft chlorite seam at 158.4'	GRANITIC MATERIAL 160.0'

Boring started 3-7-74, completed 3-19-74  
 Final depths: Boring = 160.0' Casing = 14.5'  
 Casing diameter = 3-1/2"  
 Average depth of ground water observed in the boring = 9.5' = El. +264.5

REMARKS:  
 Observation well consisting of 1-1/2" steel pipe installed with tip at 150' depth.  
 Four water pressure tests with packers run in this borehole.

BORING NUMBER AM-42		Ground surface elevation +292.0	
1D	3,4	Stiff brown fine sandy silt w/gravel fine sand in bot, tr gravel (Fill) (SM)	GRAY FINE SAND FILL 4.5'
2D	13,16,22	Compact light gray-brown silty fine sand, some mica trace hornblende fragments residual soil (SM)	LIGHT GRY-BRN SILTY FINE SAND AND ROCK FRAGMENTS 15.0'
3D	150/6'	Light gray-brown silty fine sand & rock fragments (decomposed Schistose gneiss) (SM)	
1C	70/0	H1 jts & wthrd brown quartz-diorite gneiss, several small soft pieces, weathers into f-m sand	
2C	40/0	1 piece 0.8", vertical jointing, highly weathered, many small pieces	HIGHLY JOINTED AND WEATHERED BROWN QUARTZ DIORITE GNEISS, 30' FOLIATION, IRON STAINING ON JOINTS 43.0'
3C	90/0	Jtd, wthrd, 30° fol, 7 fol jts, 50° diag jts 26'-28', dark stains on joints	
4C	90/15	Mod wthrd, 80° diag jt 30'-32', 9 fol jts, 45° fol, diag jt 33'-34', weathered in bottom	
5C	80/0	H1 jtd, 7 fol jts, 6 diag jts, some iron staining on foliation	
6C	95/10	Mod weathered, 15 fol jts, 7 diag jts w/iron stains, gneiss altered w/gray-brown slate 43'-45'	
7C	95/0	Highly jointed, moderately weathered slate & quartz diorite gneiss	HIGHLY WTHRD QUARTZ DIORITE GNEISS 50.0'
8C	90/35	Quartz vein 7" in top jtd, mod to wthrd w/ slate bands 54.5' to 55.0'	50.5'
9C	95/30	12 fol jts, 6 diag jts, slightly weathered on joints w/iron staining	JOINTED MOD WTHRD GRAY QUARTZ DIORITE GNEISS WITH SLATE BANDS 65.0'
10C	80/10	Highly jointed & weathered 6 fol jts & 2 diag jts in top 2 ft., 4 jts 64'-65'	
11C	95/50	Jtd unwthrd gray quartz diorite gneiss, 7 fol jts, 4 diag jts	JOINTED UNWEATHERED GRAY QUARTZ DIORITE GNEISS 45° FOLIATION 80.0'
12C	90/55	Finely laminated, 11 fol jts, 5 diag jts	
13C	80/50	Jtd unwthrd, 7 fol jts, 2 diag jts, 45° fol	
14C	95/85	Rel sound to sound light gray diorite gneiss, thinly laminated, thin calcite & filling, 3 fol jts, 1 diag jt	RELATIVELY SOUND GRAY QUARTZ DIORITE GNEISS, THIN CALCITE LAMINATIONS 97.0'
15C	95/90	Sound, 3 pieces 2.7', 8.5', 3.5', 2 jts	
16C	95/90	Finely laminated, thin calcite fillings along 45° fol, 1 diag jt 92.4' to 93', thin calcite	
17C	95/85	Med to lt gry, h1 feldspar content, 1 diag jt @ 97', 4 diag jts, 5 horz jts	
18C	95/60	Cave-in at 97' twisted off drill rods in attempting to recover barrel, jointed gneiss, possible shear zone	

(CONTINUED)

BORING NUMBER AM-42 (CONTINUED)		Ground surface elevation +292.0	
19C	98/10	Dark green highly jointed schist layers; 2 diag jts, 5 fol jts, 45° fol	JOINTED TO HIGHLY JOINTED GRAY TO DARK GRAY QUARTZ DIORITE GNEISS 112.5'
20C	95/10	Highly jointed, 2 diag jts, 6 fol jts, talc on joints	
21C	90/75	Mod jtd light to dark gray-green, 3 diag jts w/thin talc	
22C	98/90	Relatively sound, 3 diag jts, 2 fol jts	REL SND QUARTZ DIOR GNS 116.5'

Boring started 9-28-73, completed 10-25-73  
 Final depths: Boring = 116.5' Casing = 40'  
 Average depth of ground water observed in the boring = 18.0' = El. +311.0

REMARKS:  
 Casing was pushed from 15' to 40' depth; casing blows not reported.  
 During time interval between 10-17-73 and 10-25-73 the boring hole caved in at 97' depth, while boring was at 116.5' depth. Boring terminated at this level.  
 Rock coring was performed by 4" x 5-1/2" diamond drill bit yielding core of approximately 4" in diameter.

<b>REFERENCE DRAWINGS</b> NUMBER DESCRIPTION F-1 General Notes & Legend F-A-86 to Plans & Geological Sections F-A-104		<b>REVISIONS</b> DATE BY DESCRIPTION			<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> NUMBER - RUTLEDGE - WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y. SUBMITTED <i>William H. Weese</i>		DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT		<b>ROCKVILLE ROUTE (A011)</b> <b>LOGS OF AM-SERIES BORINGS Nos. AM-40 to AM-42</b> SCALE: VERT. 0" = 10' DRAWING NO. <b>F-A-115</b>	
DATE 12/74 DATE 12/74 DATE	DATE DATE	DATE DATE	DATE		APPROVED	APPROVED				

BORING NUMBER AM-43U		Ground surface elevation +250.7	
1D	4,4,6	Medium compact brown silty fine sand, trace fine mica, some veg mtl, (Fill) (SM)	MEDIUM COMPACT BROWN SILTY FINE SAND, TRACE MICA
2D	4,6,8	Do 1D, with some brown clay pockets, residual soil (SM)	
3D	11,12	Medium compact red-gray silty fine sand, trace clay lenses (SM)	RED-GRAY SILTY FINE SAND
AUD.	P	Brown medium to fine sand & rock fragments, some silt (SM)	
1C	60/0	Jtd, mod wthrd pink-light gray grano-diorite, vert jt 15' to 17', jtd & wthrd in bot w/ thin red clay seams	JTD MOD WTHRD GRANO-DIORITE
Core	NR	Soft weathered material	
25	Core	NR	Soft weathered material
30	5D	34,31	Very compact brown silty fine sand (SM)
2C	35/0	Jtd, wthrd brown grano-diorite, 5 fol jts at 45', 1 diag jt at 34', dark stain	HIGHLY JOINTED AND WEATHERED BROWN GRANO-DIORITE GNEISS
3C	50/0	Highly jointed, weathered in top - soft, 1 45° jt at 38.5', wthrd soft brown diorite gneiss	
40	Core	NR	
45	4C	85/35	Highly jtd & wthrd dark green-brown chlorite & diorite gneiss, 1 solid piece 46.5'-48'
50	5C	98/70	Mod jtd gray-green chlorite diorite with feldspar bands at 49.5' & 51', 4 15° jts, 1 45° jt at 49.8', 60° diag jts 51'-52'
55	6C	80/70	4 fol jts, 1 horiz jt, 1 45° jt @ 55.1', vert jt @ 56.5', thin feldspar bands
60	7C	85/80	Mod jtd lt gray grano-diorite, 1 45° jt @ 57.8', 2 fol jts, 70° contact at 61'
65	8C	98/90	Sound lt gray grano-diorite w/dark diorite gneiss to 64.5'
70	9C	98/95	Sound gray-green diorite gneiss
75	11C	98/95	Sound, 1 vert jt 71.5' to 72.4' with talc, 2 fol jts
80	12C	98/98	Sound, 5 20° jts, 2 diag jts 73.5' - 73.6' w/talc, 1° chlorite inclusion 76', 1-1/2" quartz vein at 76.5'
85	13C	98/95	Sound lt gray quartz diorite gneiss, 2 20° jts w/thin talc, old 1/8" vert jt 79.5'-83'
90	14C	98/65	3 15°-20° jts, old vert jts 1/8"/1/4" to 87.3', chlorite inclusions & bands at 84' & 87'
95	15C	98/95	Mod jtd, 4 diag jts
100	16C	98/85	Rel sound, 2 20° jts, 1 30° jt, talc plus some gouge material on jts at 94', 1-1/2" quartz vein at 95'
			1 horiz jt at 79', 1 45° jt at 79', 4 10° jts, 4 horiz jts 100.0'-101.3'

BORING NUMBER AM-43U (CONTINUED)		Ground surface elevation +250.7	
105	17C	98/95	Sound, 3 pieces, old filled diag jts at 70', 1° feldspar band 103.4' at 45'
110	18C	98/70	Mod jtd, 1 80° jt 107.5' to 109', 2 45° jts at 107.8', 109.1' chlorite intrusions
115	19C	90/65	Light gray, 6 20°-30° jts, 1 50° jt at 112.3', 1 45° jt at 113', 1 70° jt at 113.5'-114' with thin talc deposit on joints
120	20C	98/65	Green & light gray, 3 fol jts 117.1'-117.5' with talc deposit, 2 diag jts 119', 4 20°-30° jts
125	21C	98/95	Gray-green, sound, 3 fol jts, 2° feldspar quartz band 120.8'-121.0', thin feldspar inclusion 121.5' to 122.9'
	22C	90/65	Light gray quartz diorite gneiss to 127.7', 10 15° to 30° jts, 3 45° jts, chlorite band 127.7' to 128.7'

BORING NUMBER AM-44U		Ground surface elevation +285.7	
10	1D	2,2,4	Red-brown micaceous clay silt, trace roots (Fill) (ML)
20	2U	P	Red-brn micaceous silt, residual soil (ML)
30	3D	3,4,6	Brown micaceous fine sandy silt (ML)
40	4U	P	Do 2U, trace fine sand (ML)
50	5D	4,8, 11	Brown micaceous fine sandy silt (ML)
60	6U	P	Red-brown fine sandy silt, micaceous, trace gravel (ML)
70	7D	8,9, 13	Do 6U (ML)
80	8U	10,12 16	Red-brown micaceous fine sandy silt (ML)
90	9D	20,29 24	Brown micaceous silty fine sand, trace rock fragments (SM)
100	10D	9,13, 15	Gray micaceous silty fine sand (SM)
110	11D	17,35 36	Do 10D (SM)
120	12D	21,37 42	Do 10D (SM)
130	13D	150/5	Do 10D, w/rock fragments, transition to weathered rock (SM)
140	14D	100/0	Probable soft wthrd rock Dark gray micaceous silty fine sand (SM)
150	15D	75/0	Mod wthrd & hi stained jointed coarse grained diorite gneiss
160	16D	95/65	Jtd, unwthrd, 2 60° jts, 5 20° jts, 2 horiz jts, 1 slightly stained, 6 jts/4', chlorite mixed with diorite at 57.5'-58'
170	17D	95/85	Mixed w/chlorite schist, 45° fol, 3 40° jts, 2 20° jts, 5 jts/4.8', jts unstained & unwthrd
180	18D	95/80	Rel sound, fol 50° 3 45° jts, 1 60° jt, 1 horiz jt quartz vein 63' 2'-63.4', widely spaced, 3 jts/4'
190	19D	95/80	Greenish-gray chlorite mixed w/diorite gneiss, rel sound, 60° fol, 2 30° jts, wthrd soft zone 68.3'-69.3'
200	20D	100/ 100	Do 5C, sound
210	21D	100/ 100	Do 5C, 1 20° jt, 1 jt/4.7' poorly foliated
220	22D	100/ 95	Do 5C, 1 70° jt, wide spacing, jt/4.8', slightly foliated at 45°
230	23D	95/65	Do 5C, mod jtd, 2 60° jts, 1 70° jt, 1 30° jt, 1 60° jt, healed jts, 4 jts/4.8'
240	24D	95/85	1 50° jt, 3 20° jt, widely spaced, 1 jt/3.5', quartz feldspar vein at 91.3' to 91.7'
250	25D	95/75	Rel sound, 5 15°-20° jts, 1 50° jt, 4 jts/4', jts unwthrd & unstained
260	26D	95/90	2 45° jts, 2 jts/5', slight calcite deposit on joint planes

BORING NUMBER AM-44U (CONTINUED)		Ground surface elevation +285.7	
105	13C	95/95	Sound, 1 30° jt, 1 jt/3'
110	14C	95/95	2 30° jts, slight talc along jts, 2 jts/3'
115	15C	95/90	1 60° jt, 1 30° jt, slight calcite deposit, joints widely spaced
120	16C	95/90	Sound, quartz layer 113.5' to 114.8'
125	17C	85/60	Quartz, 2 60° jts, 1 70° jt, 3 20° jts, broken zone 118.5'-119.5', highly jtd 118'-119.6', calcite on jts
130	18C	90/60	119.6'-120.4' jtd quartz, 120.4'-124.4' diorite gneiss w/chlorite, 2 60° jt jts, 1 20° jt, vertical fracture 119.5'-120'
135	19C	95/65	Jtd gneiss, 1 60° jt, 2 30° jts
140	20C	95/70	Jointed quartz at 130' to 133.5', 3 60° jts, 2 45° jts, 2 30° jts
145	21C	95/85	Rel sound gneiss, 1 80° jt, 1 horiz jt.
150	22C	95/85	Gray diorite gneiss, rel sound w/quartz intrusion, 2 30° jts, 1 45° jt at 140.5' open 1/4", possible movement at 140.5'
155	23C	95/75	3 45° jts, 2 15° jts, 1 70° jt, slight calcite deposit on jts
160	24C	95/80	3 30° jts, 1 60° jt, 1 10° jt, slight calcite deposit
	25C	95/75	2 60° jts, 1 30° jt, 1 20° jt, mod jtd, rel sound, healed jts 152' 152.5', jtd 151.5'-153'
	26C	95/90	2 60° jts, 1 20° jt, 3 jts/3.5', quartz intrusion, chlorite layers
	27C	75/70	1 30° jt, 1 70° jt, 1 0° jt

BORING NUMBER AM-45U		Ground surface elevation +237.5	
10	1D	10,12	Medium compact brown silty fine to medium sand & angular gravel (Fill) (SM)
20	2D	39,51 100	Very compact brown & light gray silty fine to coarse sand, medium gravel, trace cinders (Fill) (SM)
30	3D	16,20 131	Compact to very compact brown silty fine to medium sand & fine gravel (Fill) (SM)
40	4U	P	Brown fine sandy silt & angular rock fragments (ML)
50	5D	18,37 102	Very compact brown silty fine sand, trace clay, few rock fragments (Decomposed & Weathered Rock) (SM)
60	6C	80/0	Jointed, broken & weathered brown gneiss, moderately weathered from 27'
70	7C	95/0	Moderately weathered, jointed brown-gray schistose gneiss, 1/8" vertical quartz veins
80	8C	95/0	Highly jointed gray schistose gneiss 80° to vert/jtg through-out, iron stains and wthrd on jts
90	9C	95/0	Vert jtg & wthrd, 6 45° to 60° jts, gouge material possible shear, 1/2" vert quartz vein at top
100	10C	80/0	Jtd & wthrd gray-brown finely laminated 80° to vert jtg, iron stains on joints
110	11C	95/10	Jtd gray, wthrd to mod wthrd, 3 80° jts, 3 30° jts, 2 45° jts, 80° to vert fol.
120	12C	60/50	Jtd to mod jtd, finely laminated, irregular vert fol, iron stain & talc on 1/8" open curved 80° jt
130	13C	95/95	Rel sound, 1 10° jt at 57.6', vert fol, 1/16" to 1/8" quartz vein, irregular 1/16" open, 3 45° jts
140	14C	95/80	2 30° jts, vert fol, thin laminated & folded quartz veins 1/32"-1/4", 60.3' to 61.5'
150	15C	95/40	1 45° jt, 3 50° jts, 1 20° rough, iron stain, 1 60° jt, 1 80° jt, 3 vert jts to 80° in 2'
160	16C	80/60	5 30°-45° jts, rough open 1/8" to 1/4", 2 80° jts slight stain, 80° fol, thin quartz veins
170	17C	95/40	4 60° fol jts in-2', 2 30° jts, 2 10° jts with quartz veins, 70° f, possible shear & gouge
180	18C	80/25	Hi jtd 81.2'-84.3', hi quartz content 81'-82.5' 80° to vert fol, gouge mtl, talc deposits & iron stains on jts
190	19C	95/60	Mod jtd lt gray quartz diorite gneiss, 2 20° rough jts, 1 10° jt, iron stain on jts, hi jtd in bot
200	20C	95/80	Rel sound lt gray, 1 60° jt at 91', slightly rough, dk brown iron stain, gouge, 3 30° jts thin calcite deposit, 1 50° jt, talc deposit at 93'

BORING NUMBER AM-46U		Ground surface elevation +264.9	
10	1D	6,11, 14	Red-brown fine sandy silt, trace clay (Fill) (SM)
20	2D	P	Brown silty clay (Fill) (CL)
30	3D	14,22 31	Compact brown-gray silty fine to medium sand, trace rock fragments, residual soil (SM)
40	4D	41, 119, 68	Brown & light gray silty fine to medium sand, trace clay pockets (SM)
50	5D	19,23 43	Do 4D, some rock fragments (SM)
60	6D	12,22 25	Compact red-brown silty fine to medium sand (SM)
70	7D	30,40 22	Brown fine to coarse sand, rock fragments (GP)
80	8D	25,52 49	Gray slightly micaceous silty fine to medium sand, some rock fragments (SM)
90	9D	52, 100, 153	Do 8D (SM)
100	10D	66, 150/11 45/0	Do 8D (SM)
110	11D	2C	40/0
120	12D	3C	85/0
130	13D	4C	90/0
140	14D	5C	90/20
150	15D	6C	90/55
160	16D	7C	90/60
170	17D	8C	95/60
180	18D	9C	95/70
190	19D	10C	95/85
200	20D	11C	95/80
210	21D	12C	95/75
220	22D	13C	95/75
230	23D	14C	90/85

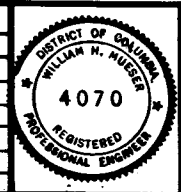
(CONTINUED)

(CONTINUED)

(CONTINUED)

DESIGNED	DATE	NUMBER	DESCRIPTION
RAWN	12/74	F-1	General Notes & Legend
CHECKED	12/74	F-A-96 to F-A-104	Plans & Geological Sections
APPROVED			

REFERENCE DRAWINGS		REVISIONS	
NUMBER	DESCRIPTION	DATE	BY



**WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY**

**WEESER - RUTLEDGE - WENTWORTH & JOHNSTON**  
CONSULTING ENGINEERS  
415 MADISON AVE. NEW YORK 17, N. Y.

DE LEUW, CATHAR & COMPANY  
GENERAL ENGINEERING CONSULTANT

HARRY WEESE & ASSOCIATES  
GENERAL ARCHITECTURAL CONSULTANT

APPROVED *William H. Weese*

**ROCKVILLE ROUTE (A011)**

**LOGS OF AM-SERIES BORINGS Nos. AM-43U to AM-46U**

DRAWING NO. **F-A-116**

SCALE: VERT. 1" = 10'

**BORING NUMBER AM-47**  
Ground surface elevation +318.8

31	1D	5.13, 17	Brown silty fine sand, trace roots (F11) (SM)	BROWN SILTY FINE SAND, FILL	7.0'
40	2D	40.40, 44	Brown silty fine sand (Possible F11) (SM)		
45	3D	33, 100	Gray silty fine sand (SM)		
50	4D	50, 130	Do 3D (SM)	VERY COMPACT GRAY SILTY FINE SAND, TRACE COARSE SAND, NUMEROUS ROCK FRAGMENTS	
55	5D	150/6'	Do 3D, trace coarse sand (SM)		
60	6D	27, 150/3'	Silty medium to fine sand, some rock fragments (SM)		
65	7D	150/6'	Do 6D (SM)		
70	8D	80/55	Jtd & wthrd to 32.5' brn-gry diorite, 1 vert jt, 1 60° jt, 1 45° jt, 2 30° jts	JOINTED AND WTHRD BRN-GRAY QUARTZ DIORITE	38.0'
75	9D	95/50	Do 1C, 1 70° jt, 3 30° jts 1/16" open, 4 20° jts, iron stained	JOINTED GRAY QUARTZ DIORITE, IRON STAINS ON JOINTS	48.0'
80	10D	95/50	1 80° jt, heavy iron stain, pyrite crystals & calcite deposit, 1 30° jt 1/8" open, jtd, broken		
85	11D	98/55	5 20° jts, 1 30° jt in 2.2', 1/16" to 1/8" open, iron stained & talc on jts, 1 45° jt		
90	12D	95/85	Rel snd, 2 45° jts in 0.8' 1/16" open, 2 70° jts in 1/2"	RELATIVELY SOUND TO SOUND LIGHT GRAY QUARTZ DIORITE, NO FOLIATION, JOINTS SMOOTH TO SLIGHTLY ROUGH	76.0'
95	13D	95/85	2 45° jts, 1 30° jt 1/8" open, iron stain in 0.3' 2 60° jts, 1 45° jt in 2.1'		
100	14D	95/95	1 80° jt @ 60.5'-61', slickenside across jt at 25'. 1 30° jt, thin calcite	JOINTED QUARTZ DIORITE	80.0'
105	15D	95/95	1 30° jt @ 65'		
110	16D	98/95	1 45° jt, 1 30° jt in 1.0', smooth jts	RELATIVELY SOUND LIGHT GRAY QUARTZ DIORITE, THIN CALCITE DEPOSITS SLICKENSIDE AT 61'	76.0'
115	17D	75/75	30° jt 1/16" open with calcite, 1 45° jt in 2.0', 2" chlorite band		
120	18D	98/85	1 80° jt @ 75', trace talc calcite on jt, 3 60° jts, 1 45° jt, 2 30° jts, 1 20° jt in 0.8', iron stained	JOINTED QUARTZ DIORITE	80.0'
125	19D	80/50	Jtd, 1 70° jt iron stained 1 50° cross jt, 1 50° 1/16" jt open in 0.8' broken 0.5'		
130	20D	98/85	1 60° jt, 1 45° jt in 0.3' 2 45° jts in 0.1' talc deposit, 1 30° jt, 1 60° jt 2 45° jts in 1.5'	RELATIVELY SOUND LIGHT GRAY QUARTZ DIORITE, THIN CALCITE AND TALC DEPOSITS ON JOINTS	97.8'
135	21D	98/95	1 60° jt, 2 10° jts in 2', 1 30° jt @ 88.4'		
140	22D	98/90	1 30° jt thin calcite deposit, 1 70° jt 1/8" open, gouge mtl, talc & calcite along jt	JTD TALC & CALCITE DEPOSITS	102.5'
145	23D	98/90	1 30° jt 1/8" open, 1 20° jt, 1 60° jt in 1.8', 1 20° jt & 1 vert jt in 0.3' @ 97.5'		
150	24D	98/20	Jtd, 1 vert 85° jt curved, 99'-101.5', thin talc & calcite, 5 20° cross jts in 1.5'		

(CONTINUED.)

**BORING NUMBER AM-47 (CONT'D)**

105	18C	98/95	Rel snd, 3 jts in 0.5', 1 30° jt, 1 20° jt in 2.0' thin calcite deposits	REL SND LIGHT GRAY QUARTZ DIORITE GNEISS, THIN CALCITE DEPOSIT ON JOINTS NO FOLIATION	118.6'
110	19C	95/90	1 10° jt, 1 45° jt with thin calcite deposit in 0.3', 1 70° jt		
115	20C	98/90	1 80° curved jt, 1 10° cross jt, 1 45° jt in 0.6' thin calcite deposit, trace talc	JOINTED DIORITE, PLANAR TO OPEN JOINTS	125.3'
120	21C	95/70	3 30° jt, 1 vert jt in 0.6', thin calcite & talc deposits, 1 60° jt		
125	22C	98/75	Mod jtd, 1 30° jt, 1 45° jt talc deposit & stained, 6 10°-20° jts 1/16" open, 1 50° jt (planar) in 4.0'		
130	23C	98/90	Rel snd, 2 45° jts, 1 60° jt, 3 20° jts in 0.8', 2 20° jts are 1/2" apart, gouge mtl	RELATIVELY SOUND LIGHT GRAY QUARTZ DIORITE, THIN CALCITE DEPOSITS, SEVERAL THIN GOUGE ZONES INDICATE POSSIBLE MINOR SHEAR	159.0'
135	24C	98/90	1 45° jt @ 130', 1 10° jt, 1 20° jt, 1 30° jt, 1 45° jt in 0.9', gouge mtl on jts, possible shear		
140	25C	98/90	Sound, 1 10° jt rough 1/8" open, 1 70° jt, 1 30° jt in 0.3' with thin gouge		
145	26C	95/90	2 10° jts in 1.3', 2 30° jts in 1/2", gouge mtl, 1 70° jt, 1 50° jt crossing @ 143'		
150	27C	95/90	1 10° jt, 1 20° jt, 1 horiz jt, 1 45° jt in 1.7'		
155	28C	75/75	1 30° jt, 1 20° jt @ 152.5' slight iron stain, rough 1/16" open		
155	29C	50/50	1 10° jt, 1 20° jt in 1.2' thin calcite deposit		
155	30C	95/90	1 horiz jt, 1 70° jt in 0.2', planar with calcite deposits		

Boring started 6-12-74, completed 6-19-74  
Final depths: Boring = 159.0', Casing = 40.5'  
Casing diameter = 4" & 3"  
Average depth of ground water observed in the boring = 21.0' = El. +297.8

REMARKS:  
Drilled ahead of casing from 13' to 40.5' depth; casing blows not recorded.

**BORING NUMBER AM-48**  
Ground surface elevation +345.6

35	1D	8.9, 10	Brown & dark brown fine sandy silt, trace gravel (F11) (ML)	BROWN SANDY SILT, FILL	7.0'
40	2D	15.11, 11	Stiff yellow-red silty clay, some fine sand, trace gravel (F11) (CL)	YELLOW-RED SILTY CLAY, TRACE MICA	15.0'
45	3D	11.12, 15	Soft micaceous silty clay (CL)		
50	4D	8.8, 10	Gray-brown micaceous silty fine sand (SM)	BROWN-GRAY SILTY AND CLAYEY FINE TO MEDIUM SAND, TRACE MICA	30.0'
55	5D	19.21, 30	Brown-gray silty fine sand (SM)		
60	6D	20.11, 13	Brown-gray silty fine to medium sand, some angular rock fragments (SM)	GRAY SILTY FINE SAND, SOME ANGULAR ROCK FRAGMENTS	50.0'
65	7D	40.28, 31	Brown-gray silty fine sand, some fine mica (SM)		
70	8D	48.41, 56	Gray micaceous silty fine sand, few angular rock fragments (SM)		
75	9D	40.52, 60	Gray silty fine sand, numerous rock fragments (SM)		
80	10D	43.61, 82	Do 9D (SM)		
85	11D	45/20	Jtd, mod wthrd gray-brn diorite gneiss, 2 60° jts, 1 vert jt, 60° fol	JOINTED MOD WTHRD GRAY-BROWN QUARTZ DIORITE GNEISS	60.0'
90	12D	75/25	Do 1C, broken in top, 7 30° jts, 2 50° jts, red & dark stains on jts, 2 60° jts, 1 70° jt, 2 45° jts		
95	13D	95/75	Rel snd gray, 5 30° jts, 2 60° jts, 1 45° jt, smooth planar jts	JOINTED TO RELATIVELY SOUND GRAY QUARTZ DIORITE GNEISS, IRON STAINS ON JOINTS, SMOOTH TO 1/8"	78.8'
100	14D	95/80	3 30° jts, 2 60° jts, 1 45° jt, smooth planar jts, some iron stain		
105	15D	95/85	1 70° jt, 3 30° jts, 1 45° jt, iron stains on jts, 3.5' spacing, 60° fol		
110	16D	90/60	1 70° jt, 3 30° jts, 4 20° jts, smooth to 1/8" open in 0.8' spacing, iron stain on jt, 80° fol to vert		

Boring started 7-23-74, completed 7-24-74  
Final depths: Boring = 78.8', Casing = 50'  
Casing diameter = 4" & 3"  
Average depth of ground water observed in the boring = 34.0' = El. +309.6

REMARKS:  
Drilled ahead of casing from 20' to 50' depth; casing blows not recorded.

**BORING NUMBER AM-58U**  
Ground surface elevation +328.0

14	1D	2.2, 2	Brown fine sandy silt, trace roots & gravel (F11) (ML)	LOOSE TO MEDIUM STIFF BROWN SILT AND CLAY, FILL	10.0'
15	2D	3.4, 6	Red-brown slightly micaceous silty clay, some fine sand (F11) (CL)		
20	3D	9.10, 13	Red-brown micaceous silty fine sand (SM)	MEDIUM COMPACT TO COMPACT BROWN MICACEOUS SILTY FINE SAND BECOMING VERY COMPACT WITH DEPTH, DECOMPOSED ROCK	37.0'
25	4D	P	Do 3D (SM)		
30	5D	P	Brown micaceous silty fine sand (SM)		
35	6D	14.13, 16	Do 3D (SM)		
40	7D	18.22, 25	Do 3D (SM)		
45	8D	18.22, 23	Do 3D (SM)		
50	9D	27.44, 92	Brown micaceous fine to medium sand, some silt, trace rock fragments (SM)		
55	10D	84, 100/3'	Do 9D, numerous rock fragments (SM)		
60	11D	15/0	Hi wthrd soft diorite gneiss		
65	12D	65/25	Do 1C, 42'-45.6', from 45.5' to 47', jointed unwthrd diorite gneiss	HIGHLY WEATHERED AND JOINTED BROWN OXIDIZED DIORITE GNEISS	60.0'
70	13D	65/0	Hi wthrd & jtd red-brn, jts wthrd & stained		
75	14D	90/10	Diorite & quartz layer @ 53.8'-54.2'		
80	15D	85/5	Do 3C		
85	16D	90/20	Do 3C, 60.8' to 63', mod wthrd, 63.8' to 65.8' hi jtd, practically unwthrd, jts hi stained, 12 jts	MOD WTHRD GNEISS	63.0'
90	17D	90/25	Hi jtd, silt wthrd, jts stained & oxidized, vert jt 66'-67', 3 60° jts, 2 20° jts, poorly fol at 60'	SLIGHTLY WEATHERED HIGHLY JOINTED DIORITE GNEISS OR META DIORITE, HEAVY OXIDATION, STAINING OF JOINTS	60.0'
95	18D	90/20	Silt wthrd, jts hi wthrd, 7 10° jts, vert jt at 71'-72', 1 60° jt		
100	19D	85/30	Do 8C, 75.8'-78', 78' to 80.8' jtd & unwthrd	INDISTINCT, APPROX 60'	86.0'
105	20D	95/20	Do 8C, 6 10° jts, 2 90° jts, 2 60° jts	REL SND UNWTHRD DIO GNS	90.4'
110	21D	95/80	Do 8C, 85.6'-86', 86'-90.4', rel snd unwthrd jts unstained, 3 jts per 4 feet	JOINTED GNEISS	95.0'
115	22D	95/40	Jtd unwthrd diorite gneiss, 6 80° jts, 2 15° jts, jts generally stained wthrd @ 94.2' to 94.8'		
120	23D	95/75	Do 12C, rel snd, 1 80° jt talc deposit, 1 cross 60° jt, 2 10° jts, 1 30° jt, spacing 4 jts per 4 feet		
125	24D	95/80	Rel snd, 6 jts per 5 feet widely spaced		

Boring started 5-24-74, completed 6-3-74  
Final depths: Boring = 140.0', Casing = 45'  
Casing diameter = 4" & 3"  
Average depth of ground water observed in the boring = 21.6' = El. +306.4

REMARKS:  
Drilled ahead of casing from 20' to 45' depth; casing blows not recorded.  
Dirty drilling water and low recovery indicate that rock core from 1C was probably soft and highly weathered and therefore washed away during drilling.

(CONTINUED.)

DESIGNED	DATE	REFERENCE DRAWINGS	REVISIONS		<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> MOESER - RUTLEDGE - WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y.	DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT	<b>ROCKVILLE ROUTE (A011)</b> LOGS OF AM-SERIES BORINGS Nos. AM-47 to AM-58U
DRAWN	DATE	NUMBER	DESCRIPTION				
CHECKED	DATE	DATE	DATE				
APPROVED	DATE	DATE	DATE				
					SUBMITTED	APPROVED	SCALE
					William H. Weese		VERT. 0' 3' 6' 12'
							DRAWING NO. F-A-117

**BORING NUMBER AM-59U**  
Ground surface elevation +353.2

10	10	3,4,5	Brown silty clay, trace gravel & mica (Fill) (CL)	FILL	2.0'
15	20	P	Brown micaceous silty fine sand (SM)		
20	30	4,4,6	Do 2U, residual soil (SM)		
25	40	4U	Do 2U (SM)		
30	50	5,7,10	Do 2U (decomposed rock)		
35	60	14,16,20	Brown micaceous silty fine sand, trace rock fragments (SM)		
40	70	7,15,26	Do 6D (SM)		
45	80	23,51,111	Do 6D, becoming rockier (SM)		
50	90	150/4'	Do 6D, numerous rock fragments (SM)		
55	100	90/15	Highly weathered & jointed quartz diorite gneiss		
60	20	85/20	Do 1C		
65	30	90/10	Do 1C		
70	40	75/15	Hi wthrd & jtd, 5 20° jts, 2 60° jts, 1 80° jt, jts highly stained, soft rock breakable with hand		
75	50	55/0	Do 4C, 1 70° jt, 2 50° jts, 2 30° jts 50'-53', broken & weathered		
80	60	80/0	55'-57' Do 4C, 57'-60', unwthrd hi jtd gray fine grained, 4 60° jts, 5 30° jts, joints stained & filled w/calcite layer		
85	70	95/40	Do 6C, 4 60° jts, 3 20° jts slight staining at jts, 7 jts per 4', thin calcite on jts		
90	80	98/98	Sound, no joints		
95	90	95/80	Rel sound, 1 60° jt, 4 10°-20° jts, 1 jt per 4' widely spaced		
100	100	100/70	Mod jtd, 1 60° jt, 2 45° jts, 3 20° jts, jts widely spaced at 72'-75', closely spaced 75' to 76.7'		
	110	98/95	Rel sound, 1 30° jt, 1 45° jt, 76.7' to 77.3' diorite mixed w/chlorite & fine grained		
	120	95/75	Mod jtd, 4 60° jts, 1 10° jt, slight calcite on jts, 1 jt per 3', rel sound 80'4" to 83.5", jtd from 83.5' to 84.6'		
	130	95/65	Jointed, 8 45° jts, 1 30° jt, 1 70° jt, 9 jts per 4.5'		
	140	95/75	Mostly rel sound, jointed from 90.5'-91.5', 4 30° jts, 1 70° jt, slight calcite deposit on joints		
	150	98/95	1 20° jt smooth, 1 45° jt thin calcite		
	160	98/95	1 10° jt 1/8" open, 1 30° jt 1/8" open, iron stains on joint, 50°-60° fol		

**BORING NUMBER AM-59U (CONTINUED)**

100	170	98/95	1 30° jt @ 102.8', looks fresh, tr pyrite, 1 30° jt healed jt, 1/8" calcite deposit on 10° jt, rough at 105.8'	REL SOUND TO SOUND GRAY DIORITE GNEISS, META-DIORITE AND DIORITE WITH INDISTINCT FOLIATION	118.7'
105	180	95/95	1 45° jt irregular, slightly iron stained 2.3', 1 20° jt irregular, thin calcite dep, 0.7' chlorite band, 60° contact w/diorite @ 109.7'		
110	190	98/95	Rel sound diorite, 1 30° jt @ 112.2', slightly rough, no foliation		
115	200	98/98	1 30° jt at 116.7', many dk mineral inclusions, no foliation		
120	210	95/25	Jtd, 3 60° jts, 2 45° jts, 3 30° jts, 1 vert jt, 5 10° jts in 2.9', sli stain on jts, gouge mtl, pos shear	JTD QTZ DIO GMS	121.0'
125	220	98/90	1 60° jt, 1 30° jt, 1 45° jt in 1.2', 2 60° jts in 1/2", close, talc & calcite deposits on joints	RELATIVELY SOUND DIORITE GNEISS	130.0'
130	230	98/95	1 70° jt, 1 perp jt in 0.5' @ 129.5', 1 60° jt, slightly rough, thin calcite deposits & pyrite	JOINTED GNEISS, WAVY JOINTS, POSSIBLE SHEAR ZONE	139.5'
135	240	95/55	2 50°-60° jts in 0.1', sli rough, thin calcite dep, 2 45° jts, 1 60°-85° jt curved w/talc & calcite dep, 3 20° jts 1/16" open in 4.0', possible shear	REL SND DIORITE GNEISS	144.1'
140	250	95/30	Jtd, 2 30° jts, 1 70° jt, iron stained, 2 85° jts 1/16" open w/1-1/2" altered chlorite bands along jt planes, 3.2' spacing @ 135.8'	MOD JOINTED TO JOINTED DIORITE GNEISS	153.0'
145	260	95/90	Sound, 2 45° jts, 1 10° jt in 2.4'	RELATIVELY SOUND TO SOUND DIORITE GNEISS	167.6'
150	270	95/45	Vert fracture @ 144.1' to 145', 8 45° jts, 2 30° jts, 1 45° jt, slight calcite on jts		
155	280	95/70	Moderately jointed, 3 45° jts, 3 30° jts, 1 10° jt		
160	290	95/80	Relatively sound, 4 20° jts, 2 45° jts		
165	300	95/85	6 20° jts, 1 45° jt, 1 60° jt, 7 jts per 4.8'		
170	310	98/90	1 10° jt, 1 30° jt, 3 jts per 4.8'		

Boring started 6-17-74, completed 6-19-74  
Final depths: Boring = 167.6', Casing = 57'  
Casing diameter = 4" & 3"  
Average depth of ground water observed in the boring = 27.3' = El. +325.9

REMARKS:  
5" of asphalt at surface. Drilled ahead of casing from 15' to 57' depth; casing blows not reported.

**BORING NUMBER AM-60**  
Ground surface elevation +336.0

10	48	1D	6,6,5	Brown fine sandy clay occasional gravel (Fill) (CL)	SA CLAY, FILL	4.0'
15	38	2D	12,13,16	Orange-brown micaceous silty fine sand (Decomposed Rock) (SM)		
20	40	3D	15,20,28	Brown micaceous silty fine sand (Decomposed Rock) (SM)		
25	42	4D	22,29,35	Do 3D (SM)		
30	44	5D	15,39,45	Do 3D (SM)		
35	46	6D	20,20,30	Brown micaceous silty fine sand (SM)		
40	48	7D	18,27,31	Do 6D (SM)		
45	50	8D	20,30,35	Brown micaceous fine to medium sand, some silt, trace rock fragments (SM)		
50	52	9D	150/3'	Do 8D, numerous rock fragments (SM)		
55	54	1C	70/10	Hi wthrd & jtd hornblende schist, occasional quartz vein		
60	56	2C	80/15	Brn hi wthrd & jtd, quartz augens		
65	58	3C	95/30	Do 2C, coarse grained, feldspar intrusion		
70	60	4C	95/40	Do 2C, 54'-56.5', brown, jointed 56.5'-59'		
75	62	5C	95/60	Gry-brn mod wthrd, jtd, Hi wthrd @ 62.8'-63.8'		
80	64	6C	95/40	Brn wthrd schist, rel less wthrd 65'-66'		
85	66	7C	75/10	Sli wthrd schist, broken & hi wthrd 68.6' to 69.4', vert jt 70'-73', jts hi stained, 45° fol		
90	68	8C	95/0	Do 7C, hi wthrd & stained broken cross jts 66.2' to 77.2'		
95	70	9C	80/40	Sli to mod wthrd & jtd diorite gneiss, 45° fol, 2 fol jts, 1 60° jt, bot 3" schistose chlorite gneiss		
100	72	10C	90/0	Gry-gry chlorite schist, very closely spaced, hi jtd & mod wthrd, hi wthrd at 86.4' to 86.8'		
	74	11C	90/10	Do 10C, fractured & broken, Hi broken & wthrd at 91.5' to 91.8'		
	76	12C	90/0	Sli wthrd, hi jtd, 60° fol, 6 fol jts, rusty, stained		
	78	13C	95/0	Do 12C, 45° fol, 8 fol jts, wthrd sections & broken 96'-97' & 99' to 100', probable shear		

Boring started 5-14-74, completed 5-28-74  
Final depths: Boring = 165.0', Casing = 99'  
Casing diameter = 4" & 3"  
Average depth of ground water observed in the boring = 23.0' = El. +323.0

REMARKS:  
5" of asphalt at surface. Drilled ahead of casing from 13' to 40' depth; casing blows not reported. Hole began caving in at 80' depth while drilling at 142' depth, therefore flush joint driven to 99' depth. 38' of 4" casing left in place for borehole photography.

**BORING NUMBER AM-60 (CONT'D)**

100	140	98/25	Unwthrd, jtd, 45° fol, 10 fol jts, staining on jts	UNWTHRD JOINTED SCHIST	105.4'
105	150	95/85	Rel snd schistose, widely spaced 5 jts at 45° fol	REL SOUND SCHIST	110.2'
110	160	98/40	Schist mixed with diorite, closely spaced, 15 fol jts @ 45°, slight staining on jts, jts coated with calcite & talc	JOINTED SCHIST & DIORITE	115.0'
115	170	98/40	Gry unwthrd jtd diorite, vert jt stained @ 115.4', @ 117.6', 45° fol	GRAY UNWTHRD AND JOINTED DIORITE, FOLIATION 45°	126.0'
120	180	90/55	Do 17C, fol 45°, 6 fol jts 2 10° jts, slight staining along jts	GRN-GRAY JOINTED AMPHIBOLITE	131.8'
125	190	85/60	Do 18C, 45° fol, 3 fol jts stained amphibolite @ 126' to 127', 3" broken at contact, quartz augens	GRAY UNWEATHERED JOINTED DIORITE, 40° FOL	141.2'
130	200	95/75	Gry-gry unwthrd amphibolite w/ feldspar bands, 7 jts @ 45°-60°, 2 jts @ 10°	RELATIVELY SOUND GRAY UNWEATHERED DIORITE	151.0'
135	210	95/60	Jtd meta diorite, 40° fol, 6 fol jts, 2 10° jts, thin 2" quartz band @ 133.5'	MODERATELY JOINTED GRAY UNWEATHERED DIORITE, FOLIATION 40° TO 60°	165.0'
140	220	90/60	Gry jtd meta diorite, 40° fol, 2 60° jts, 5 35° jts, chlorite schist from 142' to 143', jts unwthrd & stained		
145	230	98/75	Widely spaced jts, 4 fol jts, fol 45°, sli talc coating on jts, rel snd		
150	240	98/75	45° fol, 4 fol jts, fractured @ 149.5' to 150'		
155	250	90/60	Mod jtd, 3 jts @ 45°, 1 jt @ 30°, vert jt @ 154'		
160	260	98/65	6 jts @ 45°, vert jt 159.5'-160', chlorite schist alt		
165	270	98/55	2 jts @ 45°, 45° fol, vert jt @ 163'-165'		

**BORING NUMBER AM-61**  
Ground surface elevation +320.8

10	8	1D	1,2,3	Topsoil & brown micaceous fine sand with silt (Fill) (SM)	SANDY TOPSOIL AND FILL	5.0'
15	20	2D	15,15	Stiff brown micaceous fine sand with silt, possible fill at top (SM)		
20	22	3D	8,3,14	White-brown micaceous fine sand, some silt, trace rock fragments (SM)		
25	24	4D	21,22,24	Gray-green silty fine to medium sand, (Decomposed Rock) (SM)		
30	26	5D	8,12,15	White-brown micaceous sand with silt, (Decomposed Rock) (SM)		
35	28	6D	35,80,100/2'	Do 5D, with rock fragments (SM)		
40	30	1C	70/0	Decomposed, highly jtd hornblende quartz diorite, 15 horiz jts, 2 diag jts, hi stained, mainly at jts		
45	32	2C	85/30	Hi jtd & wthrd in top, 7 fol jts, iron stains to 33'		
50	34	3C	80/35	Mod wthrd, 2 45° jts diag, 2 30° jts, 4 10° jts, 8 small pieces		
55	36	4C	100/50	Hi jtd, mod wthrd, 10 diag jts, 2 cross jts, 1 vert jt, 5 horiz jts, mod wthrd, staining at jts		
60	38	5C	90/55	Hi jtd, mod to sli wthrd, 8 diag jts, 3 horiz jts, 1 vert jt, 2 cross jts, staining at jts, quartz at 144'		
65	40	6C	100/60	Hi jtd, sli wthrd, 8 diag jts, 7 horiz jts, staining at jts, slickenside 50' to 51.5'		
70	42	7C	90/55	Jtd, sli wthrd, 8 diag jts, 1 vert jt, 4 horiz jts, staining at jts, mod wthrd 55'-56'		
75	44	8C	85/45	Mod jtd, mod wthrd, 4 diag jts, 6 horiz jts, 1 vert jt, staining at jts 56'-57.5'		
80	46	9C	90/20	Gradual alteration in diorite, hi jtd, sli wthrd, 10 diag jts, 9 horiz jts, 2 cross jts, staining, talc & quartz at jts		
85	48	10C	90/30	Hi jtd & fract 66'-68' w/ slick jts, probable fault, 10 horiz jts, 7 diag jts, 3 cross jts		
90	50	11C	65/15	Fract & sli wthrd, 5 diag jts, 3 horiz jts, broken & mod wthrd w/slick jts, possible shear 70'-71' & 73'-73.5'		
95	52	12C	98/90	Rel snd, 1 diag jt, 1 horiz jt, 4 jts per 5 feet 3 horiz jts, 1 diag jt		
100	54	13C	90/75	1 diag jt, 2 horiz jts		
	56	14C	90/90	Jtd, 1 diag jt, 5 horiz jts, typical jts 30° & 60°		
	58	15C	98/95	Mod jts, 1 diag jt, 3 horiz jts, chlorite inclusion		
	60	16C	75/70	Rel snd, 1 horiz jt, 2 20° 1 30° jt, 1 45° jt, hornblende inclusion, 0.3' band 98.7'		
	62	17C	95/95	2 20° jts, 1 60° jt 97.5' with thin calcite deposit, 0.8' quartz vein 99.2' to 100' @ 70°, trace chlorite inclusion		
	64	18C	95/90			

**BORING NUMBER AM-61 (CONT'D)**

100	190	95/95	With chlorite, 2 horiz jts becoming fresh & unstained		
105	200	95/90	2 horiz jts		
110	210	100/100	1 diag jt, 2 horiz jts, pyrite at jts		
115	220	98/80	2 diag jts, 4 horiz jts, pyrite at jts		
120	230	98/85	Gry sli wthrd & jtd diorite, 4 diag jts @ 50°, 45°, 60° & 30°, talc on jts		
125	240	98/98	Gry-grn rel snd to snd diorite with hornblende inclusions		
130	250	100/100	No joint except mechanical break		
135	260	100/95	1 diag jt at 45°		
140	270	98/95	Gry-grn diorite, 1 diag jt 45°, little talc on jt		
145	280	95/90	2 diag jts, 45°, very little talc on jts		
150	290	98/98	No breaks except one mechanical break, hornblende inclusions at intervals		
155	300	95/85	Gry-grn diorite, 4 vert jts, 3 diag 50°-60° jts, hornblende inclusions, very little talc on jts		
160	310	98/90	1 diag jt @ 85°, 1 almost vert jt, chloritic mtl on jt, quartz vein @ 150'		

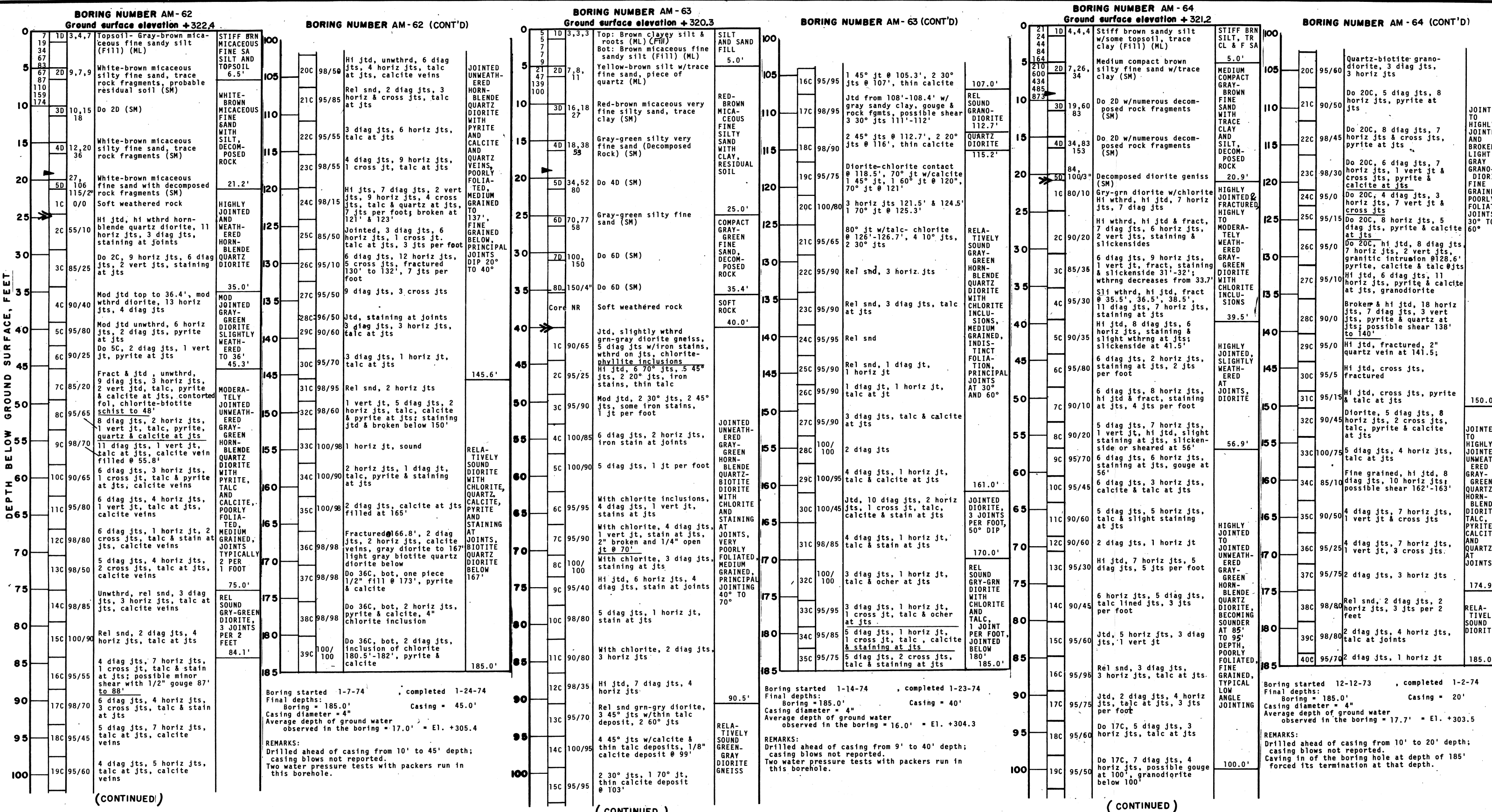
Boring started 1-4-74, completed 2-6-74  
Final depths: Boring = 152.4', Casing = 25.0'  
Casing diameter = 4"  
Average depth of ground water observed in the boring = 15.7' = El. +305.1

(CONTINUED)

(CONTINUED)

(CONTINUED)

<p>DESIGNED: IHL          DRAWN: IHL          CHECKED: SLT &amp; JPG          APPROVED:</p>	<p>DATE: 12/74          DATE: 12/74          DATE:</p>	<p>REFERENCE DRAWINGS          NUMBER: F-1          DESCRIPTION: General Notes &amp; Legend          NUMBER: FA-66 to          DESCRIPTION: Plans &amp; Geological Sections          NUMBER: FA-104</p>	<p>REVISIONS          DATE BY DESCRIPTION</p>		<p><b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b>          MESER - RUTLEDGE - WENTWORTH &amp; JOHNSTON          CONSULTING ENGINEERS          415 MADISON AVE., NEW YORK 17, N. Y.</p>	<p>DE LEUW, CATHER &amp; COMPANY          GENERAL ENGINEERING CONSULTANT          HARRY WEESE &amp; ASSOCIATES          GENERAL ARCHITECTURAL CONSULTANT</p>	<p><b>ROCKVILLE ROUTE (A011)</b>          LOGS OF AM-SERIES BORINGS Nos. AM-59U to AM-61</p>	<p>SCALE: VERT. 0" = 5' = 10'          DRAWING NO. F-A-118</p>
---	--	---	---	--	---	--	--	--



DESIGNED DRAWN IHL CHECKED SLT & JPG APPROVED	DATE 12/74 DATE 12/74 DATE	<b>REFERENCE DRAWINGS</b> NUMBER DESCRIPTION F-1 General Notes & Legend FA-62 to Plans & Geological Section FA-104	<b>REVISIONS</b> DATE BY DESCRIPTION		<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> MUESER • RUTLEDGE • WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y. SUBMITTED <i>William H. Niles</i>	DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT	<b>ROCKVILLE ROUTE (A011)</b> <b>LOGS OF AM-SERIES BORINGS Nos. AM-62 to AM-64</b> SCALE VERT. 0' 3' 6' 12' DRAWING NO. F-A-119
--	----------------------------------	--	---	--	--	---	--



**BORING NUMBER AM - 65**

Ground surface elevation + 321.6

0	7 1D	3.8, 8	Topsoil, stiff brown micaceous fine sandy silt (Fill) (ML)	STIFF SILT, FILL	5.0'
5	50 175 200 225	4.4, 7	Brown completely decomposed residual soil, fine sandy silt (ML)	SANDY SILT	10.0'
10	3D	20, 30 34	Brown-green compact micaceous fine sand with some silt (Decomposed Rock) (SM)	BRN-GRN COMPACT MICACEOUS FINE SAND WITH SOME SILT AND DECOMPOSED ROCK FRAGMENTS	26.0'
15	4D	28, 80 100	Do 3D, with some rock fragments (SM)	WTHR'D, JOINTED DIORITE	30.0'
20	5D	45, 100	Do 3D, with numerous rock fragments (SM)	JOINTED DIORITE UNWEATHERED WITH STAINING AT JOINTS	43.0'
25	6D	50, 140	Do 3D, with numerous rock fragments (SM)	RELATIVELY SOUND GRAY-GREEN DIORITE GNEISS	56.0'
30	1C	60/15	Wthr'd, hi jtd diorite, 7 horiz jts, 3 diag jts, staining at jts	JOINTED TO HIGHLY JOINTED GRAY-GREEN DIORITE	76.2'
35	2C	95/40	Jtd, sll wthr'd diorite, 7 horiz jts, 3 diag jts, staining at joints	RELATIVELY JOINTED TO MODERATELY JOINTED GRAY-GREEN DIORITE	93.7'
35	3C	98/40	1 vert jt, 8 diag jts, 3 cross jts, hi jtd, unwthr'd except for stain at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
40	4C	98/50	Jtd, 6 diag jts, 7 horiz jts, shear @ 39.5'	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
45	5C	100/100	Rel snd, 1 diag jt, stain & talc at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
50	6C	100/100	Do 5C, 1 diag jt, 1 horiz jt, stain at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
55	7C	100/98	Do 5C, 3 diag jts, calcite & pyrite at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
55	8C	100/70	Mod jtd, 4 diag jts, 2 vert jts, 2 horiz jts, staining & calcite at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
60	9C	98/25	Jtd, fract, possible shear @ 63', 6 diag jts, 2 vert jts, 2 horiz jts, stain & calcite at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
65	10C	90/20	Jtd, fract, shear 67.5' to 69.5', 2 vert jts, 7 diag jts, 3 horiz jts, stain & calcite at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
70	11C	90/70	4" clay seam @ 69', 2 vert jts, 5 horiz jts, 2 diag jts, talc at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
75	12C	90/15	Hi jtd & fract, 8 horiz jts, 9 diag jts, 2 vert jts, calcite, talc & quartz at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
80	13C	95/0	Hi jtd & fract, shear & clay seam, 79' to 83'	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
85	14C	98/0	Hi jtd, fract, shear to 83', broken @ 84', calcite at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
90	15C	98/40	4 diag jts, 3 horiz jts, fractured	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
90	16C	98/25	Hi jtd, 11 diag jts, 4 horiz jts, quartz & talc at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
95	17C	70/45	2 diag jts, 1 vert jt, talc & calcite at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
95	18C	90/85	3 diag jts, calcite & talc at jts	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'
100	19C	90/90	1 cross jt, 3 diag jts mostly @ 45°, calcite & talc on jt planes	MODERATELY JOINTED TO RELATIVELY SOUND GRAY-GREEN DIORITE	99.0'

**BORING NUMBER AM - 65 (CONT'D)**

100	20C	90/80	Mod jtd, 3 diag jts 45° & 60°	WEATHERED AND JOINTED	108.5'
105	21C	90/55	3 diag jts @ 30°, 45°, 60°, 1 vert jt, calcite on jts, veins at 106.5'-107.3'		
110	22C	98/55	3 diag jts @ 30° & 60°, horiz calcite coating on joint at 109'		
115	23C	98/30	Jts @ 30°, 60° & 75°, 1 vert jt, calcite & talc on jt planes		
120	24C	98/35	8 diag jts @ 30° to 60°, calcite & talc on jts	LIGHT COLORED GRAY-GREEN JOINTED GRANO-DIORITE, POORLY FOLIATED, FINE GRAINED, SOME JOINTS STAINED OR COATED, BROKEN AT 116', 138', 139', 142' TO 144' AND 146'	
125	26C	98/35	Diag jts mainly @ 10° & 80°		
125	27C	88/65	4 diag jts, 1 cross jt 40° to 60°		
130	28C	95/70	Mod jtd to jtd, 7 diag jts mostly @ 38°, 45° & 50°		
135	29C	90/30	11 diag jts mostly 30° to 60°, 1 cross jt @ 135.8', talc on jts		
140	31C	98/40	2 diag jts @ 45° & 60°, talc at jts, broken at 138'-139'		
145	33C	98/0	2 diag jts, 1 vert jt, talc on jt		
150	34C	80/45	5 jts @ 80° to 85°		
155	35C	98/80	Mod jtd to jtd diorite, 2 diag jts @ 45° & 50°, calcite, talc & iron staining at jts		
160	37C	90/90	Jtd diorite, 1 diag jt @ 45°, calcite & talc on jt		
165	39C	100/85	3 diag jts, 2 @ 80° & 1 @ 45°, calcite & iron staining on jts		
170	40C	95/85	3 diag jts, 1 60°, 2 45°, calcite on jts		

Boring started 1-24-74, completed 2-5-74  
 Final depths:  
 Boring = 173.0'  
 Casing diameter = 3"  
 Average depth of ground water observed in the boring = 17.0' = El. +304.6

REMARKS:  
 Drilled ahead of casing from 9' to 24' depth; casing blows not reported.  
 Caving in occurred at 166' to 168' depth.  
 Observation well consisting of 1-1/2" steel pipe was installed with tip at 166.5' depth.

**BORING NUMBER AM - 66**

Ground surface elevation + 320.7

0	21 1D	7.10, 10	Stiff brown fine sandy silt & medium compact silty fine sand (Fill) (ML & SM)	STIFF BRN F SAND & SILT, FILL	5.0'
5	2D	7.32, 50	Very compact gray-brown silty fine sand (SM)	VERY COMPACT GRAY-BROWN SILTY FINE SAND, DECOMPOSED ROCK	20.2'
10	3D	184, 300/11	Do 2D, with numerous weathered rock fragments (SM)	MOD WTHR'D GRAY-GREEN DIORITE WITH CHLORITE	27.7'
15	4D	270/6	Very compact moist gray-brown fine sand & rock fragments (SM)		
20	5D	100/2	Do 4D, more rock fragments (SM)		
25	1C	20/10	Wthr'd & broken gray-green diorite with chlorite		
25	2C	50/15	7 jts almost horiz, sm staining at jts		
30	3C	98/70	Sll wthr'd at jts, 3 diag jts, 4 horiz jts, staining at jts, chlorite inclusions		
35	4C	98/90	3 diag jts, talc & some pyrite at jts, unwthr'd, rel snd		
40	5C	98/98	Do 4C, 2 horiz jts		
45	6C	95/90	Do 4C with 2 diag jts, 1 horiz jt, calcite vein		
50	7C	90/85	3 diag jts, talc at jts		
55	8C	98/80	3 diag jts, 1 horiz jt, talc at jts, possible movement at 52.5'		
60	9C	90/85	3 diag jts, 1 horiz jt, talc & pyrite at jts, calcite vein at 55.7'		
65	10C	100/100	3 diag jts, 1 horiz jt, talc at jts		
70	11C	100/90	with 3 diag jts		
75	12C	90/65	Mod jtd, 4 diag jts, 3 horiz jts, talc at jts, some with calcite		
80	13C	98/60	Fine grained from 76.2' to 78', 3 diag jts		
85	14C	98/98	5 diag jts, 1 horiz jt, talc at jts, filled jt at 84.4'		
90	15C	98/98	3 diag jts, 2 diag jts		
95	16C	100/98	3 horiz jts, 2 diag jts		
100	17C	98/98	3 diag jts, 1 horiz jt		
105	18C	100/85	5 diag jts, 1 horiz jt, talc & pyrite at jts		
105	19C	98/98	3 diag jts, 1 horiz jt, talc & pyrite at jts		
105	20C	98/90	3 diag jts, 3 horiz jt, pyrite, calcite & talc at jts		

**BORING NUMBER AM - 66 (CONT'D)**

105	21C	98/98	2 diag jts, 1 horiz jt, pyrite, calcite & talc on jts	GRAY-GREEN DIORITE, UNFOLIATED WITH Talc, PYRITE & CALCITE	111.7'
110	22C	95/90	5 diag jts, pyrite & talc at jts, 2 jts smooth & probably slickensided	MODERATELY JOINTED TO HIGHLY JOINTED GRAY-GREEN DIORITE WITH Talc, PYRITE AND CALCITE	138.2'
115	23C	90/75	Mod jts, 6 diag jts, 2 horiz jts, pyrite, talc & calcite on jts, jts filled at 117.6'		
120	24C	98/75	Mod jtd, 6 diag jts, 7 horiz jts, talc & pyrite on jts		
125	25C	98/65	Jtd, 9 diag jts, 6 horiz jts, 1 vert jt, talc & pyrite at jts, filled at 126.4'		
130	26C	95/70	Mod jtd, 9 diag jts, 5 horiz jts, 1 vert jt, talc at jts		
135	27C	98/45	Hi jtd, 6 diag jts, 2 horiz jts, 2 vert jts, talc & pyrite at jts		
140	28C	90/60	5 diag jts, 3 horiz jts, sm staining at jts, quartzite intrusion at 140.2'		
145	29C	98/70	3 diag jts, 6 horiz jts, talc & calcite at jts		
150	30C	98/75	4 diag jts, 4 horiz jts, 2 vert jts, staining, pyrite & talc at jts		
155	31C	98/60	with chlorite inclusions, 5 diag jts, 2 horiz jts		
160	32C	98/60	6 diag jts, 1 horiz jt, talc & pyrite at jts		
165	33C	98/75	4 diag jts, 3 horiz jts, fractured at 161', staining & calcite at jts		
165	34C	100/75	4 diag jts, 3 horiz jts, 1 vert jt, talc & quartz at jts		
170	35C	98/80	7 horiz jts, 3 diag jts, filled at 169.8'		
175	36C	98/60	9 horiz jts, 7 diag jts, 2 vert jts, 1/2" quartz vein at 175.5', jtd & fractured		
180	37C	98/65	4 diag jts, 4 horiz jts, 1 vert jt, quartz & talc at jts		
185	38C	95/50	Hi jtd, 5 diag jts, 5 horiz jts, 1 vert jt, quartz vein at 184.3', pyrite & calcite jts		
190	39C	98/50	Hi jtd, 3 diag jts, 10 horiz jts, 1 vert jt, fractured between 187' & 188'		

Boring started 11-30-73, completed 12-27-73  
 Final depths:  
 Boring = 190.2'  
 Casing diameter = 4"  
 Average depth of ground water observed in the boring = 17.7' = El. +303.0

REMARKS:  
 At beginning of work day 12-11-73 the boring had to be offset 3 feet south because of debris in the hole then 15.5' deep.  
 Drilled ahead of casing from 5' to 20' depth; casing blows not reported.  
 Three water pressure tests with packers run in this borehole.

**BORING NUMBER AM - 67**

Ground surface elevation + 325.7

0	14 1D	2.5, 5	Medium stiff brown-red clayey silt, trace fine sand, trace mica (Fill) (ML)	M STIFF BRN-RED SILT, FILL	5.0'
5	2D	21, 25 27	Stiff brown-red clayey silt, trace fine sand, trace mica, residual soil (ML)	BROWN-RED CLAYEY SILT, TRACE FINE SAND	15.0'
10	3D	30 34	Medium stiff brown micaceous silty fine sand, trace clay (SM)		
15	4D	6, 10, 13	Medium compact gray-brown micaceous silty fine sand (SM)		
20	5D	10, 13 14	Do 4D, trace rock fragments (SM)	COMPACT WHITE-BROWN FINE SAND, SOME SILT, NUMEROUS ROCK FRAGMENTS	35.5'
25	6D	10, 28 35	Compact white-brown fine sand, some silt (Kaolinite) (SM)		
30	7D	40, 100	Very compact dark brown fine to medium sand, some silt, numerous rock fragments (SM)	GRAY-BRN FINE TO MEDIUM SAND	40.1'
35	8D	100	Do 7D (SM)	HIGHLY JOINTED TO RELATIVELY SOUND DIORITE WITH CHLORITE INCLUSIONS	53.0'
40	9D	25/0	Hi jtd, slightly wthr'd light gray-white diorite gneiss	JOINTED DIORITE	57.5'
45	10D	98/0	Hi jtd gray-green, altered diorite, 7 45° jts, 8 10° to 20° jts, 1 70° jt 42.4' to 43' w/ thin calcite, vert jt 43'-43.5', possible shear zone	RELATIVELY SOUND UNWEATHERED GRAY-GREEN DIORITE GNEISS WITH TALC ON JOINTS	80.0'
50	11D	98/95	Sound gry-grn altered fine grained, 2 45° jts 48'-48.3' 2 30° jts, 2 45° jts, iron stain, small rock fgmts on jt @ 52.7', gouge mtl on possible shear		
55	12D	95/75	2 45° jts, 2 30° jts, jointed diorite		
60	13D	95/90	Rel sound diorite gneiss 2 30° jts w/ calcite, 2 60° jts		
65	14D	95/90	3 diag jts, 1 horiz jt		
70	15D	100/95	2 diag jts, 2 horiz jts, talc at joint		
75	16D	90/80	3 horiz jts, 2 diag jts		
80	17D	90/70	Rel sound to mod jtd, 2 horiz jts		
85	18D	98/50	6 horiz jts, 2 diag jts, alteration & fracture at 86.5'		
90	19D	40/40	2 horiz jts		
95	20D	80/80	Rel sound diorite gneiss, 1 horiz jt		
100	21D	100/90	2 horiz jts, 1 diag jt		
105	22D	100/98	1 horiz jt, 1 diag jt, talc & pyrite on jts		
105	23D	90/75	4 horiz jts, 1 diag jt, talc & pyrite on jts, presence of calcite noted		

**BORING NUMBER AM-67 (CONTINUED)**

105	19C	95/65	1 diag jt, 3 horiz jts	RELATIVELY SOUND TO SOUND GRAY-GREEN DIORITE GNEISS WITH CHLORITE, TALC, PYRITE AND CALCITE ON JOINTS, NUMEROUS CHLORITE INCLUSIONS AND INTERBEDS	136.0'
110	20C	80/75	4 horiz jts, 1 diag jt, quartz vein at joint		
115	21C	95/35	5 horiz jts, 3 diag jts, 1 vert jt, 1 cross jt		
120	22C	98/80	With chlorite inclusions, 5 horiz jts, 1 diag jt, quartzite & calcite on jts		
125	23C	98/95	With chlorite inclusions, 2 horiz jts		
130	24C	98/95	With chlorite inclusion, 1 horiz jt		
135	25C	95/90	Pyrite & talc at joints, 2 horiz jts, 1 diag jt		
140	26C	90/70	Diorite gneiss with chlorite inclusions, 1 diag jt, 4 horiz jts, 1 cross jt, calcite & talc at jts		
145	27C	90/85	With chlorite inclusions, 4 horiz jts		
150	28C	90/70	With chlorite inclusions, 7 diag jts, 3 horiz jts		
155	29C	85/35	With chlorite inclusions, 3 diag jts, 4 horiz jts		
160	30C	100/60	2 vert jts, 5 horiz jts	MODERATELY JOINTED TO JOINTED GRAY-GREEN DIORITE GNEISS WITH CHLORITE INCLUSIONS, PARTICULARLY ABOVE 153' DEPTH	
165	31C	80/70	3 horiz jts, calcite on jts, 1 diag jt		
170	32C	100/85	2 diag jts, 3 horiz jts		
175	33C	90/60	4 horiz jts, 2 diag jts		
180	34C	100/80	3 diag jts, 3 horiz jts		
185	35C	90/50	With chlorite inclusions, 4 diag jts, 3 horiz jts, talc on jts		
190	36C	75/75	1 diag jt, talc on jts		
195	37C	100/60	3 diag jts, talc on jts, 4 horiz jts		
200	38C	100/75	Mod jts gry diorite gneiss, thinly laminated, 2 horiz jts, 2 30° jts, 3 10° jts, thin talc deposit on joints, no staining		

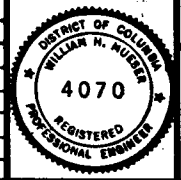
Boring started 12-18-73, completed 1-2-74  
 Final depths:  
 Boring = 189.0'  
 Casing diameter = 4"  
 Average depth of ground water observed in the boring = 29.0' = El. +296.7

REMARKS:  
 Drilled ahead of casing from 30' to 34' depth; casing blows not reported.  
 Driller was unable to retrieve the casing from this hole.  
 Three water pressure tests with packers run in this borehole.

(CONTINUED)

(CONTINUED)

(CONTINUED)

DESIGNED		DATE		REFERENCE DRAWINGS		REVISIONS				<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> MUESER - RUTLEDGE - WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y. SUBMITTED <i>William A. ...</i>				DE LEUW, CATHAR & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT				ROCKVILLE ROUTE (AO11) LOGS OF AM-SERIES BORINGS Nos. AM-65 to AM-67			
DRAWN	IHL	DATE	12/74	NUMBER	F-1	DESCRIPTION	General Notes & Legend											DATE	BY	DESCRIPTION	DATE

**BORING NUMBER AM-68**  
Ground surface elevation +321.5

0	10,16	Brown sand & gravel, trace silt (Fill) (SP-SM)	FILL	4.5'
5	13,14	Brown micaceous decomposed rock, silty fine sand (SM)		
10	11,15	Do 2D (SM)		
15	18,17	Do 2D (SM)		
20	4,9	Do 2D (SM)		
25	20,40	Do 2D, rock fragments increase (SM) Hard drilling at 28'		26.8'
30	80/30	Silt w/ thrd & jtd granitic gneiss, indistinct fol, 11 jts per 4', jts are rough, straight & iron stained	WEATHERED AND JOINTED HORN-BLENDE BIOTITE QUARTZ DIORITE, POORLY FOLIATED, JOINTS DIP TYPICALLY 40° TO 60°	54.0'
35	90/20	Do 1C, 15 jts per 4.4', jts rough, irregular & iron stained		
40	90/20	Do 1C, 16 jts per 4.5', jts are rough, straight & iron stained		
45	95/35	Do 1C, 20 jts per 3.6', joints are rough, straight & iron stained, typical 45° to 60°		
50	85/0	Do 1C, 9 jts per 1.7'		
55	90/70	Silt w/ thrd, 21 jts per 4.4' badly fractured & jtd 52.6' to 54', 4' band of dec rock at 53.7'		
60	85/55	Unw/ thrd, extremely jtd to jtd, 7 jts per 3.8', jts are rough, straight, iron stained	MOD JOINTED TO JOINTED DIORITE	62.5'
65	98/80	Do 7C, 8 jts per 4', soft badly jointed 4" zone at 61.5' to 62.5'		
70	100/10C	Unw/ thrd inc jt, 1 jt per 1.4', jts smooth, straight		
75	100/10C	Do 9C, 1 jt per 4.5', jt is rough, straight, filled with secondary mineral		
80	98/98	Do 9C, 1 jt closed, jt rough, straight, filled with secondary minerals		
85	85/50	Do 9C, breaks due to drilling action	MODERATELY JOINTED TO RELATIVELY SOUND QUARTZ BIOTITE DIORITE, UNFOLIATED TO POORLY FOLIATED, MEDIUM GRAINED	
90	98/45	Do 9C, 6 jts per 4.2', jts are smooth & straight, secondary minerals, 2' calcite vein at 78.4'		
95	90/80	Do 9C, 10 jts per 4.5', jts are rough & straight, secondary minerals, banded zone at 83'		
100	98/98	Do 9C, 4 jts per 4.3', soft chlorite zone from 86.5' to 86.8'		
100	98/98	Rel snd, 1 jt per 4.5', soft chlorite some 91' to 91.5'		
100	100/100	Do 16C, 3 jts per 4.5', jts are smooth & straight, secondary mineralization, sm dark chlorite patches		

**BORING NUMBER AM-68 (CONT'D)**  
Ground surface elevation +321.5

100	100/98	Do 16C, 3 jts per 3.8', jts are smooth, straight, secondary mineralization		104.5'
105	100/100	Sound lt gry quartz biotite diorite, 3 irregular 10° jts 107'-107.8' chlorite, no fol		
110	100/100	3 irregular 10°-20° jts, chlorite inclusions 1/2" to 2", rectangular to oval shape		
115	100/100	1 30° jt, 2 irregular jt	REL SND QTZ DIORITE WITH CHLORITE SCHIST INCLUSIONS & ALTERED FELDSPAR, HORN-BLENDE BIOTITE BANDS, THIN CALCITE DEPOSITS ON JTS, UNFOLIATED HORN-BLENDE ZONE 167' TO 182'	
120	98/98	1 30° jt, 1 20° jt, 3 pieces		
125	98/98	3 irregular horiz jts, 3 pieces, 1 piece 4"		
130	98/98	1 10° irregular jt at 125.9', 2 horiz jts at 127', 128', 1 30° healed jt at 125'		
135	100/100	1 50° jt at 131.5', 2 10° jts, 1 horiz jt at 132.8'		
140	100/100	Chlorite bands & inclusions 135.5' to 136.5', 6 pieces		
145	100/100	3 10° jts, irregular, chlorite inclusion 138.6' to 138.9', no foliation		
150	100/100	2 10° jts in 0.8', angular 60° chlorite inclusion		
155	100/100	2 10° jts, 3 pieces		
160	100/100	1 horiz jt, 2 20° irregular jts, 4 pieces, 1/4" feldspar band 45° @ 153.7'		
165	100/100	2 10° jts, 2 horiz jts, 1/2" feldspar band 30° @ 157.1'		
170	100/100	3 horiz jts, 1 30° jt @ 162.0', irregular inclusion at 160' & 161.6'		
175	95/95	2 20° jts in 1.4', quartz diorite altered 163' to 166' with quartzite and hornblende bands		
180	98/98	4 0°-15° jts in 3.0', 1 60° jt at 167', gry-grn quartz hornblende diorite		
185	100/90	Do 34C, 4 0°-10° jts, 2 30° jts in 3', thin calcite on dep		
190	100/100	Do 34C, 4 10° jts in 2.8'		
195	100/100	Do 34C, jts 0°-10° in 3.8' space, quartz-diorite at 182.7'		
198	98/95	Do 37C, bot, 2 60° jts; 1 70° jt in 2.5' yellow stain, thin calc dep		
199	85/85	Do 37C bot, 2 10° jts, 1 30° jt, 1 45° jt in 2'		190.0'

REMARKS:  
1.2' of concrete at surface. Drilled ahead of casing from 5' to 42' depth; casing blows not reported. Hard drilling at 28' depth, drill bit was lost in attempt to drill casing past 29.5' depth. Two water pressure tests with packers run in this borehole.

**BORING NUMBER AM-69**  
Ground surface elevation +322.8

10	8,12	Gray-brown silty & clayey fine to medium sand, trace coarse sand & gravel (Fill) (SM-SC)	MEDIUM COMPACT GRAY-BRN SILT AND FINE TO MEDIUM SAND, FILL 10.0'	
5	5,6,7	Red-brown clayey silt, some fine sand (Possible Fill) (ML)		
10	10,12	Brown silty fine sand (SM)		
15	15,15	Brown-gray silty fine sand (SM)	COMPACT TO VERY COMPACT BROWN-GRAY SILTY FINE SAND, SOME ROCK FRAGMENTS	
20	19,24	Do 4C (SM)		
25	26, 100	Brown-gray silty fine sand, trace rock fragments (SM)		
30	NR 100/0'	Probable soft rock		
35	1C 98/90	Jtd gry quartz diorite, unw/ thrd, 1 horiz jt, 2 30° jts in 2.2', thin talc deposit & stain at 37.2'		33.2'
40	2C 98/85	6 jts, 30° to 60°, some iron stain & calcite		
45	3C 98/75	2 45° jts, 46'-47' silt w/ thrd & stain on jts, broken - jtd 46.5'-47.2'		
50	4C 98/90	4 30° jts, thin calcite deposit	RELATIVELY SOUND TO MODERATELY JOINTED GREEN-GRAY ALTERED HORN-BLENDE QUARTZ DIORITE WITH CHLORITE, NO FOLIATION, MEDIUM GRAINED, THIN CALCITE DEPOSITS AND SLIGHT IRON STAINS ON JOINTS SEMI HEALED TO HEALED JOINTS, SOME GOUGE MATERIAL, POSSIBLE SHEAR ZONES, PREDOMINANT JOINTING AT 60°	
55	5C 98/95	1 30° jt, thin calcite 55.2', thin dark ml on jt, silt stained at 55.5'		
60	6C 98/85	2 20° jts in 0.6' at 57', thin dark gouge material		
65	7C 98/90	2 60° jts in 1.0', 58'-59' thin calcite deposit silt stained, 2 30° jts, 1 horiz jt in 0.7'		
70	8C 100/100	1 30° jt w/ thin calcite deposit @ 66', 1 piece 4.2' long		
75	9C 98/95	1 30° jt, 1 60° jt in 0.5', pyrite crystals & slight stain on joints		
80	10C 95/70	2 30° jts in 0.8' iron stained, 1 60° jt with calcite at 72.5', broken 74'-75' w/ 1/4" gouge @ 60°		
85	11C 98/80	7 low angle joints open 1/8" at 79', thin calcite deposit on jts		
90	12C 95/85	1 65° jt, broke on calcite healed joint plane, 1 45° jt, thin calcite, slightly stained 81.2'		
95	13C 98/75	1 85° semi-healed thin jt, thin calcite, 1 60° jt w/ calcite deposit @ 85.4'		
100	14C 98/95	1 30° jt, 2 45° jts in 1.7' 85° semi-healed jt 91' to 92', thin calcite deposit, slight stain on jts		
105	15C 98/80	3 30° jts, 2 45° jts in 3.0', 2 70°-80° jts in 0.8' at 92', thin calcite deposits, slight stain		
110	16C 98/90	1 60° jt smooth w/ calcite deposit @ 98', 1 70° jt & 1 50° jt, rough with 1/4" calcite		

REMARKS:  
Boring started 5-1-74, completed 5-15-74  
Final depths:  
Boring = 100.5' Casing = 34.5'  
Casing diameter = 4" & 3"  
Average depth of ground water observed in the boring = 16.0' = E1. +306.8

**BORING NUMBER AM-69 (CONT'D)**  
Ground surface elevation +322.8

105	17C 98/75	2 70° jts, 1 50° jt, 1 20° jt w/ thin calcite, slight iron stain		
110	18C 98/60	7 45° jts, 1 60° jt, 2 20° jts, 1 30° jt 1/4" in 1.5' 1 horiz jt, 1 10° jt in 0.2'		
115	19C 95/85	2 45° jts in 1.6', 1 60° jt, 1 70° jt in 1.2', thin calcite deposit		
120	20C 98/80	2 60° jts in 0.1' @ 114' slight iron stain & calcite deposit, 2 30° jts, 2 45° jts		118.5'
125	21C 95/55	9 10° jts 1/16" to 1/8" open, 2 30° jts 1/8" open 1/8" dep at 120.3', 3" shear at 121'	JOINTED GREEN-GRAY QUARTZ DIORITE WITH CHLORITE	
130	22C 98/25	Hi jtd & broken 123'-124', 125'-126', gouge material 123.5' & 125.5'		
135	23C 98/20	Hi jtd, 6 jts in 2', 2" broken at 129'		
140	24C 98/85	Broken 130.1'-130.6', 3 45° jts in 2.5', 3 10° to 15° jts in 3.1', gouge material at 133'		130.5'
145	25C 98/85	8 jts in 4.5', 4 45° jts, 1 60° jt, thin healed joints		
150	26C 98/75	2 20° jts, 1 45° jt in 0.5', thin calcite deposit on jts, 2 70° jts, 2 45° jts in 1.4'	RELATIVELY SOUND GREEN-GRAY ALTERED HORN-BLENDE QUARTZ DIORITE WITH CHLORITE, NO FOLIATION, CALCITE DEPOSITS ON JOINTS, MEDIUM TO FINE GRAINED, JOINTS CHIEFLY 40° TO 60°	
155	27C 98/70	2 45° jts in 2.6', thin calcite deposits on jts		
160	28C 100/95	3 45° jts, 2 50° jts 1/2" close in 1.7', calcite deposit on jts		
165	29C 100/100	1 45° jt at 151.2', trace calcite on jt, 2 pieces		
170	30C 98/95	1 30° jt, 1 45° jt, 1 50° jt in 1.1' thin calcite deposit		
175	31C 100/100	2 45° jts in 0.5', thin calcite deposit on jts, 1 piece is 4' long		
180	32C 98/60	2 70° jts in 0.1', 2 jts 60°-70°, 1 50° jt, 1 30° jt in 1.9', thin talc on jt		
185	33C 95/80	2 30° jts, 1 45° jt, 2 60° jts, 1 70° jt in 3.2', thin talc & calcite on jts		
190	34C 98/80	3 30° jts in 0.2', thin talc deposit on jts, 1 45° jt		
195	35C 98/70	1 45° jt, 1 70° jt in 0.3', thin talc deposit, 1 30° jt, 1 60° jt in 1'		178.8'
200	36C 98/70	1 70° jt, 1 80° jt, broken talc deposit, 2 10° jts, 1 45° jts in 2.5'	RELATIVELY SOUND GRAY QUARTZ DIORITE	
205	37C 95/65	3 70° jts slightly curved, 1 45° jts, 1 10° jt in 2.0'		188.5'

REMARKS:  
Drilled ahead of casing from 9' to 34.5' depth; casing blows not reported. Caving occurred at 125' depth when bottom of hole was at 143.5' depth.

**BORING NUMBER AM-70**  
Ground surface elevation +323.5

0	10,14	Green-brown micaceous sandy & clayey silt, some tan fine to coarse sand, fine gravel (Fill) (ML)	SILT AND SAND, FILL 5.0'	
5	10,14	Red-brown micaceous fine sandy silt, some clay (SM-SC)	RED-BROWN CLAYEY FINE SAND 10.0'	
10	60, 154, 208	Gray-green silty fine sand, trace mica (SM)	GRY-GRN SAND AND ROCK FRAGMENTS 16.0'	
15	254, 250/3'	Do 3D, trace medium sand with rock fragments (SM)		
20	75/0	Jtd, w/ thrd gry-grn quartz diorite, 4 60°-70° jts in 2', 3 30° jts in 1', 1/8" open	JOINTED, MOD W/ THRD TO W/ THRD GRAY-GREEN QUARTZ DIORITE	
25	50/0	2 60° jts in 2', 2 30° jts, 6 horiz jts, disintegrated 22.4'-25.6'		
30	50/0	7 horiz jts in 1', 3 45° jts in 1', 1 80° jt		
35	80/15	Hi jtd, mod to sli w/ thrd	HIGHLY JOINTED GRAY DIORITE, NO FOLIATION, THIN CALCITE, TALC DEPOSITS, POSSIBLE SHEAR 43.8' TO 48.0'	
40	95/35	3 45° jts in 2', broken 34'-35', no foliation		
45	95/75	4 jts in 2', no foliation		
50	95/30	1 80° jt, thin talc, rough, jtd broken from 41.6'-43.8'		
55	80/10	3 80° jts, calcite dep & stains, broken & jtd 46.5'-47.5', 3 10°-20° jts, thin calcite deposit		
60	90/15	2 80° jts in 1.0', calcite deposit & brown stains on jts, 1/2" calcite vein	JOINTED WITH BROKEN ZONES, GRAY QUARTZ DIORITE, THIN CALCITE DEPOSITS AND SLIGHT IRON STAINING ON JOINTS 1/8" OPEN, SLICKENSIDE AND POSSIBLE SHEAR AT 79.5'	
65	90/30	2 60° jts in 1.5', irregular surface, 5 10°-20° jts in 2', 1 0.4' vert jt @ 54.2', 8 horiz jts in 1'		
70	90/50	Broken to 57.3', 3 horiz jts in 0.15', 1 60° jt, 2 45° jts in 1', 5 10° jts in 2'		
75	95/45	Broken & jointed to 62.3' 2 70° jts, 1 80° jt slickenside @ 62.5', 1 60° jt @ 64.5', 4 20°-30° jts		
80	95/20	Broken & jtd 66.1'-67' & 69'-70.3' with calcite deposit		
85	95/45	2 60° jts in 1.5', thin calcite deposit, broken jtd 71.5'-72.5', 1 80° jt @ 70.3', 1 30° jt, 1 60° jt @ 72.8'-73'		
90	90/25	2 70° jts in 0.5', thin calcite deposit, broken 79.2'-73', 3 20° jts in 1.2', slickenside 79.5'		
95	98/45	2 60° jts, irregular to plane @ 82.7'-83.2', 1 80° jt @ 84.5', 2 10° jts in 1', thin calcite	JOINTED TO MODERATELY JOINTED GRAY QUARTZ DIORITE, THIN CALCITE DEPOSITS ON JOINTS, JOINTS OPEN 1/16" TO 1/4", MANY	
100	95/60	2 70° jts @ 87.5'-90', 3 30° jts in 2.4'		
105	19C 98/70	2 30° jts in 1.5' spacing, 1 70° jt with calcite deposits @ 95'		
110	20C 95/50	2 80° jts in 1', broken 98.5'-99', 1 45° jt open 1/4", irregular at 100'		

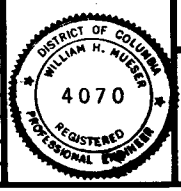
REMARKS:  
1.5' concrete street median strip at surface. Drilled ahead of casing from 10' to 15' depth; casing blows not reported. While coring rock at 75' depth the casing dropped from 15' to 23' depth. Four water pressure tests with packers run in this borehole.

**BORING NUMBER AM-70 (CONT'D)**  
Ground surface elevation +323.5

115	21C 90/85	3 10° jts in 2.1', 2 45° jts in 0.5', at 104' thin calcite deposits on jts		
120	22C 98/85	Broken top 0.2', 3 10° jts in 2.1', 1 45° jt, 2 50° jts in 0.7', 2 70° jts in 0.4', calcite deposits		
125	23C 98/50	2 horiz jts, 1 30° jt, 1 80° jt in 1.7', thin talc & calcite deposits on jts, broken & jtd from 112.5' to 114.6'	THIN HEALED JOINTS, INDISTINCTLY FOLIATED, FINE GRAINED, UNWEATHERED INCLUDING JOINTS	124.0'
130	24C 98/55	2 30° jts, 2 45° jts in 0.9' @ 145.5', calcite deposit open 1/16" 1/4", 1 45° jt, 2 60° jts in 0.4', 1 10°, 2 30°, 1 50° jts in 1.5', 1/16"-1/4" 2 30° jts, 2 45° jts, 3 80° jts, 1 70° jt, 2 15° to 25° jts in 3.5', thin calcite deposits on jts		
135	26C 98/85	Rel snd gry, 2 45° jts, 1 10° jt, thin calcite deposit in 4.0'	RELATIVELY SOUND GRAY QUARTZ DIORITE, 1 JOINT PER FOOT	136.1'
140	27C 98/85	2 30° jts, 2 45° jts in 4.0', thin calcite deposit		
145	28C 98/85	Do 26C, 2 50° jts in 0.4'		
150	29C 65/0	Jtd & broken, several small pieces, 45° & vert jts		
155	30C 98/60	Mod jtd, 3 horiz jts, 1 30° jt, 1 45° jt, 1 70° jt		
160	31C 95/65	2 jts per foot, broken & w/ thrd 145.3' to 146'		
165	32C 98/60	1 80° jt, 1 30° jt, 1 horiz jt, 1 60° jt in 2.1', 2 45° jts, 1 30° jt in 0.4' at 153'	MODERATELY JOINTED TO JOINTED GRAY QUARTZ DIORITE, MIXED	
170	33C 98/75	Rel snd, 3 horiz jts, 3 30° jts, 1 60° jt, talc deposit on joints		
175	34C 95/60	Jtd lt gry finely laminated mixed schist & diorite, 7 horiz to 10° jts, 2 30° jts, 1 70° jt 3 45° jts, 4 30° jts, 7 10°-15° jts in 2.0', 1/8" talc deposit at 165.5', 1 60° jt, 1 50° jt, 1 45° jt 1 10° jt	SCHIST AND GNEISS 158' TO 162', FOLLOWED BY LIGHT COLORED GRAY DIORITE TO BOTTOM, THIN CALCITE DEPOSITS ON JOINTS, SOME TALC, FINE GRAINED, NO FOLIATION	189.6'
180	35C 98/45	Jtd lt gry granodiorite 4 60° jts, 2 50° jts, 4 30° jts, 2 45° jts		
185	36C 90/100	3 30° jts, 3 60° jts, 1 45° jt, 4 10°-20° jts smooth to 173'		
190	37C 95/45	3 45° jts, 1 70° jt, 5 10° jts, smooth, thin talc deposit on jts		
195	38C 60/70	Hi jtd, fine grained, no foliation, 2 45° jts, 2 30° jts, 4 10° jts		
200	39C 60/20	Hi jtd in top, 4 30° jts, 1 70° jt in 0.6', 5 30° jts, 3 80° jts in 1'		
205	40C 95/50	2 30° jts, 1 60° jt in 1', thin calcite deposits		
210	41C 60/60			

REMARKS:  
1.5' concrete street median strip at surface. Drilled ahead of casing from 10' to 15' depth; casing blows not reported. While coring rock at 75' depth the casing dropped from 15' to 23' depth. Four water pressure tests with packers run in this borehole.

DESIGNED	DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION	<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> MUESER - RUTLEDGE - WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y.		DE LEUW, CATHER & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT		<b>ROCKVILLE ROUTE (A011)</b> LOGS OF AM-SERIES BORINGS Nos. AM-68 to AM-70	
DRAWN	12/74	F-1	General Notes & Legend									
CHECKED	12/74	FA-86	Plans & Geological Sections				SUBMITTED <i>William H. Mueser</i>		APPROVED		SCALE	DRAWING NO.
APPROVED		FA-104									VERT. 0' 3' 6' 12'	F-A-121



**BORING NUMBER AM-71**  
Ground surface elevation + 323.5

2	1D	20.14	Gray-brown clayey fine sand, some silt (Fill) (SM)
5	2D	7.8, 14	Compact brown silty fine sand (Possible Fill)(SM)
10	3D	14.28, 28	Brown-gray silty fine sand (SM)
15	4D	25.33, 60	Do 3D (SM)
20	5D	40.60, 85	Do 3D, trace silt (SM)
25	6D	100/3'	Do 3D (SM)
30	7D	100/4'	Brown-gray silty fine sand, trace mica (SM)
35	1C	40/0	Jtd & wthrd brn-gry hornblende quartz diorite
35	2C	85/0	Hi jtd & wthrd on jts, broken to 36.5' and 38' to 39'
40	3C	90/45	Jtd, 4 60° jts, 2 45° jts, 80° jt curved in 3.3', slightly iron stained
45	4C	90/20	2 60° jts, 1 80° jt curved, thin calcite deposit on jts
45	5C	98/85	Jtd, 25° fol, 2 fol jts, 2 50° jts, fine grained
50	6C	90/80	Do 5C
55	7C	100/90	Mod jtd, faintly fol, 4 60° jts, slight calcite deposit on jts, unwthrd & unstained
55	8C	95/70	Do 7C, 4 50° jts, 2 10° jts, slight calcite dep on jts
60	9C	100/95	Rel snd, 2 50° jts
65	10C	100/90	2 45° jts, 2 20° jts, slight alteration on jts
70	11C	95/85	2 45° jts smooth, spacing 1.1'
75	12C	100/100	Jtd, 1 60° jt, 3 45° jts smooth, 3 80° jts, thin calcite deposit 1/16" open in 2.8'
80	13C	90/65	Jointed diorite
85	14C	98/90	3 45° jts, 1 irregular jt in 1.0', 2 45° jts in 0.5', thin healed jts
90	15C	98/98	Sound, 1 horiz jt @ 86.5', several healed 45° jts
95	16C	95/75	Rel snd, 8 jts in 3.2', thin calcite deposit, slight red-brn stain, broken in bottom
95	17C	100/100	Sound, 1 30° jt, thin calcite deposit on jt, chlorite inclusions
100	18C	100/100	3 10° jts, 2 50° jts in 3.0', thin calcite deposit on jts
105	19C	100/90	2 50° jts in 0.4', thin calcite deposits on jts, 1 60° jt, 1 10° jt in 1.0' thin calcite deposits

**BORING NUMBER AM-71 (CONT'D)**

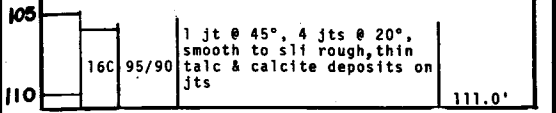
105	20C	100/95	2 60° jts in 0.8', thin calcite deposit on jts
110	21C	100/85	1 15° jt, 1 45° jt, 1 80° jt in 4.0', thin calcite deposit on jts
115	22C	100/70	5 jts per 3.2', slightly rough to 1/16" open, thin calcite deposit on jts, few healed jts
120	23C	100/90	2 60° jts, thin calcite deposit, 2 10° jts in 0.8', 1 45° jt @ 122.3', thin calcite deposits
125	24C	95/95	1 20° jt, 1 horiz jt, 1 45° jt smooth, thin calcite deposits
130	25C	100/90	1 45° jt, 1 10° jt, 1 horiz jt in 1.2', thin calcite deposits
135	26C	98/85	2 45° jts, 1 60° jts, 1 horiz jt, thin calcite & talc deposits, possible gouge at 133'
135	27C	98/95	2 30° jts, 1 open 1/4" in 1/2', smooth, thin calcite deposit on jts
140	28C	98/95	2 horiz jts, 1 30° jt open 1/8" rough, thin calcite deposits
145	29C	98/85	5 horiz jts, 1 60° jt in 4.0', thin calcite deposit on jts
150	30C	100/90	2 horiz jts, 2 30° jts, 2 45° jts, thin calcite deposit
155	31C	100/90	1 60° jt, 1 45° jt, smooth 2 20° jts, 1 30° jt in 0.2', 2 10° jts in 0.8' possible slickenside at 156.5'
160	32C	100/90	1 70° jt w/calcite dep & 2 30° jts in 0.3', 1 horiz jt, 3 10° jts in 2.8' 1/16" open
165	33C	100/95	2 30° jts, 2 20° jts, 1 45° jt in 2.5', thin calcite deposit
165	34C	98/90	2 10° jts 1 20° jts in 3.5', thin calcite dep, 1/16" open jts
170	35C	90/85	1 horiz jt, 1 45° jt in 0.8', thin calcite dep, 1 20° jt, 2 10° jts 1/16" open
175	36C	95/80	1 horiz jt, 1 10° jt, 4 thin healed hts
175	37C	98/90	3 60° jts, 1 20° jt in 2.2' 1/16" open, thin calcite deposit, 2 30° jts at 180'
180	38C	100/80	2 45° jts, 1 80° jt in 0.8', thin calcite & talc deposits, irregular, 1/16" open
185	39C	98/80	3 60° jts, 1 10° jt in 2.0' slightly rough 1/16" open, thin calcite deposits

Boring started 5-16-74, completed 6-11-74  
Final depths: Boring = 189.5' Casing = 43'  
Casing diameter = 4" & 3"  
Average depth of ground water observed in the boring = 19.8' = El. +303.7

REMARKS:  
1.5' of concrete at surface.  
Drilled ahead of casing from 9' to 43' depth; casing blows not reported.  
Three water pressure tests with packers run in this borehole.

**BORING NUMBER AM-74**  
Ground surface elevation + 259.0

0	31	1.5, 6	Med cpt brn-red si fine sand (Prob Fill) (SM)
5	2D	3, 6, 8	Med cpt brn-red cl & si f sand (Residual Soil) (SC-SM)
10	3D	8, 9, 25	Cpt brn si f sand, tr dk minerals (SM)
15	4D	12, 12, 23	Cpt light gry-brn si f sand (SM)
20	5D	14, 19, 28	Do 4D, more compact (SM)
25	6D	20, 31, 44	Brn si f sand, tr rock fgmts (SM)
30	7D	67, 120/4'	Do 6D, numerous rock fragments (SM)
35	1C	65/0	Hi wthrd & dec brn, vy soft diorite, friable, crumbles to sand
40	2C	40/0	Vy loose & soft f sand to 40', jtd & bkn qtz-dio
45	3C	65/15	Jtd, mod wthrd grano-diorite, 4 jts @ 30°, 2 jt 45°, 2 jts @ 60°, 1 vert jt, 1 jt @ 80°, thin cl seams on jts
50	4C	65/0	Hi jtd & bkn lt gray granodiorite, wthrd & bkn 46'-47', cl seams on jts @ 47.5', 50'-51'
55	5C	65/0	3 jts @ 70°, stains on jts, 15 jts 20°-30°, several small pcs, 1 pc 0.3' @ 52.5'
60	6C	45/10	Lt gry granodiorite & quartzite, several small pieces
65	7C	30/0	0.2' brn cly sm @ 61', jtd & mod wthrd-wthrd dk grn hornblende-diorite, 1 jt @ 10°, 3 jts @ 20°, 2 jts @ 30°, 1 jt @ 45°, smooth to slightly rough
70	8C	65/0	4 Horiz jts, 8 jts @ 10°-20°, 2 jts @ 45°, 1 jt @ 60°, 2 jts @ 80°, smooth, dk stains on jts
75	9C	80/10	Jtd & mod wthrd, grn hornblende diorite, vert jting 71'-73', thin cl & f sand deposits (gouge)
80	10C	60/0	Jtd & bkn, wthrd, brn to 80.6'
85	11C	90/45	Jtd, gry qtz-dio, 1 jt @ 80° @ 81.6', 3 jts @ 20°, 1 horiz jt, 1 30° jt, thin talc dep, 2 jts @ 86° @ 85', 1 jt @ 60° @ 86'
90	12C	85/70	Mod jtd, bkn & wthrd 85.2' to 87.6', 1 horiz jt, 1 60° jt in 0.6'; 1 15° jt & 1 vert jt in 0.2'; 1 30° jt, smooth, thin talc
95	13C	95/80	Rel snd hornblende & qtz-dio, 2 45° jts, smooth, thin talc dep on jts, 1/8" angular qtz veins
100	14C	98/70	Mod jtd, 2 jts 20°, rough, 2 jts 10°, 1 jt 30°, 2 jts 45°, 1 jt 45°, calc dep, 1/4" qtz veins @ 25° & 45° chlorite inclusions
105	15C	98/85	Rel snd gry, 2 jts @ 10°, 1 jt @ 30°, 1 jt @ 60°, irreg 1/2" qtz veins, 60° & vert fol



Boring started 11-25-74, completed 12-3-74  
Final depths: Boring = 110.0' Casing = 66'  
Casing diameter = 4" & 3"  
Average depth of ground water observed in the boring = 12.5' = El. +246.5

REMARKS:  
Lost water at 52' depth.  
This boring was made after completion of the original program to investigate the deep weathering and jointing which appeared in Boring No. AM-15.

(CONTINUED)

(CONTINUED)

Note: Borings Nos. AM-72 and AM-73 were in progress in the field on Dec. 30, 1974 and logs of these borings will be included on a subsequent revision of this drawing.

DESIGNED	DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION		<b>WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY</b> NUESER - RUTLEDGE - WENTWORTH & JOHNSTON CONSULTING ENGINEERS 415 MADISON AVE., NEW YORK 17, N. Y. SUBMITTED <i>William H. Nueser</i>	DE LEUW, CATHAR & COMPANY GENERAL ENGINEERING CONSULTANT HARRY WEESE & ASSOCIATES GENERAL ARCHITECTURAL CONSULTANT	<b>ROCKVILLE ROUTE (A011)</b> LOGS OF AM-SERIES BORINGS Nos. AM-71 to AM-74 SCALE: VERT. 0' 3' 6' 9' DRAWING NO. F-A-122
DRAWN	12/74	F-1	General Notes & Legend							
CHECKED	12/74	F-A-96 to	Plans & Geological Sections							
APPROVED	12/74	F-A-104								

REPORT NO. 117, MRWJ SERIES  
SECTION A011, ROCKVILLE ROUTE  
SUBSURFACE INVESTIGATION

APPENDIX A  
SUMMARY OF INFORMATION FROM  
SUPPLEMENTARY INVESTIGATIONS,  
INCLUDING THE FOLLOWING EXCERPTS:

1. Nesbitt, R.H. "Report on Results of Borehole Photography, Washington Metropolitan Area Transit Authority, Design Section A011," pages 1 to 8.
  2. Tarkoy, P.J. and E.J. Cording, "Feasibility of Using a Tunnel Boring Machine on Project A-11, METRO Washington, D.C.," December 10, 1974, title page and pages 22, 23 and 24.
  3. Cording, E.J. and A.S. Nieto, "Direct Shear Tests on Samples of Clay Gouge and Rock Surfaces from Shear Zones in Washington METRO Tunnels," Sept. 27, 1974, Title page and Application of Results, pages 7 and 8 and Table 1.
  4. Nieto, A.S. and E.J. Cording, "Direct Shear Tests on Natural Fractures in NX Core from Project A-11, Washington, D.C. METRO," Dec. 11, 1974, Title page and Summary and Conclusions, pages 1 to 3.
-

REPORT ON RESULTS OF BOREHOLE PHOTOGRAPHY  
WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY  
DESIGN SECTION A011

By

Robert H. Nesbitt  
Engineering Geologist

1. Pursuant to a telephonic communication on May 30, 1974, followed by a letter of the same date, both from Dr. Philip C. Rutledge, Mueser-Rutledge-Wentworth-&-Johnston (MRWJ), the writer agreed to review the borehole photography operations of the Corps of Engineers in Design Section A011 of the Wisconsin Avenue <sup>reach</sup> of "METRO" in Bethesda, Maryland. Following arrangements with Mr. Arthur M. Mayes (MRWJ), on June 11, 1974 the writer inspected and logged the cores of the five deep borings, made for borehole photography. These included boreholes AM-3U, AM-4, and AM-60 in the Bethesda station and AM-68 and AM-70 in the Medical Center Station. On the following day he made a general reconnaissance of the site. Both the inspection and reconnaissance were made with the effective assistance of Mr. V. Tepordei (MRWJ). On July 9 and 10, 1974 the writer visited the Corps of Engineers Southwestern Division Laboratory in Dallas where, with Mr. William Tanner who photographed and logged the borings, he carefully studied the picture film strip for each boring and discussed the various features to be included in the Laboratory report. Here it should be stated that due to cloudy conditions in the borehole water, the photographs of boreholes AM-3U and AM-68 were too

indistinct to permit logging. In view of the very considerable delay in any attempt to prepare these borings for rephotography, and the considerable uncertainty from past experience that such efforts might be successful, it was concluded by mutual consent with MRWJ that the photographic report would be based on the three borings which provided good pictures. It was noted that correlations between borings AM-4 and AM-60 would partially offset the lack of AM-3U.

## 2. Scope of Laboratory Report

The principal objective of the borehole photography was to provide information on the structure and orientation of bedrock foliation, jointing and faulting at the location of the five borings in Design Section A-011 of METRO. To accomplish this, the Laboratory report would include:

- (a) borehole<sup>e</sup> photo logs,
- (b) polar diagrams of joint and foliation structure (faulting if present),
- (c) plan and profile plots of joint patterns at each borehole<sup>e</sup>, and
- (d) a narrative explaining the illustrative data and summarizing the principal joint and foliation structure at each boring location.

Among the considerations on which our client hoped the photography would provide information was the occurrence of any major shear or fault zone which could create a construction problem. The writer's comments on various elements of the Laboratory report,

together with his opinion on the significance of the data in determining tunnelling conditions at each boring site, follow.

3. Bedrock Geology at the Bethesda and Medical Center Stations is well outlined in the profiles on Drawings F-A-17 and F-A-22, respectively, from the MRWJ report titled "Final Report - Subsurface Investigations Rockville Route, Stations 221+00 (A009) to 590+00 (A013)". These drawings, supplemented by the photography borings, indicate quartz diorite and diorite gneiss and schist to be the dominant rock types. Secondary chloritic or serpentized and calcitized zones occur here and there through the Section. The occurrences of the latter were scrutinized in the photo study for any possible significance in rock stability. Of obvious importance is the depth of rock decomposition and weathering at each boring, which is considerably greater than was observed in the photographic study of Section A-09 two years ago. Decomposition and severe weathering extend to the following depths for the five borings logged for photography: AM-3U, 63.0 ft; AM-4, 79.6; AM-60, 81 ft; AM-68, 33.0 ft; AM-70, 32.0 ft. Structurally Borings AM-4 and AM-60 show a predominant N-S strike to the foliation with a prevailing steep westward dip. In contrast the photography of Boring AM-70 failed to produce any marked foliation. Joint orientation is well demonstrated in the drawings accompanying the Laboratory report (NOTE: All structure orientations in logs and drawings refer to magnetic north.)

#### 4. Comments on Laboratory Report

The writer studied all the film for the three borings, with Mr. Tanner's preliminary logs as references. During this study we singled out the joints or suspected shear zones considered to be of structural significance and flagged these with asterisks in the left margin of the log. This procedure simplified the construction of the "overlay" profiles, since a drawing of suitable scale could not possibly show all joints that appear on the polar plots. The writer concurs completely with the logging of the photographs and with the procedure used by the Corps Laboratory for illustrating joint structure. There is no attempt to include lithologic information since the latter can be described more accurately in field logs. The reader is referred to the Laboratory report for the procedure used in developing the polar plots and profile and plan "overlay" drawings. A diagram illustrating the trigonometric procedure for translating true dip of joints in the boring to apparent dip on the profiles is attached to the report.

#### 5. Summary of Conditions at Each Boring

##### a. Boring AM-3U. (No pictures due to cloudy water).

Top of tunnel Elev. 248 (depth 100.0 ft). Below a depth of 63 ft (E1. 285.0) the gneissic rock has several sound massive-structure zones, such as the thick section which I logged between depths 87.3 and 127.3 (E1. 260.7 to 220.7). Another similar sound section extends between depths 128.0 and 136.4 (E1. 220.0 to 211.6), which latter depth is just above the base of the tunnel opening. However, this rather favorable picture is offset by the occurrence of



significant fracturing and staining of joints in the Section between depths 80.8 and 87.3 (El. 267.2 to 260.7). On this basis, and considering the size of the Station opening, continuous steel support of the arch is indicated.

b. Boring AM-4. Top of tunnel Elev. 247.6 (depth 100 ft). The top of sound rock in this boring occurs at Elev. 267.7 (Depth 79.6). With the exception of a few high-angle unstained joints between Elev. 260.5 and 257.2 (depth 87.1 to 90.4) and a closely fractured section extending for 7 ft. directly above the top of the tunnel, the gneissic rock possesses massive structure between the sound rock line and the top of the tunnel. As the illustrative data in the Laboratory report indicate, two principal joint sets occur, i.e., a north-south set with  $40^{\circ}$  to  $70^{\circ}$  dips to the west, and a northeast striking set with dips of  $30^{\circ}$  to  $65^{\circ}$  southeast. The occurrence of the closely-fractured and jointed zone in the 7 ft. section directly above the tunnel roof would indicate continuous rib support of the arch. To add an additional dimension to the information obtained from AM-4, joint structure developed from the later was plotted in relation to AM-60 in the first overlay profile.

c. Boring AM-60. Top of tunnel El. 226.0 (depth 120.0 ft). Moderately to intensely weathered rock occurs in most of the interval from top of weathered rock at El. 306.7 (depth 39.3 ft) to the sound rock line at Elev. 245.4 (depth 100.6). Closely jointed to massive gneissic rock characterizes the section extending

from 20 ft. directly above the top of the tunnel to 6 ft. below the arch. Directly below, between Elev. 220.0 and 219.5, is a serpentized or chloritized joint striking N 30°E and dipping 60° NW which could be structurally significant in excavation. Below this 1/2 ft. zone the rock is moderately jointed to massive fresh gneiss. As in Boring AM-4 the photography again shows a prominent N-S strike and rather steep westward dip for the foliation of the rock. On the polar plot for this boring the dominant joint set appears related to the schistosity or foliation of the strata. However, random joints, which can be just as troublesome from the rock excavation and support standpoint as the dominant set, do occur and should be kept in mind in evaluating the photo log data.

d. Boring AM-68 (No pictures due to cloudy water).

Top of tunnel Elev. 235, (depth 83 ft). Examination of the cores of this boring indicate massive sound fine grained dioritic rock from a depth of 64.5 feet to the bottom of the hole, well below the bottom of the tunnel opening. Very few moderately steep to steep diagonal joints were observed. Most of the core is fractured horizontally. However, only a few steep joints intersecting these horizontal breaks would be necessary to produce buckling of the rock forming the arch of the opening during excavation. This situation should be a determining factor when selecting the type of rock support, bolting versus ribs.

e. Boring AM-70. Top of Tunnel Elev. 238.5 (depth 85.0). The base of intense weathering in this boring occurs at Elev. 291.5 (depth 32.0 ft). From this level down to the top of the tunnel the rock is chiefly hard, gray, closely jointed and blocky fresh diorite. The laboratory report properly recognizes two major joint sets. The dominant set appears to strike N 55° to N 85° W, and dip 65° to 75° NE. The other set strikes N-S and dips 50° to 75° E. However, there is quite a scattering of random joints, particularly open and low angle ones which must be kept in mind in evaluating arch stability. Of structural significance are several closely-space low-angle stained joints between Elev. 66.9 (depth 256.6) and Elev. 254.8 (depth 68.7). Also significant are a flat joint at E1. 249.5 (depth 74.0) and two open joints showing broken rock between E1. 246.5 (depth 77.0) and E1. 245.8 (depth 77.7) (See the log for the details of the above-mentioned joints). From the top of the tunnel down to E1. 186.6 (depth 136.9) the hard fine grained diorite is moderately massive with scattered steep joints in several directions. Directly below this interval is a 3 ft. thick blocky zone, followed by a rather massive section down to E1. 165.4 (depth 158.1).

## 6. Conclusions

a. A north-south strike with a steep westward dip characterizes the foliation in borings AM-4 and AM-60 in the Bethesda Station. The dominant joint structure in these two borings corresponds with the same orientation.

b. Core studies for Borings AM-68 and AM-70, and bore photos of the latter, disclosed more massive and unfoliated rock for the Medical Center Station. They also showed considerably shallower covers of decomposed and weathered rock. Horizontal, as opposed to diagonal, fracturing marked much of the massive hard rock in Boring AM-68, and bore photos for Boring AM-70 show a N 55° - 85° W Strike and 65° to 75° NE dip for the dominant joint set, with a subordinate set striking N-S and dipping 50° - 75° E.

c. The "overlay" profiles show a fairly wide orientation range for the open joints emphasized in red. Low-angle stained and open joints, which are prominent in Boring AM-70 and less prominent in Borings AM-4 and AM-60 should be flagged as possible trouble makers (see para. 5.e.).

d. No feature which could be identified as a major fault or shear zone was disclosed by the photography.

e. It is the writer's opinion that frequently reentrant blocky, broken and stained zones within the best rock could be as important in determining excavation and support procedures as the orientation and structure of joints and foliation disclosed by the photography.

Robert H. Nesbitt

August, 1974

Feasibility of Using a Tunnel Boring Machine  
on Project A-11, METRO  
Washington, D.C.

- Laboratory Test Results  
(from Sept. 1974 Report)
- Rock Type and Hardness on  
Projects A-6 and A-11, Metro
- Effect of Rock Hardness on  
Penetration Rates

Prepared by

P. J. Tarkoy  
and  
E. J. Cording

for

Mueser, Rutledge, Wentworth & Johnston  
415 Madison Avenue  
New York, New York 10017

December 10, 1974

effect on rates have been the shear zones which, beside gouge, may consist of fractured and weathered rock. For some machines, penetration rates have been known to drop as low as 1 ft/hr and may average less than one half of the otherwise normal rates, at least while the shear zone intersects the heading. This is generally a result of reduced cutterhead rotation which is required to keep fractured rock from damaging cutters and reduced thrust to keep the cutters from being buried in the face. In soft materials, clogging of the cutterhead is also known to occur.

---

#### ROCK CONDITIONS ALONG THE A-11 ALIGNMENT

Available geological information and borings suggest that geological conditions for the twin-tunnels of the A-11 alignment are not unlike conditions encountered in the outbound A-6 tunnel, although zones of the softer chlorite schist on project A-11 do not appear as prominent as at the south end of project A-6. There are the typical varieties of chlorite, hornblende, quartz, and biotite, schists, diorites, and diorite gneisses. Although lithologic relationships are complex and in many cases gradational, general areal distribution of units can be delineated as shown in Figure 7. The map is based on preliminary information transmitted by Mueser, Rutledge, Wentworth, & Johnston (October 1974). Discrepancies between the lithologic map and sample descriptions are a result of inherently complex lithologic inter-relationships both laterally as well as with depth.

#### Rock Hardness

Samples selected by Mueser, Rutledge, Wentworth, & Johnston from exploratory borings along sections of the A-11 alignment were tested for rock hardness, for unconfined compressive strength, and sonic velocity.

Hornblende-Schistose Gneiss  
(Chlorite Schist & Quartz  
Diorite Gneiss)

Quartz-Diorite  
Gneiss (Chlorite  
Schist &  
Schistose  
Gneiss)

Hornblende-  
Schistose  
Gneiss &  
Chlorite Schist  
(Quartz Diorite  
Gneiss)

Quartz-Diorite  
Gneiss (Quartz-  
Hornblende Gneiss)

Hornblende-Quartz  
Schistose Gneiss  
(Chlorite Schist  
Phyllite, Diorite  
Gneiss, Gabbro?)

Quartz-Diorite  
Gneiss (Schistose  
Gneiss & Diorite  
& Chlorite  
Schist)

Hornblende-  
Schistose Gneiss  
(Chlorite Schist)

Quartz-Diorite Gneiss

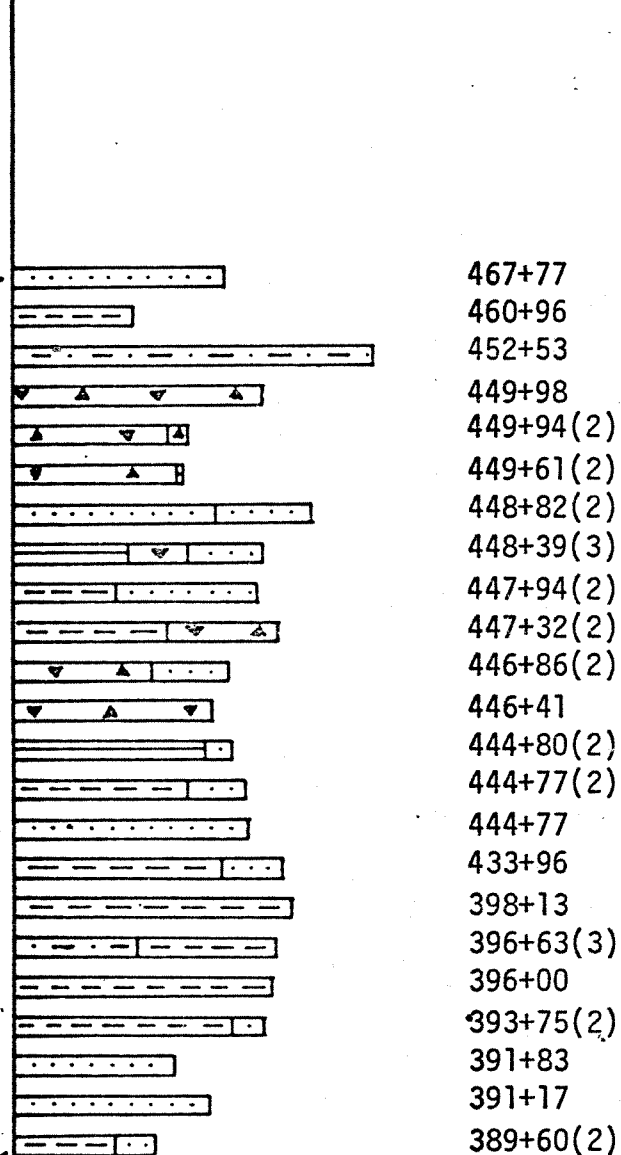
Quartz-Hornblende  
Gneiss (Chlorite  
Schist)

Quartz-Hornblende  
Diorite Gneiss  
(Chlorite Schist  
& Schistose  
Gneiss)

Total Hardness

$$H_T = H_R \sqrt{H_A}$$

0 100 200 Tunnel Station



ROCK LEGEND

- Chlorite Schist
- Hornblende Schist
- Diorite, Gneiss, & Diorite Gneiss
- Hornblende Diorite
- Quartz, Quartzite
- Sample Location

Figure 7. Test Sample Lithology, Rock Hardness, and Location Along the A-11 Alignment.

Results are summarized in Table III in Part 1. Sample locations and respective Total Hardness ( $H_T$ ) are shown in Figure 7. Comparison of Figures 4 and 7 suggests that samples from A-11 borings were generally harder than the slightly altered schists and chlorite schists which were present in the first 2000 feet of the outbound A-6 tunnel, but had similar ranges of hardness to the rock north of the first 2000 ft of the A-6 tunnel. The mafic lithologies tend to be softer than the felsic rocks as illustrated by rock group averages in Figure 8. Test results indicate that 1) rocks and their hardness along the A-11 alignment are similar to those in A-6 except for the soft chlorite schists in the southernmost 2000 ft section of A-6, 2) schists bored in N.Y.C. are similar in rock type and hardness to expected rock along the A-11 alignment, and 3) the most significant factor, therefore, will be the effect of machine design on penetration rates. Relationships defined between rock hardness and penetration rates for the machine operating in the A-6 (19-ft-diameter) tunnel and the machine which bored the (11-ft-diameter) tunnel in N.Y.C., are valid for machines having the same respective diameters and design parameters. Machine diameter is inversely proportional to cutterhead rotational speed and therefore penetration rates. Other variables, and their effects, such as cutter design have been less well defined.



Direct Shear Tests on  
Samples of Clay Gouge and Rock Surfaces  
From Shear Zones in Washington Metro Tunnels

Prepared for  
Mueser, Rutledge, Wentworth, and Johnston  
415 Madison Avenue  
New York, New York 10017

by

E. J. Cording  
and  
A. S. Nieto

September 27, 1974

## 5. Application of Results

In applying the laboratory results of tests on clay gouge for estimating the strength along shear zones in the field, it is recommended that the residual angle of friction, rather than the peak angle of friction of the clay be used. The peak angles of friction for the remolded clays are dependent on the method of remolding of the sample and on details of the test and confining pressures in the test. The laboratory peak strengths may thus have little relation to the peak strength of the undisturbed but geologically sheared materials in the field.

It is also recommended that some interference for the clays be assumed, because most of the natural rock surfaces and clay seams are relatively thin with respect to the amplitude of the small-scale irregularities. It is felt that  $\phi_r = 11^\circ$  to  $14^\circ$  is a realistic value for the residual angle of friction under interfering conditions.

Finally, the waviness of the large-scale irregularities may result in interlocking conditions which will cause dilatancy of the shear surfaces as they ride over irregularities. For the sheared surfaces in Washington, such waviness can add inclinations,  $i$ , of approximately  $0$  to  $5^\circ$  to the friction angle.

The total angle of friction along the sheared surfaces could then be taken as:

$$\phi_r + i = (11^\circ \text{ to } 14^\circ) + (0^\circ \text{ to } 5^\circ),$$

or a range of  $11^\circ$  to  $19^\circ$ . It is recommended that  $\phi_r + i$  of  $15^\circ$  be used for continuous shears and shear zones whose surfaces are planar to slightly wavy, and which contain thin (1/64 in. to 1/2 in.) clay gouge fillings. Where extremely planar seams with clay gouge filling are present, the lower values for  $\phi_{\text{total}}$  would be appropriate.

6. Future Testing

Natural joints from core samples on project A-11 are presently being prepared for testing. It is expected that higher friction values will be obtained than are presented in this report because some of the surfaces have not been previously sheared and many surfaces do not contain clay gouge fillings. For surfaces not previously sheared, peak friction values rather than residual values may more closely represent the field strength conditions.

TABLE 1: WASHINGTON METRO  
 Drained Direct Shear Tests; Residual Strengths;  
 Clay Gouge Between Rock Surfaces

By: A. S. Nieto-Pescetto  
 March, Spril 1974

A. Location: Project A4b, Shear Zone 9

Sample	Type	Thickness inch	Normal Stress psi	Shear Stress psi	$t/\sigma$	$\phi'_r$	$P_w$ %	$L_w$ %	Description
W-A-1	1	.062	22.82	3.18	.139	8.0°	37.8	65.5	Red plastic clay coating (originally coated WA-3, WA-4) non-interfering
W-A-2	1	.050	42.70	5.90	.139	8.0°	37.8	65.5	Red plastic clay coating non-interfering
W-A-3	1	.080	20.80	3.82	.183	10.4°	39.6	51.8	Reddish gray micaceous clay non-interfering
W-A-4	1	.070	42.70	7.41	.174	9.9°	39.6	51.8	Reddish gray micaceous clay non-interfering
W-B-1	2	.062	42.70	7.41	.174	9.9°	30.4	40.5	Silty, sandy gray clay non-interfering
W-B-2	2	.062	20.95	3.91	.186	10.5°	30.4	40.5	Silty, sandy gray clay non-interfering

B. Location: Project A-6, WT-1 Shaft; shear zone bounding wedge.

WC-2	3	.062	20.4	5.2	.254	14.3°			interfering
WC-4	3	.062	51.2	10.2	.265	11.3°			non-interfering
WC-1	4	.015	21.2		.240	13.5°			

- Type: 1 - Sample sheared between two 4" x 7" slabs of Bedford Limestone.  
 2 - Sample sheared between two 4" x 7" slabs of Berea Sandstone.  
 3 - Clay sample sheared between slickensided natural shear surface and sulfaset slab, 4" x 5".  
 4 - Very thin clay sample of #3 sheared between smooth limestone slab, 2" x 6".

Note: All values represent individual tests, no incremental loading was used.

Direct Shear Tests on  
Natural Fractures in NX Core  
from Project A-11, Washington, D.C. Metro

Prepared for

Mueser, Rutledge, Wentworth and Johnston  
415 Madison Avenue  
New York, New York 10017

by

A. S. Nieto  
and  
E. J. Cording

December 11, 1974

## SUMMARY AND CONCLUSIONS

1. Our previous report, dated Sept. 27, 1974 summarized the results of direct shear tests on clay gouge samples obtained from shear zones in existing Metro tunnels, Projects A-4b and A-6, Connecticut Avenue Route. The gouge samples were tested in the laboratory by shearing them between rock or rock-like surfaces. Residual (or minimum) angles of friction for the clay gouge were in the range of  $8^\circ$  to  $14^\circ$ . The recommended angle of friction to be used for shear zones in the field included the minimum angle of friction ( $\phi_r = 8^\circ$  to  $14^\circ$ ) plus a dilatant component of shear strength ( $i = 5^\circ$ ) to account for the overriding of irregularities typically occurring on the shear planes in the field. Thus the angle of friction for shear zones in the field was estimated to be  $\phi_r + i = 13^\circ$  to  $19^\circ$ . A conservative estimate of the shear strength for shear zones was considered to be  $\phi_r + i = 15^\circ$ .

2. This report summarizes results of direct shear tests on natural joint surfaces in NX rock core obtained from Project A-11, Connecticut Avenue Route, Washington Metro. Five of seven core segments sent us were tested. Of the five samples, four were unweathered mica-quartz gneisses and one was a partially weathered chlorite schist.

3. Some joints contained no fillings, others were coated with thin layers consisting principally of calcite, although some contained small amounts of clay, pyrite, and chlorite. Most of the coatings were thin (less than 1/32 in. thick) and discontinuous.

4. Two direct shear tests were performed on each sample. Each sample was first tested at an average normal stress of 25 psi, then at a normal stress of 50 psi. (Peak friction angles and dilation angles for the second test would be expected to be lower than for the first because some of the irregularities may be removed during shearing in the first test).

5. The joint surface of the chlorite schist was bounded by platy minerals and exhibited much lower peak and minimum friction angles than the four gneissic samples whose joint surfaces were bounded by equi-dimensional minerals such as quartz and calcite. There was little difference in strength between gneissic samples with coatings (principally calcite) and gneissic samples without coatings.

6. Individual peak friction angles for the gneiss samples ranged from 29° to 38°. The higher peak values were generally associated with samples having rougher surfaces. The average peak friction angle for the schist was about 20°.

7. Nearly all the tests on the gneissic rocks displayed dilatational behavior during the early parts of the test. The values range from about 1° to 6°. Dilation angles for the schist were negative, indicating that compression and crushing of the surface of the joints took place as the sample was sheared.

8. Minimum friction angles for the gneiss specimens were fairly typical for unweathered discontinuities in rocks of this kind and ranged between 24° to 27°. The average minimum friction angle for the schist was about 16.5°. This low value is caused by platy minerals which coat the

foliation lenses or laminae of the schist. A drained test on remolded schist (ground up and sieved through a #70 sieve) yielded an average minimum friction angle of  $29^\circ$ . This high value was a result of the disruption of the foliation lenses containing platy minerals and the presence of quartz in the ground-up sample.

9. It is recommended that the following values of shear strength be used for naturally occurring joints and shear zones in the tunnels on the Connecticut Avenue route; Project A-11:

- a) Planar joints, unweathered, either no filling or calcite filling, gneissic rock:  $\phi_{\text{minimum}} = 27^\circ$
- b) As above, but joint is tight, and rough or wavy: add  $i$  value of  $3^\circ$  or more, depending on the angle of the irregularity, to the minimum angle of  $27^\circ$ .
- c) Planar joints with thin clay fillings or slickensided surfaces in gneissic or schistose rock:  $\phi_{\text{minimum}} = 15^\circ$
- d) All planar joints in schistose rock that are parallel to foliation (No. 1 joints) and are bounded by platy minerals:  
 $\phi_{\text{minimum}} = 15^\circ$
- e) Shear zones with clay gouge fillings, slightly undulatory ( $i = 5^\circ$ ):  $\phi = 15^\circ$ .

10. In addition to the minimum shearing resistance, the waviness, tightness, and continuity of the joint surface is a major factor affecting the stability of the rock block. The relationship shown in Figure 8, of the paper by Cording and Mahar summarizes the effect of waviness on block stability.